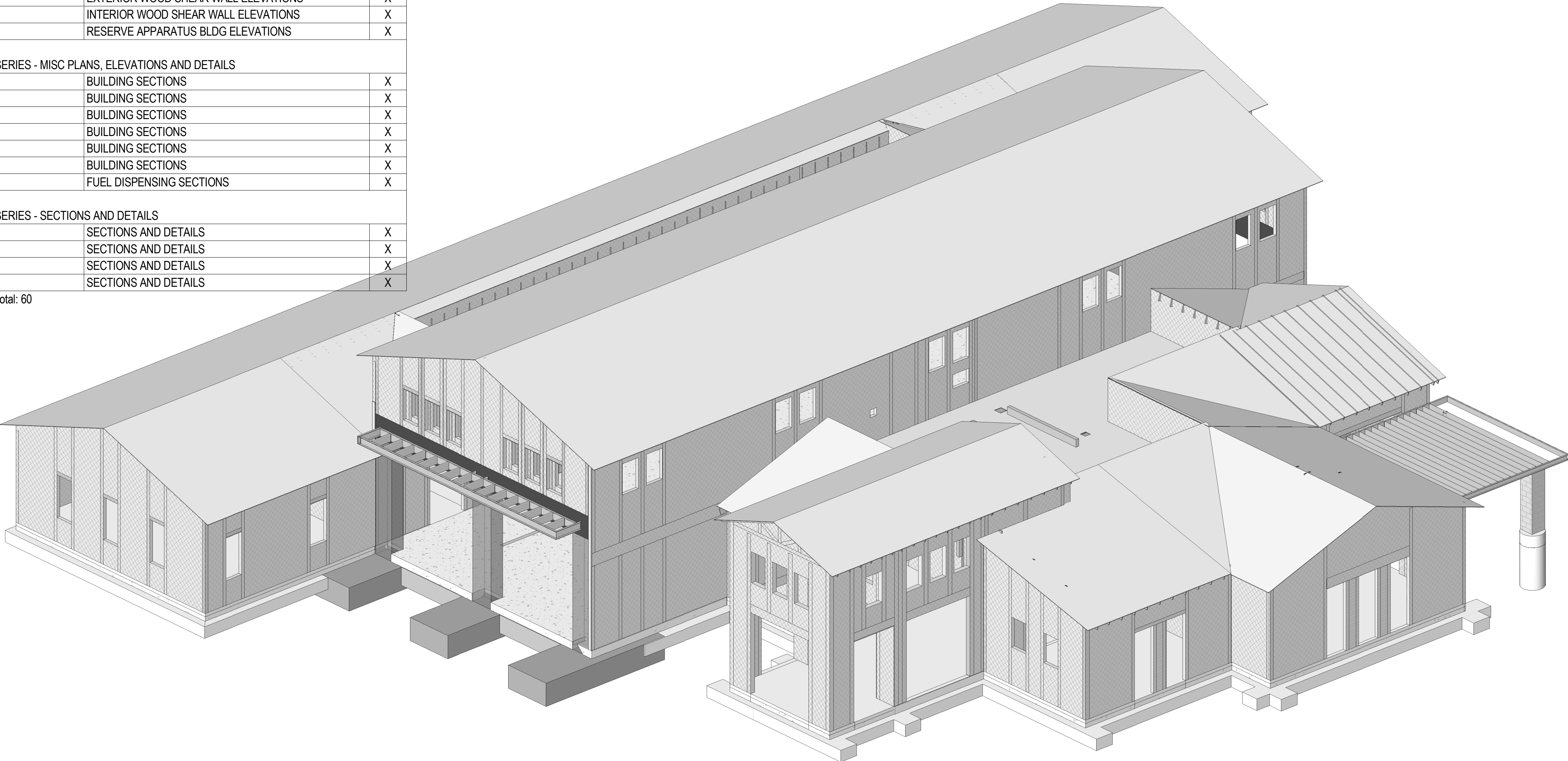
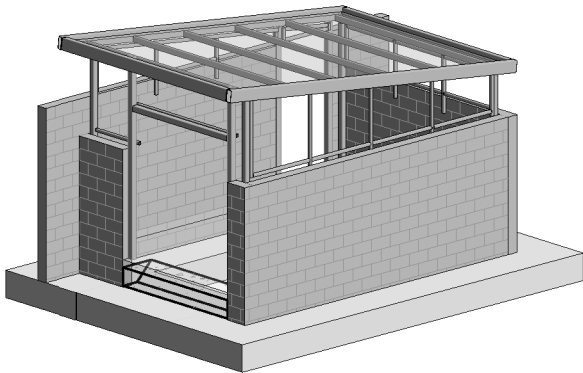
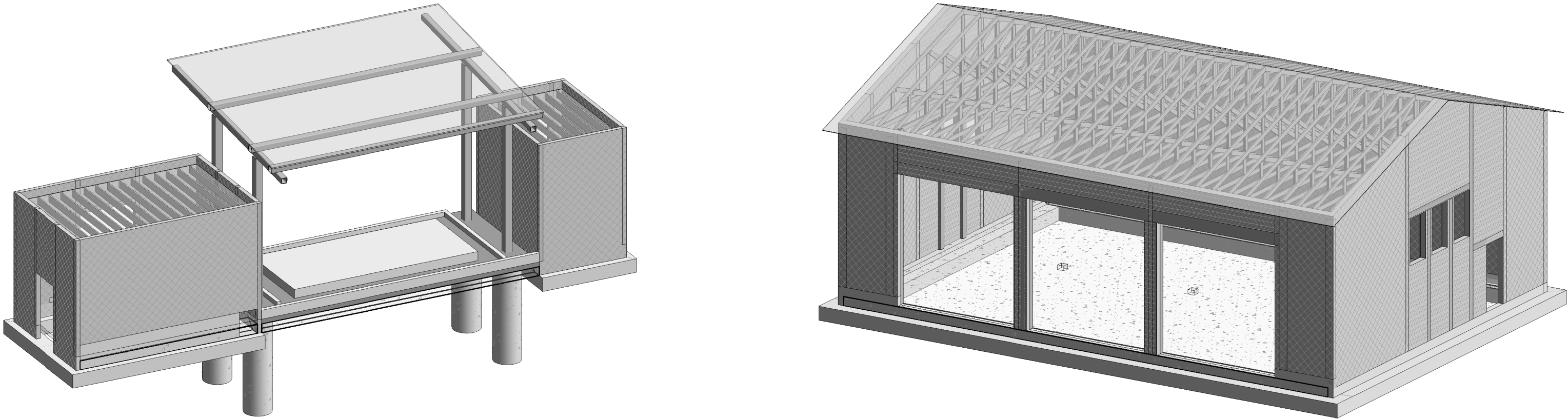


FIRE STATION 46

OCTOBER 31, 2025 - PLAN CHECK SUBMITTAL



Sheet Number	Sheet Name	PLAN CHECK SUBMITTAL
S0XX SERIES - GENERAL NOTES		
S000	COVER SHEET	X
S001	GENERAL NOTES	X
S002	GENERAL NOTES	X
S003	GENERAL NOTES	X
S004	GENERAL NOTES	X
S005	GENERAL NOTES	X
S006	STATEMENT OF SPECIAL INSPECTIONS	X
S008	GENERAL NOTES ABBREVIATIONS	X
S0XX SERIES - TYPICAL DETAILS		
S011	TYPICAL REINFORING DETAILS	X
S012	TYPICAL REINFORING DETAILS	X
S013	TYPICAL CONCRETE DETAILS	X
S015	TYPICAL STEEL FRAMING AND DECKING DETAILS	X
S021	TYPICAL CMU DETAILS	X
S030	TYPICAL WOOD DETAILS	X
S031	SECTIONS AND TYPICAL DETAILS	X
S031B	TYPICAL WOOD DETAILS	X
S032	TYPICAL WOOD DETAILS	X
S032B	TYPICAL WOOD DETAILS	X
S033	TYPICAL WOOD DETAILS	X
S034	TYPICAL WOOD DETAILS	X
S034B	TYPICAL WOOD DETAILS	X
S035	TYPICAL WOOD DETAILS	X
S036	TYPICAL WOOD DETAILS	X
S037	TYPICAL WOOD DETAILS	X
S038	TYPICAL WOOD DETAILS	X
S039	TYPICAL WOOD DETAILS	X
S039B	TYPICAL WOOD SHEAR WALL DETAILS	X

Sheet Number	Sheet Name	PLAN CHECK SUBMITTAL
S1XX SERIES - OVERALL VIEWS, LOADING PLANS, & OTHER PLANS		
S100	ISOMETRIC VIEWS	X
S101	ISOMETRIC VIEWS	X
S110	LOADING PLANS	X
S2XX SERIES - PLANS		
S210	FOUNDATION PLAN	X
S220	LOW ROOF FRAMING PLAN	X
S230	HIGH ROOF FRAMING PLAN	X
S240	ROOF WOOD DIAPHRAGM PLANS	X
S250	RESERVE APPARATUS PLANS AND SECTION	X
S260	FUEL AREA BUILDING AND CANOPY PLANS	X
S270	TRASH ENCLOSURE	X
S271	TRASH ENCLOSURE ELEVATION AND DETAILS	X
S3XX SERIES - FOUNDATION PLANS, SCHEDULES AND DETAILS		
S301	CONCRETE FOUNDATION DETAILS	X
S302	CONCRETE FOUNDATION DETAILS	X
S310	CONCRETE FRAME ELEVATION AND FOUND	X
S311	CONCRETE FRAME DETAILS	X
S312	CONCRETE FRAME DETAILS	X
S5XX SERIES - WOOD SHEAR WALL ELEVATIONS & DETAILS		
S501	EXTERIOR WOOD SHEAR WALL ELEVATIONS	X
S502	EXTERIOR WOOD SHEAR WALL ELEVATIONS	X
S503	EXTERIOR WOOD SHEAR WALL ELEVATIONS	X
S504	EXTERIOR WOOD SHEAR WALL ELEVATIONS	X
S505	INTERIOR WOOD SHEAR WALL ELEVATIONS	X
S510	RESERVE APPARATUS BLDG ELEVATIONS	X
S7XX SERIES - MISC PLANS, ELEVATIONS AND DETAILS		
S701	BUILDING SECTIONS	X
S702	BUILDING SECTIONS	X
S703	BUILDING SECTIONS	X
S704	BUILDING SECTIONS	X
S705	BUILDING SECTIONS	X
S706	BUILDING SECTIONS	X
S720	FUEL DISPENSING SECTIONS	X
S8XX SERIES - SECTIONS AND DETAILS		
S801	SECTIONS AND DETAILS	X
S810	SECTIONS AND DETAILS	X
S811	SECTIONS AND DETAILS	X
S812	SECTIONS AND DETAILS	X

Grand total: 60



County of Los Angeles
DEPARTMENT OF PUBLIC WORKS BUILDING & SAFETY DIVISION

Los Angeles Regional Uniform Code Program

STRUCTURAL OBSERVATION REPORT FORM

STRUCTURAL OBSERVATION means the visual observation of the structural system, for general conformance to the approved plans and specifications, at significant construction stages and at completion of the structural system. Structural observation does not include or waive the responsibility for the inspections required by Section 108, 1705 or other sections of the Los Angeles County Building Code.

This report includes all construction work through _____ day of _____ 20____, Report No. _____, Page No. _____ of _____.

Project Address:	Structural Observer of Record(SOR):	SOR Phone No.:
Building Permit No.:	Observer Professional Lic/Reg. No.:	Observer Phone No.:

OBSERVED STRUCTURAL ELEMENTS AND THEIR CONNECTIONS				
FOUNDATION	WALL	FRAMES	DIAPHRAGM	ELEMENTS/CONNECTION OBSERVATION LOCATION
<input type="checkbox"/> Footing, Stem Walls	<input type="checkbox"/> Concrete	<input type="checkbox"/> Steel Moment Frame	<input type="checkbox"/> Concrete	
<input type="checkbox"/> Mat Foundation	<input type="checkbox"/> Masonry	<input type="checkbox"/> Steel Braced Frame	<input type="checkbox"/> Steel Deck	
<input type="checkbox"/> Caisson, Piles, Grade Beams	<input type="checkbox"/> Wood	<input type="checkbox"/> Concrete Moment Frame	<input type="checkbox"/> Wood	
<input type="checkbox"/> Retaining Foundation Hillside Special Anchors	<input type="checkbox"/> Others:	<input type="checkbox"/> Masonry Wall Frame	<input type="checkbox"/> Others:	
<input type="checkbox"/> Others:		<input type="checkbox"/> Others:		

NOTE DEFICIENCIES with the proposed corresponding corrective actions with respect to general conformance with the approved plans: (A final report by the structural observer which states that all observed deficiencies have been resolved is required before acceptance of the work by the building official.)

I DECLARE THAT THE FOLLOWING STATEMENTS ARE TRUE TO THE BEST OF MY KNOWLEDGE:

- I AM THE ENGINEER OR ARCHITECT RETAINED BY THE OWNER TO BE IN RESPONSIBLE CHARGE FOR THE STRUCTURAL OBSERVATION IN ACCORDANCE WITH THE REQUIREMENTS OF THE COUNTY OF LOS ANGELES.
- I, OR ANOTHER ENGINEER OR ARCHITECT WHO I HAVE DESIGNATED ABOVE AND IS UNDER MY RESPONSIBLE CHARGE, HAS PERFORMED THE REQUIRED SITE VISITS AT EACH SIGNIFICANT CONSTRUCTION STAGE TO VERIFY IF THE STRUCTURE IS IN GENERAL CONFORMANCE WITH APPROVED PLAN SPECIFICATIONS;
- ALL NOTED DEFICIENCIES WHICH REMAIN TO BE CORRECTED HAVE BEEN INDICATED ABOVE;
- I RECOMMEND THAT ACCEPTANCE OF THE STRUCTURAL SYSTEMS BY THE COUNTY OF LOS ANGELES BE WITHHELD UNTIL ALL OBSERVED DEFICIENCIES ARE CORRECTED.

(STAMP OF STRUCTURAL OBSERVER)

SIGNATURE OF STRUCTURAL OBSERVER _____ DATE _____
Version 2.0 (2017 LACBC) Dec. 8 2016

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

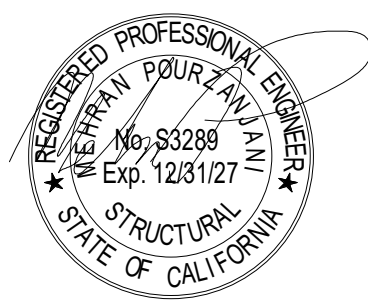
so
saiful-bouquet
structural engineers

725 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

COVER SHEET

FIRE STATION 46

MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA



THE ABOVE DRAWINGS AND SPECIFICATIONS ARE THE ORIGINAL DESIGNS AND ARRANGEMENTS REPRESENTED BY THE ARCHITECT AND REMAIN THE PROPERTY OF THE ARCHITECT. NO PART OF THESE DRAWINGS SHALL BE COPIED, DISCLOSED TO OTHERS OR USED IN CONNECTION WITH ANY WORK OR PROJECT OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY WERE PREPARED AND DEVELOPED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT. VERBAL CONTACT WITH THESE DRAWINGS OR SPECIFICATIONS SHALL NOT CONSTITUTE CONCLUSIVE EVIDENCE OF ACCEPTANCE OF THE ARCHITECT'S INSTRUCTIONS.

WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALED DIMENSIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THE OFFICE MUST BE NOTIFIED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS SHOWN IN THESE DRAWINGS. SHOP DRAWINGS MUST BE SUBMITTED TO THIS OFFICE FOR APPROVAL BEFORE PROCEEDING WITH FABRICATION.

Date _____ Issue Date _____
Drawn _____
Checked _____
Scale _____ AS NOTED
Job No. _____ Project Number _____

S000
ADDENDUM 5

FOUNDATIONS (cont'd)

8.

EXCAVATION, BACKFILL, AND COMPACTION SHALL BE DONE IN STRICT ACCORDANCE WITH GEOTECHNICAL ENGINEERING INVESTIGATION REPORT RECOMMENDATIONS.
9.

FOUNDATION EARTHWORK SHALL BE OBSERVED BY A QUALIFIED GEOTECHNICAL ENGINEER, OR THEIR ASSIGNEE RETAINED BY OWNER AND SATISFACTORY TO ARCHITECT (STRUCTURAL ENGINEER) AND GOVERNING CODE AUTHORITY. PERFORM REQUIRED OBSERVATIONS OF THIS CONTRACT AND CBC SECTION 1705.6.
10.

FOUNDATION EXCAVATION, BACKFILLING, AND COMPACTION SHALL BE OBSERVED AND APPROVED BY A GEOTECHNICAL ENGINEER AND THE GOVERNING AGENCY PRIOR TO PLACING REINFORCING STEEL AND CONCRETE. GEOTECHNICAL ENGINEER SHALL PROVIDE A LETTER OF COMPLIANCE TO THE OWNER.
11.

TEMPORARY CUT SLOPES SHALL NOT EXCEED THOSE RECOMMENDED IN THE GEOTECHNICAL ENGINEERING INVESTIGATION REPORT. DO NOT PERMIT ANY PERSON TO DESCEND INTO TRENCHES OR EXCAVATIONS GREATER THAN FIVE FEET IN DEPTH UNLESS NECESSARY PERMIT FROM STATE OF CALIFORNIA DIVISION OF INDUSTRIAL SAFETY IS OBTAINED PRIOR TO ISSUANCE OF BUILDING OR GRADING PERMIT. CONTRACTOR TO PROVIDE FOR DESIGN, PERMIT, AND INSTALLATION OF ALL SHORING AND SHEATHING NECESSARY TO SAFELY RETAIN EARTH BANKS.
12.

CONTRACTOR TO PROVIDE FOR DEWATERING OF EXCAVATIONS FROM SURFACE WATER, GROUND WATER OR SEEPAGE. DEWATERING SHALL EFFECTIVELY ELIMINATE ANY HYDROSTATIC PRESSURE ON SHORING. ENSURE THAT CONTAMINATED WATER IS NOT DISPOSED OF IN PUBLIC SEWER OR STORM DRAIN SYSTEM AND ENSURE THAT DIRTY WATER IS NOT DISPOSED OF INTO PUBLIC RIGHT-OF-WAY.
13.

UNLESS ADEQUATELY BRACED AND SHORED, RETAINING WALLS SHALL NOT BE BACKFILLED UNTIL WALLS HAVE ATTAINED FULL DESIGN STRENGTH. FOR PIT WALLS AND BUILDING WALLS BELOW GRADE, BRACING AND SHORING SHALL REMAIN IN PLACE UNTIL ATTACHED FLOORS ARE PLACED, CURED FOR AT LEAST 7 DAYS, AND HAVE ATTAINED FULL DESIGN STRENGTH. BACKFILL PLACED IMMEDIATELY BEHIND RETAINING WALLS SHALL BE COMPACTED WITH HAND OPERATED EQUIPMENT.
14.

SIDEWALKS OR PAVING IMMEDIATELY ADJACENT TO BUILDING PERIMETER SHALL BE SLOPED TO PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING. LANDSCAPE IRRIGATION IS NOT PERMITTED WITHIN FIVE FEET OF BUILDING PERIMETER FOOTINGS EXCEPT WHEN ENCLOSED IN PROTECTED PLANTERS THAT DIRECT DRAINAGE AWAY FROM STRUCTURE AND FOUNDATIONS. DISCHARGE FROM DOWNSPOUTS, ROOF DRAINS AND SCUPPERS IS NOT PERMITTED ONTO UNPROTECTED SOILS WITHIN FIVE FEET OF BUILDING PERIMETER.
15.

COMPACTION REPORT SHALL BE SUBMITTED TO THE BUILDING INSPECTOR PRIOR TO FOOTING INSPECTION.

REINFORCING STEEL

1.

REINFORCING STEEL SHALL BE PLACED IN ACCORDANCE TO AMERICAN CONCRETE INSTITUTE ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" AND CONCRETE REINFORCING STEEL INSTITUTE (CRSI) "MANUAL OF STANDARD PRACTICE".
2.

REINFORCING STEEL SHALL CONFORM TO ASTM A615/A615M, GRADE 60, UNLESS NOTE OTHERWISE. BARS TO BE WELDED SHALL CONFROM TO LOW ALLOY ASTM A706/A706M GRADE 60.
3.

DEFORMED LONGITUDINAL REINFORCEMENT RESISTING EARTHQUAKE-INDUCED MOMENT, AXIAL FORCE, OR BOTH, IN SPECIAL MOMENT FRAMES, SHALL COMPLY WITH ASTM A706/A706M, GRADE 60. ASTM A615/A615M GRADES 40 AND 60 REINFORCEMENT SHALL BE PERMITTED IN THESE MEMBERS IF:

A.

THE ACTUAL YIELD STRENGTH BASED ON MILL TESTS DOES NOT EXCEED THE SPECIFIED YIELD STRENGTH BY MORE THAN 18,000 PSI.

B.

FOR ASTM A615/A615M GRADE 40 REINFORCEMENT, THE RATIO OF THE ACTUAL TENSILE STRENGTH TO THE ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25.

C.

FOR ASTM A615/A615M GRADE 60 REINFORCEMENT, THE MINIMUM ELONGATION IN 8 INCHES SHALL BE AS FOLLOWS:

1.

NO. 3 THROUGH NO.6

= 14 PERCENT

2.

NO. 7 THROUGH NO. 11

= 12 PERCENT

3.

NO. 14 AND NO. 18

= 10 PERCENT

4.

WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A1064. WELDED WIRE REINFORCEMENT LAP SPLICE LENGTH MEASURED BETWEEN OUTERMOST CROSS WIRE OF EACH REINFORCEMENT SHEET. SHALL BE PER SCHEDULE BELOW (8 INCHES MINIMUM).
- | WELDED WIRE SIZE | LAP SPLICE LENGTH IN INCHES
(ACI 318-14 § 25.5.3) | | |
|------------------|--|------------------|------------------|
| | 3000 PSI (LT WT) | 4000 PSI (LT WT) | 5000 PSI (LT WT) |
| D4 - D6 | 12 | 12 | 12 |
| D7 - D9 | 16 | 14 | 13 |
| D10 - D12 | 24 | 21 | 19 |
5.

DEFORMED BAR ANCHORS SHALL BE NELSON STUD WELDING, INC. TYPE D2L (ICC EVALUATION SERVICE REPORT ESR-2907), OR AN APPROVED EQUAL, AND SHALL BE MADE FROM DEFORMED STEEL WIRE CONFORMING TO ASTM A1064, WITH A MINIMUM YIELD STRENGTH OF 70 KSI AND A MINIMUM TENSILE STRENGTH OF 80 KSI.

6.

LENTON FORM SAVER COUPLERS, FA OR FS SERIES, SHALL BE BY ERICO INTERNATIONAL CORPORATION (IAPMO UES EVALUATION REPORT NO. 0129) OR APPROVED EQUAL.

7.

HEADED DEFORMED BARS SHALL BE LENTON TERMINATORS BY ERICO INTERNATIONAL CORPORATION (IAPMO UES EVALUATION REPORT NO. 0188) OR APPROVED EQUAL. HEADED DEFORMED BARS MAY BE USED IN LIEU OF STANDARD HOOKS ONLY WHERE SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER.

8.

MECHANICAL COUPLERS FOR SPLICING REINFORCING BARS SHALL BE LENTON STANDARD COUPLERS, LENTON TRANSITION COUPLERS, SA OR FS SERIES, BY ERICO INTERNATIONAL CORPORATION (IAPMO UES EVALUATION REPORT NO. 0129) OR APPROVED EQUAL. MECHANICAL COUPLERS MAY BE USED IN LIEU OF LAP SPLICING REINFORCING BARS ONLY WHERE SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER.

9.

PREPARE REINFORCING STEEL SHOP DRAWINGS IN ACCORDANCE TO ACI 315, PART B. SHOP DRAWINGS MAY BE PREPARED MANUALLY OR BY COMPUTER. PLACING DRAWINGS SHALL BE PREPARED TO THE SAME STANDARD AS CONTRACT DRAWINGS. SHOW REINFORCING PLACEMENT, SPLICE LOCATIONS, REINFORCING LENGTHS, DETAILS, ELEVATIONS, BEND DETAILS, ETC. SUBMIT TO ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW PRIOR TO FABRICATION. PROMPTLY NOTIFY ARCHITECT (STRUCTURAL ENGINEER) PRIOR TO DEVELOPING REINFORCING STEEL SHOP DRAWINGS IF INSUFFICIENT CLEAR DISTANCES BETWEEN REINFORCING STEEL OR OTHER CONGESTION IS ENCOUNTERED. DEVIATIONS FROM THE CONTRACT DOCUMENTS SHALL BE CLEARLY IDENTIFIED ON THE SHOP DRAWINGS. IF SUBMITTAL IS PARTIAL, CLEARLY INDICATE ITEMS EXCLUDED FROM SUBMITTAL. SHOP DRAWINGS WILL BE REJECTED IF NOT PREPARED TO THE STANDARDS STATED ABOVE.
- GENERAL (cont'd)
14.

EQUIPMENT MANUFACTURER SHALL PROVIDE EQUIPMENT ANCHORAGE TO THE STRUCTURE MEETING THE REQUIREMENTS OF ASCE/SEI 7, CHAPTER 13.6. USE ISOLATORS, FASTENERS AND BRACING HAVING CURRENT ICC-ES OR IAPMO UES EVALUATION REPORT. EQUIPMENT ANCHORAGE SHALL BE CAPABLE OF TRANSMITTING CODE REQUIRED LATERAL LOADS BUT IN NO EVENT LESS THAN LATERAL LOAD EQUIVALENT TO 50 PERCENT OF THE OPERATING WEIGHT OF EQUIPMENT. SECURE SUSPENDED EQUIPMENT WITH LATERAL OR SWAY BRACING HAVING CURRENT ICC-ES OR IAPMO UES EVALUATION REPORT

15.

PIPING AND DUCTWORK BRACING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION (SMACNA) "SEISMIC RESTRAINT MANUAL - GUIDELINES FOR MECHANICAL SYSTEMS", INCLUDING ADDENDA.

16.

"TYPICAL DETAILS" ARE APPLICABLE THROUGHOUT CONSTRUCTION DOCUMENTS AND MAY NOT BE SPECIFICALLY REFERENCED THEREIN. CONTRACTOR IS RESPONSIBLE FOR IDENTIFYING THESE TYPICAL DETAILS AND UNDERSTANDING EXTENT OF THEIR APPLICATION PRIOR TO PERFORMING WORK.

17.

UNLESS SPECIFICALLY SHOWN ON THE PLANS NO STRUCTURAL MEMBER SHALL BE CUT, DRILLED OR NOTCHED WITHOUT PRIOR WRITTEN AUTHORIZATION FROM THE STRUCTURAL ENGINEER REVIEW AND APPROVAL.

18.

UNLESS NOTED ON STRUCTURAL DRAWINGS SEE ARCHITECTURAL DRAWINGS FOR INFORMATION NOTED BELOW :

A.

SIZE AND LOCATION OF DOOR AND WINDOW OPENINGS IN STRUCTURAL WALLS

B.

SIZE AND LOCATION OF FLOOR AND ROOF OPENINGS AND SLAB EDGES

C.

SIZE AND LOCATION OF NON-BEARING CMU WALLS AND OPENINGS THEREIN

D.

SIZE AND LOCATION OF CONCRETE CURBS, SLOPES, DEPRESSIONS, CHANGES IN LEVEL, CHAMFERS AND REVEALS, INSERTS FOR FINISH SYSTEMS

E.

EXTERIOR WALL SYSTEM AND LOCATION

F.

STAIR SIZE AND LOCATION, FRAMING AND DETAILS

G.

DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS

H.

ALL HOLES IN STRUCT WALLS NOT SPECIFICALLY SHOWN AND IDENTIFIED ON THE STRUCT DWGS. AND AFFECTING THE STRUCTURAL INTEGRITY OF THE WALL SHALL BE BROUGHT TO THE ATTENTION OF THE S.E.O.R. & FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION

19.

SEE MECHANICAL, ELECTRICAL, PLUMBING DRAWINGS FOR INFORMATION NOTED BELOW:

A.

SIZE AND LOCATION OF EQUIPMENT PADS, EQUIPMENT ANCHORAGE TO STRUCTURE, AND EQUIPMENT WEIGHTS

B.

ANCHORAGE OF DUCTWORK, PIPING, ELECTRICAL CONDUITS TO STRUCTURE

C.

ELECTRICAL CONDUIT RUNS, OUTLETS AND BOXES IN CONCRETE SLABS AND WALLS

D.

PIPE SLEEVES, TRENCHES, AND OPENINGS THROUGH WALLS AND SLABS FOR DUCTWORK, PIPE RUNS, ELECTRICAL CONDUIT RUNS

E.

ALL HOLES IN STRUCT WALLS NOT SPECIFICALLY SHOWN AND IDENTIFIED ON THE STRUCT DWGS. AND AFFECTING THE STRUCTURAL INTEGRITY OF THE WALL SHALL BE BROUGHT TO THE ATTENTION OF THE S.E.O.R. FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

20.

MECHANICAL, ELECTRICAL AND PLUMBING LOADS SHALL BE SUPPORTED FROM BEAMS. EXCEPTION: LIGHT MECHANICAL, ELECTRICAL AND PLUMBING LOADS MAYBE SUPPORTED BY CONCRETE-ON-METAL DECK ASSEMBLY, BUT MUST BE ANCHORED INTO STRUCTURAL CONCRETE SYSTEM BY A SYSTEM HAVING CURRENT ICC-ES OR IAPMO UES EVALUATION REPORT.

21.

NON-STRUCTURAL ITEMS, INCLUDING BUT NOT LIMITED TO, STAIR FRAMING, ARCHITECTURAL CLADDING, ETC., WHEN NOT DETAILED ON THE STRUCTURAL OR ARCHITECTURAL DRAWINGS, SHALL BE THE DESIGN RESPONSIBILITY OF THE CONTRACTOR. THESE NON-STRUCTURAL ITEMS MAY BE SUPPORTED BY THE PRIMARY STRUCTURE, BUT SHALL NOT IMPOSE TORSIONAL LOADS ONTO THE PRIMARY SUPPORT MEMBERS. PROVIDE BRACES, KICKERS, STIFFENERS, ETC., AS NECESSARY TO ELIMINATE TORSIONAL LOADS AT NO ADDITIONAL COSTS TO THE OWNER.
- FOUNDATIONS
1.

DESIGN OF FOUNDATION SYSTEM BASED ON RECOMMENDATIONS IN GEOTECHNICAL ENGINEERING INVESTIGATION REPORT BY ENGEO, INC. PROJECT NO. 6538.100312, DATED MAY 14, 2025, AND ALL SUBSEQUENT ADDENDA. GEOTECHNICAL REPORT AND ADDENDA SHALL BE CONSIDERED PART OF THESE CONTRACT DOCUMENTS AND SHALL BE KEPT AT JOB SITE AT ALL TIMES.

2.

FOUNDATION SYSTEM FOR THE STRUCTURE SHALL BE SPREAD FOOTING AND CONTINUOUS FOOTING.

3.

ISOLATED SPREAD FOOTING AND CONTINUOUS FOOTING DESIGN BASED ON ALLOWABLE NET BEARING PRESSURES OF 3000 PSF. BOTTOM OF ALL FOOTINGS SHALL BE A MINIMUM OF 18 INCHES BELOW LOWEST ADJACENT FLOOR OR GRADE. FOOTING DIMENSIONS SHALL NOT BE LESS THAN 18 INCHES FOR ISOLATED SPREAD FOOTINGS, AND 12 INCHES FOR CONTINUOUS STRIP FOOTINGS. ALLOWABLE BEARING PRESSURES CAN BE INCREASED 33 PERCENT FOR SEISMIC OR WIND LOADING.

4.

RESISTANCE TO LATERAL LOADS PROVIDED BY EITHER FRICTION AGAINST BASE OR BY PASSIVE EARTH PRESSURES. ALLOWABLE COEFFICIENT OF FRICTION IS 0.35 AND ALLOWABLE PASSIVE PRESSURE OF 350 PCF. WHERE BOTH THE FRICTION AND THE PASSIVE RESISTANCE ARE UTILIZED FOR SLIDING RESISTANCE. A ONE THIRD INCREASE IN THE FRICTION AND PASSIVE RESISTANCE VALUE MAY BE USED FOR WIND OR SEISMIC LOADS.

5.

LIGHT POLE FOUNDATION MAY BE DESIGNED TO IMPOSE AN ALLOWABLE LATERAL BEARING PRASSURE OF 350 PSF PER FOOT BELOW GRADE. LATERAL BEARING PREASSURE MAY BE INCREASED BY ONE-THIRD FOR SHORT-DURATION LOADING SUCH AS WIND OR SEISMIC LOADING.

6.

FOUNDATIONS MAY BE CAST DIRECTLY AGAINST EXCAVATIONS PROVIDED EXCAVATION IS CAPABLE OF MAINTAINING A VERTICAL CUT WITHOUT SLOUGHING. FOUNDATION DIMENSION SHALL BE ENLARGED BY AN ADDITIONAL ONE INCH IN THE DIRECTION OF THE SIDE CAST AGAINST EARTH.

7.

CONCRETE SHALL NOT BE PLACED ON FROZEN GRADE. IF FOOTING IS SUBJECT TO FREEZING TEMPERATURE AFTER FOUNDATION CONSTRUCTION, THEN FOOTING SHALL BE ADEQUATELY PROTECTED FROM FREEZING.
- GENERAL
1.

ALL WORK SHALL CONFORM TO THE STANDARDS OF LOS ANGELES COUNTY BUILDING CODE, 2023 EDITION, AND THOSE CODES AND STANDARDS LISTED IN THE CONTRACT DOCUMENTS.

2.

THE PROJECT MANUAL FORMS A PART OF THESE GENERAL NOTES. CODES, STANDARDS, AND SPECIFICATIONS, INCLUDING ADDENDA AND SUPPLEMENTS, REFERENCED IN THE CONTRACT DOCUMENTS SHALL BE THE LATEST APPROVED ISSUE, UNLESS SPECIFICALLY NOTED.

3.

NOTES AND DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. IF CONFLICT OCCURS BETWEEN THE CONTRACT DRAWINGS AND THE PROJECT MANUAL, IMMEDIATELY NOTIFY ARCHITECT (STRUCTURAL ENGINEER) FOR RESOLUTION. DIMENSIONS TAKE PRECEDENCE OVER SCALED DRAWINGS.

4.

DESIGN LOADS : REFER TO LOAD MAP PLANS, SHEETS S110.

DESIGN LIVE LOADS ARE NOTED AS FOLLOWS:

ROOF

= 20 PSF

MECHANICAL AREAS

= 50PSF (UNREDUCED)

5.

CODE LEVEL WIND DESIGN DATA:

BASIC WIND SPEED

= 105 MPH

EXPOSURE CATEGORY

= C

ENCLOSURE CLASSIFICATION

= ENCLOSED BUILDING

INTERNAL PRESSURE COEFFICIENT, GCpi

= ± 0.18

COMPONENTS AND CLADDING WIND PRESSURES, ASD (W/1.6).

	EFFECTIVE AREA	GENERAL (ZONE 4)	CORNER (ZONE 5)
WALL SURFACE	50 SF 100 SF	15.6 PSF 15.0 PSF	18.5 PSF 16.6 PSF
6.

CODE LEVEL EARTHQUAKE DESIGN DATA:

SITE COORDINATES

= 34.143°N, 118.6020°W

MAPPED SPECTRAL RESPONSE ACCELERATION, SS

= 2.263g

MAPPED SPECTRAL RESPONSE ACCELERATION, S1

= 0.817g

SITE CLASS

= C

DESIGN SPECTRAL RESPONSE COEFFICIENT, SDS

= 1.601g

DESIGN SPECTRAL RESPONSE COEFFICIENT, SD1

= 0.710g

RISK CATEGORY

= IV

IMPORTANCE FACTOR, I_e

= 1.5

SEISMIC DESIGN CATEGORY

= F

SEISMIC RESPONSE COEFFICIENT, CS

= 0.369

ANALYTICAL PROCEDURE:

=EQUIVALENT LATERAL FORCE PROCEDURE

SEISMIC-FORCE RESISTING SYSTEM

= LIGHT FRAME BEARING WOOD PANEL SHEAR WALLS

RESPONSE MODIFICATION FACTOR, R

= 6 1/2

DEFLECTION AMPLIFICATION FACTOR, Cd

= 4

OVERSTRENGTH FACTOR, Ω_o

= 3

DESIGN BASE SHEAR

MAIN BUILDING

= 154 KIP

RESERVE APPARATUS

= 26.5 KIP
7.

GOVERNING CODE AUTHORITY: COUNTY OF LOS ANGELES
8.

CONTRACT DOCUMENTS INDICATE INFORMATION SUFFICIENT TO CONVEY DESIGN INTENT. REVIEW CONTRACT DOCUMENTS AND VERIFY FIELD AND EXISTING CONDITIONS. PROMPTLY NOTIFY ARCHITECT (STRUCTURAL ENGINEER), PRIOR TO PROCEEDING WITH WORK, IF FURTHER CLARIFICATION OF DESIGN INTENT IS NEEDED.
9.

VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION AND NOTIFY THE ARCHITECT (STRUCTURAL ENGINEER) OF ANY DISCREPANCIES.
10.

PERFORM STRUCTURAL RELATED WORK AND DEVELOP SHOP DRAWINGS CONSIDERING CONTRACT DOCUMENTS IN THEIR ENTIRETY. CONDITIONS NOT SPECIFICALLY DETAILED SHALL BE CONSTRUCTED AS DETAILED FOR SIMILAR WORK.
11.

CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE MEANS AND METHODS OF CONSTRUCTION. PROVIDE ALL NECESSARY MEASURES TO PROTECT THE STRUCTURE DURING CONSTRUCTION. COMPLY WITH THE STATE OF CALIFORNIA, DIVISION OF OCCUPATIONAL SAFETY AND HEALTH REGULATIONS. CONSTRUCTION MATERIALS, IF PLACED ON FRAMED FLOORS AND ROOFS, SHALL BE SPREAD OUT SUCH THAT THE DESIGN LIVE LOAD PER SQUARE FOOT IS NOT EXCEEDED. PROVIDE ADEQUATE SHORING IF OVERLOAD IS ANTICIPATED OR WHERE STRUCTURAL ELEMENTS HAVE NOT ATTAINED DESIGN STRENGTH. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT CONSTITUTE ACCEPTANCE OF CONSTRUCTION MEANS AND METHODS.
12.

SUBMIT SHOP DRAWINGS FOR REVIEW BEFORE FABRICATION. CONTRACTOR SHALL REVIEW FOR COMPLETENESS AND COMPLIANCE WITH CONTRACT DOCUMENTS PRIOR TO SUBMISSION TO ARCHITECT (STRUCTURAL ENGINEER). ARCHITECT'S (STRUCTURAL ENGINEER'S) REVIEW IS FOR GENERAL CONFORMANCE WITH DESIGN INTENT AND DOES NOT CONSTITUTE AN AUTHORIZATION TO DEVIATE FROM TERMS AND CONDITIONS OF CONTRACT. WHEN INDICATED, THE SUBMITTAL SHALL BE SIGNED AND SEALED BY A PROFESSIONAL CIVIL OR STRUCTURAL ENGINEER LICENSED IN THE STATE OF CALIFORNIA. MAINTAIN AT SITE A COPY OF REVIEWED AND ACCEPTED SUBMITTALS.
13.

MODIFICATIONS AND SUBSTITUTIONS MUST BE ACCEPTED IN WRITING BY ARCHITECT (STRUCTURAL ENGINEER). NO MODIFICATION OR SUBSTITUTION WILL BE ACCEPTED VIA SHOP DRAWING REVIEW. MANUFACTURED MATERIALS SHALL BE APPROVED BY THE GOVERNING CODE AUTHORITY PRIOR TO THEIR USE. ADHERE TO ALL CONDITIONS OF THOSE APPROVALS. MODIFICATIONS AND SUBSTITUTIONS AFFECTING STRUCTURAL SAFETY, FIRE LIFE SAFETY OR ACCESSIBILITY ASPECTS OF THE PROJECT SHALL BE SUBMITTED TO THE ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW AND APPROVAL PRIOR TO FABRICATION/CONSTRUCTION.

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

so
structural engineers

726 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.sofabouquet.com
(213) 315-2277
Project #25534

GENERAL NOTES

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA



THE ABOVE DRAWINGS AND SPECIFICATIONS ARE THE SOLE DESIGN AND
ARRANGEMENTS REPRESENTED THE PROPERTY OF THE ARCHITECT AND NO PART SHALL BE COPIED,
DISCLOSED TO OTHERS OR USED IN CONNECTION WITH ANY WORK OR PROJECT
OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY HAVE BEEN PREPARED,
AND DEVELOPED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT. VERBAL
CONTACT WITH THESE DRAWINGS OR SPECIFICATIONS SHALL CONSTITUTE
CONCLUSIVE EVIDENCE OF ACCEPTANCE OF THE CONTRACTOR'S
WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR
SCALED DIMENSIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND CONDITIONS
SHOWN IN THESE DRAWINGS, SHOP DRAWINGS, AND FIELD CONDITIONS PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR
OFFICE FOR APPROVAL, BEFORE PROCEEDING WITH CONSTRUCTION.

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ADDENDUM 5
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CAST-IN-PLACE CONCRETE (CONTINUED)

- (4) "BEAM" STANDS FOR BEAM SOFFIT, JOISTS AND OTHER STRUCTURAL ELEMENTS THAT SUPPORT WEIGHT OF CONCRETE IN PLACE
- (5) "STRIPPING OF FORMWORK" OR "CRACKING OF SLAB" REFERS TO REMOVAL OF ANY PORTION OF THE FORMWORK SYSTEM SUPPORTING THE NEWLY CASTED CONCRETE MEMBER TEMPORARILY OR OTHERWISE.

CAST-IN-PLACE CONCRETE (CONTINUED)

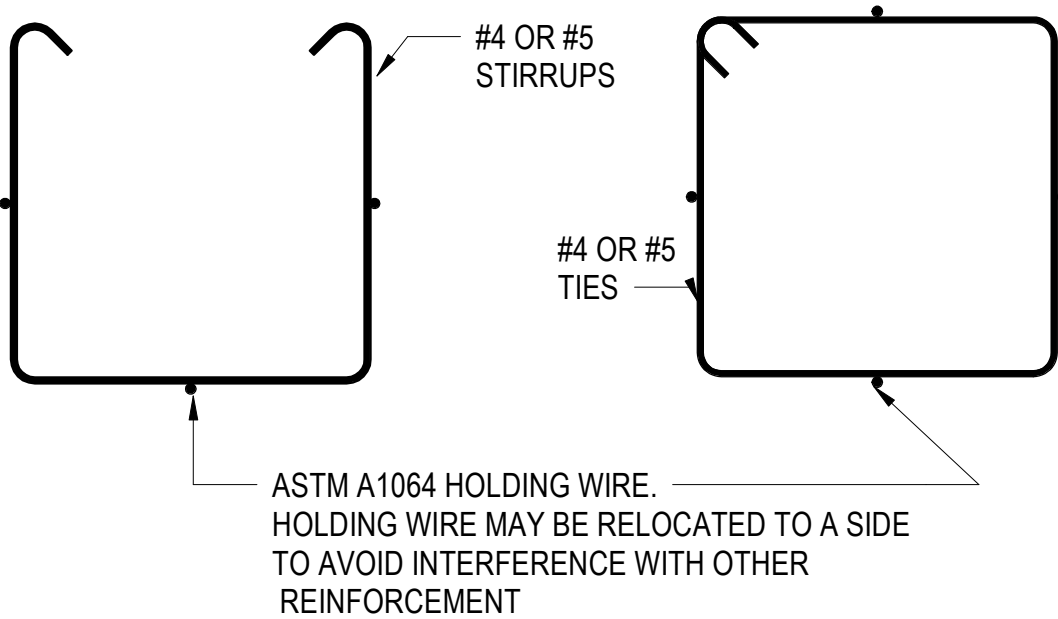
10. FOR CONCRETE SLABS-ON-GRADE PLACED DIRECTLY ON VAPOR BARRIER:
- A. CONCRETE MIXTURE:
1. USE INCREASED SIZE OF MAXIMUM-SIZE COARSE AGGREGATE (1" MINIMUM) AND COARSER SAND.
2. COARSE AGGREGATE TO BE WELL GRADED WITH MINIMUM FLAT OR ELONGATED PARTICLES.
3. REDUCE SAND CONTENT TO LOWEST LEVEL CONSISTENT WITH ADEQUATE WORKABILITY.
4. USE HIGH-RANGE WATER-REDUCING ADMIXTURE WITH GOOD SHRINKAGE-REDUCTION CHARACTERISTICS.
- B. FINISHING AND CURING:
1. USE PROPER FINISHING TECHNIQUES AND PROPER TIMING BETWEEN FINISHING OPERATIONS TO AVOID BLISTERING AND DELAMINATION.
2. USE CONTINUOUS MOIST CURE OR HIGH-SOLIDS CURING COMPOUND.
11. CONCRETE MIXING SHALL CONFORM TO ASTM C94.
12. SUBMIT SHOP DRAWINGS INDICATING LOCATIONS OF CONCRETE CONSTRUCTION JOINTS TO THE ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW AND APPROVAL PRIOR TO PLACING CONCRETE. LOCATE CONSTRUCTION JOINTS TO MINIMIZE EFFECTS OF SHRINKAGE AND AT POINTS OF LOW STRESS. HORIZONTAL CONSTRUCTION JOINTS ARE NOT PERMITTED IN BEAMS AND SLABS UNLESS SPECIFICALLY SHOWN ON THE DRAWINGS OR APPROVED BY THE ARCHITECT (STRUCTURAL ENGINEER) PRIOR TO CONSTRUCTION.
13. THE OUTSIDE DIAMETER OF ELECTRICAL CONDUITS EMBEDDED IN WALLS AND SLABS SHALL NOT EXCEED 1/3 THE OVERALL THICKNESS OF SLAB OR WALL IN WHICH THEY ARE EMBEDDED. LOCATE CONDUITS WITHIN THE MIDDLE THIRD OF SLABS OR WALLS AND NO CLOSER THAN 3 DIAMETERS ON CENTER WITH A CLEAR SPACING NOT LESS THAN 4 INCHES. CROSSING OF ELECTRICAL CONDUIT IS PERMITTED ONLY IF THE CROSSED CONDUITS REMAIN WITHIN THE MIDDLE THIRD OF THE SLAB OR WALL. EMBEDDED ELECTRICAL CONDUITS SHALL NOT BE LOCATED CLOSER TO COLUMNS THAN 1/6 THE CLEAR SLAB SPAN.
14. CONDUITS (JUNCTION BOXES) IN CONCRETE COLUMNS ARE NOT PERMITTED UNLESS SPECIFICALL APPROVED STRUCTURAL ENGINEER.
15. PROVIDE SLEEVES FOR ELECTRICAL AND PLUMBING OPENINGS. IF CONFLICT OCCURS BETWEEN REINFORCING AND SLEEVES, REPOSITION REINFORCING OR SLEEVES OR BOTH. DO NOT CUT ANY REINFORCING. CORING IS NOT PERMITTED UNLESS APPROVED IN WRITING BY THE ARCHITECT (STRUCTURAL ENGINEER).
16. SUBMIT COMPOSITE SHOP DRAWINGS INDICATING SIZE AND LOCATION OF ALL EMBEDDED ELECTRICAL CONDUITS, SIZE AND LOCATION OF ALL ELECTRICAL AND PLUMBING SLEEVES, AND SIZE AND LOCATION OF ALL MECHANICAL DUCT OPENINGS.
17. PRIOR TO PLACING CONCRETE, REINFORCING BARS (INCLUDING WELDED WIRE REINFORCEMENT), EMBEDDED PLATES, ANCHOR BOLTS, AND OTHER CONCRETE EMBEDMENTS SHALL BE WELL SECURED IN POSITION.
18. CONCRETE PLACEMENT SHALL CONFORM TO ACI 304 AND CONTRACT DOCUMENTS. INTENTIONALLY ROUGHEN ALL PREVIOUSLY HARDENED CONCRETE SURFACES TO A FULL AMPLITUDE OF 1/4-INCH AGAINST WHICH FRESH CONCRETE IS PLACE.
19. PROVIDE KEYED CONSTRUCTION JOINT WHERE INDICATED ON DRAWINGS. CLEAN, REMOVE LAITANCE, THOROUGHLY WET, AND REMOVE STANDING WATER IMMEDIATELY BEFORE PLACING FRESH CONCRETE.
20. FORM EXPOSED CORNERS OF COLUMNS, BEAMS AND WALLS WITH A 3/4-INCH CHAMFER, UNLESS NOTED OTHERWISE ON ARCHITECTURAL DRAWINGS.
21. AT LEAST TWO HOURS MUST ELAPSE BETWEEN THE END OF COLUMN OR WALL PLACEMENT AND THE BEGINNING OF SLAB PLACEMENT.
22. CONCRETE SHALL BE MAINTAINED ABOVE 50 DEGREES FAHRENHEIT AND IN A MOIST CONDITION FOR A MINIMUM OF 7 DAYS AFTER PLACEMENT UNLESS OTHERWISE ACCEPTED BY ARCHITECT (STRUCTURAL ENGINEER).
23. CURING COMPOUNDS, SEALERS, HARDENERS, ETC., USED ON CONCRETE THAT RECEIVES A FINISH SHALL BE APPROVED BY THE ARCHITECT BEFORE USE.
24. GROUT SHALL BE NON-SHRINK, NON-METALLIC, SHALL NOT CONTAIN CHLORIDES, AND SHALL ATTAIN A 28-DAY COMPRESSIVE STRENGTH OF 6,000 PSI.
25. LEAN CONCRETE SHALL CONTAIN 2 SACKS OF CEMENT PER CUBIC YARD OF CONCRETE. USE ONLY WHERE SPECIFICALLY INDICATED.
26. THE MINIMUM CONCRETE PROPERTIES PRIOR TO INITIATING VARIOUS CONSTRUCTION STAGES SHALL CONFORM WITH THE TABLE BELOW:

	COSTRUCTION STAGE	SPECIFIED $f_c^{(2)}$	MOE $^{(3)}$
RC SLABS AND BEAMS	STRIPPING OF FORMWORK (CRACKING SLAB) ⁽⁵⁾	SLAB: 0.85 f_c BEAM: 0.90 $f_c^{(4)}$	90%

- (1) REQUIREMENTS ARE SUMMARIZED FROM THE STRUCTURAL SPECIFICATIONS. THESE PROPERTIES/REQUIREMENTS APPLY TO THE IN-FIELD PROPERTIES. OUR EXPERIENCE HAS SHOWN THAT TRIAL-BATCH/LAB TESTS CAN BE 10-15% HIGHER THAN TESTS PERFORMED ON SAMPLES COLLECTED FROM THE FIELD DURING CONSTRUCTION. THIS REDUCTION SHOULD BE ACCOUNTED FOR WHILE QUALIFICATION/APPROVAL OF THE MIX DESIGN PRIOR TO CONSTRUCTION. REFER TO NOTES (2) AND (3) FOR REQUIREMENTS DURING CONSTRUCTION.
- (2) STRENGTH OF CONCRETE AS SPECIFIED IN THE STRUCTURAL DRAWINGS. DURING CONSTRUCTION, COMPLIANCE WITH COMPRESSIVE STRENGTH REQUIREMENTS SHOULD BE ENSURED PRIOR TO BEGINNING ANY WORK BY MEANS OF TESTING CYLINDERS TAKEN FROM THE CONCRETE BATCH DELIVERED AT SITE. REFER TO SPECS FOR FIELD TESTING REQUIREMENTS.
- (3) REFER TO STRUCTURAL SPECIFICATIONS FOR THE TARGET MOE FOR ANY GIVEN CONCRETE STRENGTH. DURING CONSTRUCTION, MOE RESULTS FROM CYLINDERS TAKEN FROM FIELD IS NOT REQUIRED PRIOR TO BEGINNING THE WORK. HOWEVER, RANDOM FIELD TESTS OF LIMITED SAMPLES COLLECTED DURING CONSTRUCTION ARE REQUIRED IF STATED IN THE SPECIFICATIONS TO VERIFY THAT ACTUAL MOE IN THE FIELD IS WITHIN 15% OF THE TRIAL BATCH RESULTS SUBMITTED FOR THE MIX DESIGN APPROVAL.

FUSION WELDING (cont'd)

3. FUSION WELD WOULD BE USED FOR ALL BEAM AND COLUMN STIRRUPS AND TIES WHICH CONSTITUTES #4 AND #5 BARS. DETAILS OF HOLDING WIRE CONNECTION TO BEAM STIRRUP AND TIES IS SHOWN BELOW.
4. HOLDING WIRES SHALL CONFORM TO ASTM A1064.
5. ALL REINFORCING STEEL TO BE WELDED SHALL COMPLY WITH ASTM A706.
6. THE FUSION WELDS SHALL BE MADE BY MACHINES USING ELECTRIC RESISTANCE WELDS. TACK WELDING OF PRIMARY REINFORCING BARS TOGETHER OR TO STIRRUPS / TIES IS NOT PERMITTED.
7. FABRICATION OF PRE-ASSEMBLED STEEL CAGES SHALL BE IN A SHOP OF A FABRICATOR LICENSED BY THE COUNTTY OF LOS ANGELES BUILDING DEPARTMENT, IN ACCORDANCE WITH THE MANUFACTURING STANDARDS SUBMITTED TO THE BUILDING DEPARTMENT.
8. HOLDING WIRES SHALL CONFORM TO ASTM A1064.
9. ALL REINFORCING STEEL TO BE WELDED SHALL COMPLY WITH ASTM A706.
10. THE FUSION WELDS SHALL BE MADE BY MACHINES USING ELECTRIC RESISTANCE WELDS. TACK WELDING OF PRIMARY REINFORCING BARS TOGETHER OR TO STIRRUPS / TIES IS NOT PERMITTED.
11. FABRICATION OF PRE-ASSEMBLED STEEL CAGES SHALL BE IN A SHOP OF A FABRICATOR LICENSED BY THE COUNTY OF LOS ANGELES BUILDING DEPARTMENT, IN ACCORDANCE WITH THE MANUFACTURING STANDARDS SUBMITTED TO THE BUILDING DEPARTMENT.
12. QUALITY CONTROL TEST SHALL BE PERFORMED ON SHOP-WELDED SPECIMENS BY THE FABRICATOR. REINFORCING STEEL SPECIMENS CONTAINING THE HOLDING WIRE SHALL BE TESTED FOR YIELD AND TENSILE STRENGTH AT THE FREQUENCY BY SECTION 1910.2 OF CBC, TEST REPORTS SHALL BE AVAILABLE ON REQUEST TO THE APPROVED AGENCY, DESIGNED PROFESSIONAL AND ENFORCEMENT AGENCY.



CAST-IN-PLACE CONCRETE

1. ALL CONCRETE WORK SHALL CONFORM TO THE STANDARDS OF THE AMERICAN CONCRETE INSTITUTE, ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE" AND ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", WITH MODIFICATIONS AS NOTED IN THE CONTRACT DOCUMENTS.
2. CONCRETE SHALL ATTAIN THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28-DAY (f_c), UNLESS NOTED OTHERWISE:
- | | | |
|---------------------------------------|---------------|---------------|
| CONTINUOUS FOOTINGS | 3,000 PSI | NORMAL WEIGHT |
| SPREAD FOOTINGS | 3,000 PSI UNO | NORMAL WEIGHT |
| SPREAD FOOTINGS UNDER CONCRETE FRAMES | 5,000 PSI | NORMAL WEIGHT |
| SLABS ON GRADE | 3,000 PSI | NORMAL WEIGHT |
| GRADE BEAMS | 5,000 PSI | NORMAL WEIGHT |
| ALL OTHER CONCRETE | 3,000 PSI | NORMAL WEIGHT |
| COLUMNS | 5,000 PSI | NORMAL WEIGHT |
| BEAMS | 5,000 PSI | NORMAL WEIGHT |
3. UNLESS NOTED OTHERWISE HEREIN, CONCRETE IS ASSIGNED TO EXPOSURE CLASSES F0, S0, W0, AND C0, AS DEFINED IN TABLE 19.3.1.1 OF ACI 318.
- A. CONCRETE IN CONTACT WITH SITE SOIL SHALL BE ASSIGNED TO EXPOSURE CLASS S2.
4. PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I OR TYPE II. PORTLAND CEMENT FOR CONCRETE IN EXPOSURE CLASS S2 SHALL CONFORM TO ASTM C150, TYPE V (OR OTHER TYPES OF PORTLAND CEMENT WITH C3A CONTENT LESS THAN 5 PERCENT).
5. AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C33. NORMAL WEIGHT CONCRETE SHALL HAVE A MINIMUM DRY UNIT WEIGHT OF 145 PCF AND A MAXIMUM DRY UNIT WEIGHT OF 155 PCF.
6. MAXIMUM AGGREGATE SIZE SHALL BE 1-1/2 INCHES FOR FOUNDATIONS AND 1 INCH ELSEWHERE, BUT NO LARGER THAN (A) 1/5 THE NARROWEST DIMENSION BETWEEN SIDES OF FORMS, (B) 1/3 THE DEPTH OF SLABS, OR (C) 3/4 THE MINIMUM CLEAR SPACING BETWEEN INDIVIDUAL REINFORCING BARS OR WIRES, BUNDLES OF BARS, INDIVIDUAL TENDONS, BUNDLED TENDONS, OR DUCTS. SMALLER AGGREGATE SIZES MAY BE ALLOWED WITH THE APPROVAL OF THE ARCHITECT (STRUCTURAL ENGINEER).
7. MAXIMUM SLUMP SHALL BE 5 INCHES TYPICALLY AND 4 INCHES IN FLATWORK, UNLESS A HIGH-RANGE WATER REDUCING ADMIXTURE (SUPERPLASTICIZER) IS USED IN THE CONCRETE MIX PROPORTIONS.
8. CONCRETE SHRINKAGE SHALL BE LIMITED TO 0.05 PERCENT AT 35 DAYS AS DETERMINED BY ASTM C157. TEST SPECIMENS SHALL BE MOIST CURED IN LIME SATURATED WATER FOR 28 DAYS AND AIR STORED FOR 7 DAYS.
9. WATER CEMENT RATIO SHALL NOT EXCEED 0.45 FOR ALL FLATWORK THAT RECEIVES A MOISTURE SENSITIVE ADHESIVE TO AFFIX FLOOR FINISHES AND 0.50 ELSEWHERE. EXCEPTION: WATER CEMENT RATIO FOR CONCRETE IN EXPOSURE CLASS S2 SHALL NOT EXCEED 0.45 AND f_c NOT LESS THAN 4500 PSI.

REINFORCING STEEL (cont'd)

10. REINFORCING STEEL SHALL BE SPLICED AS SHOWN ON THE DRAWINGS. IF NOT SHOWN, LOCATE SPLICES IN AREAS OF MINIMUM STRESS. LAP (SPLICE) LENGTHS ARE AS INDICATED ON THE DRAWINGS.
11. MINIMUM CLEARANCES BETWEEN PARALLEL REINFORCING STEEL, INCLUDING SPLICED BARS, SHALL BE AS FOLLOWS:
- A. FOR BARS PLACED IN A HORIZONTAL LAYER, CLEAR SPACING SHALL BE 1 INCH, 1 BAR DIAMETER, OR 4/3 TIMES THE MAXIMUM SIZE AGGREGATE, WHICHEVER IS GREATER.
- B. FOR BARS PLACED IN TWO OR MORE HORIZONTAL LAYERS, BARS IN UPPER LAYERS SHALL BE PLACED DIRECTLY ABOVE BARS IN THE BOTTOM LAYER WITH A CLEAR SPACING BETWEEN LAYERS OF AT LEAST 1 INCH.
- C. FOR LONGITUDINAL BARS IN COLUMNS, PEDESTALS, STRUTS, AND BOUNDARY ELEMENTS IN SHEAR WALLS, CLEAR SPACING SHALL BE 1 1/2 INCHES, 1 1/2 BAR DIAMETERS, OR 4/3 TIMES THE MAXIMUM SIZE AGGREGATE, WHICHEVER IS GREATER.
- D. FOR BUNDLED BARS, MINIMUM CLEAR DISTANCES BETWEEN UNITS OF BUNDLED BARS SHALL BE SAME AS SINGLE BARS EXCEPT BAR DIAMETER IS DERIVED FROM EQUIVALENT TOTAL AREA OF BUNDLE.
12. PROVIDE THE FOLLOWING CONCRETE COVERAGE FOR REINFORCING STEEL PLACED IN CAST-IN-PLACE CONCRETE:
- A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH.....3"
- B. CONCRETE EXPOSED TO EARTH OR WEATHER:
- NO. 6 THROUGH NO. 18 BARS.....2"
- NO. 5 BARS, W31 OR D31 WIRE, AND SMALLER.....1-1/2"
- C. CONCRETE NOT EXPOSED WEATHER OR IN CONTACT WITH GROUND:
- SLAB, WALLS, JOISTS:
- NO. 14 AND NO. 18 BARS.....1-1/2"
- NO. 11 BARS AND SMALLER (*).....1"
- BEAMS AND COLUMNS
- PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS.....1-1/2"
- D. SLAB-ON-GRADE.....MID-HEIGHT OF SLAB
- (*) CONCRETE COVERAGE ADEQUATE FOR FIRE-RESISTIVE PERIOD OF 2 HOURS.
13. WALL AND COLUMN DOWELS SHALL MATCH SIZE, GRADE, AND SPACING OF RESPECTIVE VERTICAL REINFORCING, UNLESS OTHERWISE NOTED.
14. USE PLASTIC OR PLASTIC COATED SPACERS AND CHAIRS IF RESTING ON EXPOSED CONCRETE SURFACES.
15. WELDING OF REINFORCING STEEL SHALL BE MADE WITH LOW HYDROGEN ELECTRODES IN CONFORMANCE WITH AMERICAN WELD SOCIETY AWS D1.4 "STRUCTURAL WELDING CODE - REINFORCING STEEL".
- A. EXCEPT FOR REINFORCING STEEL CONFORMING TO ASTM A706/A706M, DETERMINE CARBON EQUIVALENT OF ALL REINFORCING STEEL TO BE WELDED. SUBMIT WPS FOR ALL REINFORCING STEEL TO BE WELDED TO ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW AND TO GOVERNING CODE AUTHORITY FOR APPROVAL PRIOR TO EXECUTION. WPS SHALL INCLUDE:
1. SKETCH OF JOINT DESCRIBING GEOMETRY AND APPLICABLE DIMENSIONS, WELD TYPE AND SIZE, SEQUENCE OF WELD DEPOSITION, AND MAXIMUM LAYER THICKNESS AND BEAD WIDTHS.
2. APPLICABLE WELD PROCESS.
3. FILLER METAL PER AWS STANDARD AND ELECTRODE SPECIFICATION AND CLASSIFICATION, AS WELL AS DETAILS OF SHIELDING MATERIAL.
4. ELECTRICAL CHARACTERISTICS FOR WELD PROCESS USED SUCH AS TYPE OF CURRENT AND ACCEPTABLE RANGE OF CURRENT MEASURED IN AMPERAGE, VOLTAGE RANGE, AND ELECTRODE DIAMETER. FOR WELD FEED PROCESS, INDICATE MANUFACTURER RECOMMENDED WIRE SPEED, MELT OFF RATE AND DEPOSITION RATE.
5. PREHEAT TEMPERATURES.
6. PROCEDURE QUALIFICATION RECORDS (PQR) FOR ALL WPS'S QUALIFIED BY TESTING.
- B. WELDERS SHALL BE CERTIFIED TO CONFORM WITH AWS STANDARDS AND APPROVED BY THE GOVERNING CODE AUTHORITY.
16. REINFORCING STEEL BENDS SHALL BE MADE COLD. RE-BENDING OF PREVIOUSLY BENT REINFORCING IS NOT PERMITTED. FIELD BENDING OF REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE PERMITTED UNLESS SPECIFICALLY NOTED ON DRAWINGS.
17. ALL REINFORCING STEEL, INCLUDING WELDED WIRE REINFORCING, SHALL BE SECURELY HELD IN PLACE WHILE PLACING CONCRETE. IF REQUIRED, ADDITIONAL BARS, CONCRETE BLOCKS, CHAIRS, BOLSTERS, ETC., SHALL BE PROVIDED BY THE CONTRACTOR TO FURNISHS SUPPORT FOR ALL REINFORCING. HOOKING AND WALKING-ON IS NOT PERMITTED.
18. ALL REINFORCING STEEL SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN FINAL INSPECTION IS CONDUCTED.
19. CONTACTOR SHALL PROVIDE FOR AN ALLOWANCE OF 5 TONS OR 2%, WHICHEVER IS GREATER, OF REINFORCING STEEL TO BE FABRICATED AND/OR PLACED DURING THE PROGRESS OF WORK AS MAY BE DIRECTED BY THE ARCHITECT (STRUCTURAL ENGINEER). THE UNUSED PORTION SHALL BE CREDITED TO THE OWNER AT THE COMPLETION OF CONCRETE WORK.

FUSION WELDING

- IN-PLANT FUSION WELDING IS ALLOWED UNDER THE FOLLOWING CONDITIONS AND BASED ON LARR 25912 REPORT. REQUIREMENTS ON CBC 2022 SECTION 1903.8:
1. FUSION WELDING OF HOLDING WIRES TO TIES, STIRRUPS, AND HOOPS IN BEAMS, COLUMNS AND GRADE BEAMS TO PREASSEMBLE REINFORCING STEEL CAGES. FUSION WELDING IS NOT ALLOWED TO LONGITUDINAL REINFORCING STEEL IN ANY BEAM, COLUMN, OR GRADE BEAM. THE HOLDING WIRE AREA SHALL NOT EXCEED 5% OF THE BEAM, COLUMN, OR GRADE BEAM CROSS SECTIONAL LONGITUDINAL STEEL AREA.
2. FUSION WELDING OF HOLDING WIRES SHALL NOT OCCUR ON ANY PORTION OF A REINFORCING BAR THAT IS OR WILL BE BENT IN ACCORDANCE WITH ACI 318-14 SECTION 25.3.

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

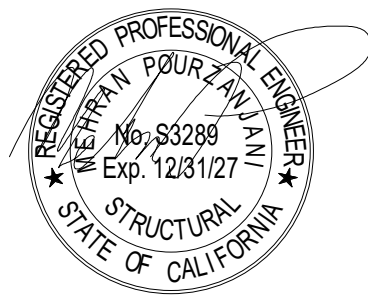
SO
saifut-bouquet
structural engineers

726 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.saifutbouquet.com
(213) 315-2277
Project #25534

GENERAL NOTES

FIRE STATION 46

MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA



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REPRODUCED, OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC
OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION
SYSTEM, WITHOUT THE WRITTEN PERMISSION OF THE ARCHITECT. THE ARCHITECT
CONTACT WITH THESE DRAWINGS OR SPECIFICATIONS SHALL CONSTITUTE
CONCLUSIVE EVIDENCE OF ACCEPTANCE OF THE DESIGN AND SPECIFICATIONS.
WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL TAKE PRECEDENCE OVER
SCALED DIMENSIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING
FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THE OFFICE MUST
BE NOTIFIED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS
SHOWN IN THESE DRAWINGS. SHOP DRAWINGS SHALL BE SUBMITTED TO THE
OFFICE FOR APPROVAL, BEFORE PROCEEDING WITH FABRICATION.
2025

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ADDENDUM 5

METAL DECKING

1. METAL DECKING SHALL BE AS MANUFACTURED BY VERCO DECKING, INC. (IAPMO UES EVALUATION REPORT NO. 0218) AND SHALL BE OF GAGE AND PROFILE AS INDICATED ON THE DRAWINGS. METAL DECK OF EQUAL QUALITY AND WITH CURRENT ICC-ES EVALUATION REPORT OR IAPMO UES EVALUATION REPORT MAY BE SUBSTITUTED IF APPROVED BY THE ARCHITECT (STRUCTURAL ENGINEER).
2. METAL DECKING AND ACCESSORIES SHALL BE COLD-FORMED FROM GALVANIZED STEEL SHEETS COMPLYING WITH ASTM A653-SS OR ASTM A1063-SS GRADE 50 (MINIMUM), WITH COATING DESIGNATION G60. GALVANIZING SHALL BE BY THE HOT-DIP PROCESS COMPLYING WITH ASTM A924.
3. MINIMUM BEARING OF METAL DECKING ON SUPPORTS SHALL BE 2 INCHES. ATTACH METAL DECKING TO SUPPORTING STEEL MEMBERS BY WELDING AS SPECIFIED ON THE DRAWINGS AND AS RECOMMENDED BY DECK MANUFACTURER USING E60 OR E70 ELECTRODES. DECK WELDING SHALL BE IN COMPLIANCE WITH ANSI/AWS D1.3. WELDERS SHALL BE AWS CERTIFIED AS REQUIRED BY THE GOVERNING CODE AUTHORITY.
4. HOURLY FIRE RESISTIVE REQUIREMENTS FOR FLOOR AND ROOF DECKS SHALL BE DETERMINED USING CBC TABLE 601. BUILDING TYPES OF CONSTRUCTION AND FIREPROOFING MATERIALS ARE AS INDICATED ON THE ARCHITECTURAL DRAWINGS.

METAL DECKING (CONTINUED)

5. ROOF DECKING:

A. FOR METAL DECK ONLY OR METAL DECK WITH INSULATING CONCRETE FILL:

1. DO NOT SUSPEND PIPING OVER 1-1/2" DIAMETER (40 LBS MAX), DUCTS LARGER THAN 12" X 16" OR EQUIVALENT PERIMETER (40 LBS MAX), OR OTHER LOADS WITH EXCEPTION OF SUSPENDED ACOUSTICAL CEILINGS AND INTEGRALLY SUPPORTED LIGHT FIXTURES FROM ROOF DECKING. HANGERS TO ROOF DECKING SHALL PENETRATE DECK AND BE ATTACHED TO A 1/2" DIAMETER BY 1'-0" LONG RODS LAID IN AND PARALLEL TO THE BOTTOM OF THE LOW DECK FLUTES. HANGERS SHALL BE TWO FLUTES APART IF THEY OCCUR ON THE SAME SPAN.
6. AT COMPLETION OF METAL DECK ERECTION, ALL WELDS IN EXPOSED AREAS SHALL BE DE-SLAGGED, CLEAND AND PRIMED WITH A ZINC RICH PRIMER.
7. SUBMIT COMPLETE METAL DECKING SHOP DRAWINGS TO ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW PRIOR TO FABRICATION.

ARCHITECTURAL CLADDING OR FENESTRATION (DESIGN-BUILD DEFERRED APPROVAL)

1. ALL ARCHITECTURAL CLADDING,INCLUDING CURTAIN WALLS, WINDOW WALLS, AND STOREFRONTS, SHALL BE DESIGN-BUILD BY CONTRACTOR. ARCHITECTURAL CLADDING SYSTEMS ARE NOT SPECIFIED ON THE STRUCTURAL DRAWINGS AND DO NOT FORM A PART OF THE PRIMARY STRUCTURAL SYSTEM. REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR ALL INFORMATION PERTAINING TO ARCHITECTURAL CLADDING SYSTEMS.
2. CONTRACT DOCUMENTS INDICATE DESIGN INTENT, INCLUDING GENERAL ARRANGEMENT, EXPECTED PROFILES, AND SUPPORT POINTS. INDICATIONS OF SPECIFIC CLADDING OR FENESTRATION ELEMENTS AND THEIR CONNECTIONS TO STRUCTURE, WHERE SHOWN, ARE INTENDED SOLELY TO SHOW INTENDED LOAD PATH AND SUPPORT POINTS. WHERE THE INDICATED SUPPORT POINTS ARE NOT ADEQUATE, PROVIDE ADDITIONAL SUPPORTS, CONNECTIONS, BRACES OR OTHER ELEMENTS AS REQUIRED AT NO ADDITIONAL COST TO OWNER. ADVISE ARCHITECT (STRUCTURAL ENGINEER) OF ANY CHANGES THAT AFFECT THE DESIGN INTENT, SUCH AS CLADDING OR FENESTRATION PROFILE OR INTENDED LOAD PATH, PRIOR TO DEVELOPING SHOP DRAWINGS FOR THE WORK.
3. DESIGN CLADDING OR FENESTRATION MEMBERS, CONNECTIONS, AND SUPPORTS TO STRUCTURE FOR ALL LOADS, FIRE RESISTIVE REQUIREMENTS, AND OTHER CONDITIONS OF GOVERNING CODE AND CONTRACT DOCUMENTS.
4. DESIGN CLADDING OR FENESTRATION MEMBERS, CONNECTIONS, AND SUPPORTS TO STRUCTURE TO ALLOW FOR ADEQUATE THERMAL EXPANSION AND CONTRACTION, AND PROVIDE ALLOWANCE FOR DIFFERENTIAL GRAVITY AND SEISMIC DEFORMATIONS BETWEEN SUPPORTING STRUCTURE AND CLADDING OR FENESTRATION IN ACCORDANCE WITH THE CONTRACT DOCUMENTS AND GOVERNING CODE. WHERE STRUCTURAL DEFORMATIONS ARE NOT SHOWN, ASSUME THE FOLLOWING VALUES FOR BID PURPOSES, AND OBTAIN ACCURATE ESTIMATE FROM ARCHITECT (STRUCTURAL ENGINEER) PRIOR TO DESIGN:

A. STORY DRIFT, DELTA (Δ) = 1.5% OF STORY HEIGHT

B. LIVE LOAD + SUPERIMPOSED DEAD LOAD VERTICAL DEFLECTION = 3/4 INCH.
5. SUBMIT SHOP DRAWINGS AND STRUCTURAL CALCULATIONS SIGNED AND SEALED BY A STRUCTURAL ENGINEER, LICENSED IN THE STATE OF CALIFORNIA, TO ARCHITECT OF RECORD (STRUCTURAL ENGINEER) FOR REVIEW AND GOVERNING CODE AUTHORITY FOR APPROVAL.
6. CONNECTIONS TO STRUCTURE SHALL NOT IMPOSE ECCENTRIC OR TWISTING LOADS ON STRUCTURAL SUPPORT MEMBERS. WHERE ECCENTRIC CONNECTIONS ARE NECESSARY, PROVIDE ADDITIONAL BRACES, STIFFENER PLATES, OR OTHER SUPPORTS TO STABILIZE STRUCTURAL MEMBERS AT NO ADDITIONAL COST TO OWNER.
7. THE DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD WHO SHALL REVIEW AND APPROVE THEM AND FOWARD THEM TO THE BUILDING OFFICIAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND APPROVED AND THAT THEY HAVE BEEN FOUND TO BE IN GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL. PROVIDE AMPLE TIME FOR THE BUILDING OFFICIAL TO REVIEW THE DOCUMENTS.

STRUCTURAL STEEL (CONTINUED)

13. SUBMIT TO ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW A WRITTEN WELDING PROCEDURE SPECIFICATION (WPS) FOR ALL WELDS USED ON PROJECT PRIOR TO FABRICATION. FOR WELDS NOT PREQUALIFIED, THE SUPPORTING PROCEDURE QUALIFICATION RECORD (PQR) SHALL ALSO BE SUBMITTED WITH THE WPS. WPS SHALL BE IN ACCORDANCE TO AWS D1.1/D1.1M, SECTION 4.6 AND SHALL INCLUDE THE FOLLOWING INFORMATION FOR EACH WELD TYPE AND POSITION:

A. SKETCH OF JOINT DESCRIBING GEOMETRY AND APPLICABLE DIMENSIONS, WELD TYPE AND SIZE, SEQUENCE OF WELD DEPOSITION, AND MAXIMUM LAYER THICKNESS AND BEAD WIDTHS. LAYER THICKNESS SHALL NOT EXCEED 1/4 INCH, AND BEAD WIDTH SHALL NOT EXCEED 5/8 INCH.

B. BASE METAL TYPES AND THICKNESS.

C. APPLICABLE WELD PROCESS (SMAW OR FCAW).

D. FILLER METAL PER AWS STANDARD AND ELECTRODE SPECIFICATION AND CLASSIFICATION, AS WELL AS DETAILS OF SHIELDING MATERIAL.

E. ELECTRICAL CHARACTERISTICS FOR WELD PROCESS USED SUCH AS TYPE OF CURRENT AND ACCEPTABLE RANGE OF CURRENT MEASURED IN AMPERAGE, VOLTAGE RANGE, AND ELECTRODE DIAMETER. FOR WELD FEED PROCESS, INDICATE MANUFACTURER RECOMMENDED WIRE SPEED, CONTACT DISTANCE, MELT OFF RATE AND DEPOSITION RATE.

F. A COPY OF ELECTRODE MANUFACTURER'S TECHNICAL INFORMATION AND CERTIFICATE OF CONFORMANCE.

14. TESTING LABORATORY WILL VERIFY COMPLIANCE WITH ACCEPTED WPS AND WILL PROMPTLY NOTIFY ARCHITECT (STRUCTURAL ENGINEER) IF DEVIATIONS ARE FOUND.

15. ELECTRODE DIAMETER SHALL NOT EXCEED PREQUALIFIED LIMITS SHOWN IN AWS D1.1/ D1.1M TABLE 3.6, AS APPLICABLE. FOR FCAW PROCESS, MAXIMUM ELECTRODE SIZE SHALL NOT EXCEED 1/8 INCH.

16. DIFFUSIBLE HYDROGEN LEVEL FOR ELECTRODES AND ELECTRODE-FLUX COMBINATION SHALL MEET THE REQUIREMENTS OF TABLE 6.3 OF AWS D1.8/D1.8M.

17. DETAILS, MATERIALS, WORKMANSHIP, AND TESTING AND INSPECTION REQUIREMENTS OF WELDED JOINTS COMPRISING THE SFRS SHALL CONFORM TO THE FOLLOWING APPLICABLE STANDARDS:

A. AWS D1.1/ D1.1M "STRUCTURAL WELDING CODE - STEEL."

B. AWS D1.8/ D1.8M "STRUCTURAL WELDING CODE - SEISMIC SUPPLEMENT."

C. ANSI/AISC 341, "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS", CHAPTER J (QUALITY CONTROL AND QUALITY ASSURANCE).

D. ANSI/AISC 358 "PREQUALIFIED CONNECTIONS FOR SPECIAL AND INTERMEDIATE STEEL MOMENT FRAMES FOR SEISMIC APPLICATIONS."

18. WELD MATERIALS USED IN SFRS WELDED CONNECTIONS SHALL CONFORM TO THE FOLLOWING TOUGHNESS REQUIREMENTS:

A. WELDED CONNECTIONS SHALL BE MADE WITH A FILLER METAL THAT CAN PRODUCE WELDS THAT HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LB AT 0°F AS DETERMINED BY THE APPROPRIATE AWS CLASSIFICATION TEST METHOD.

B. WELDED CONNECTIONS DESIGNATED AS "DEMAND CRITICAL", SHALL BE MADE WITH A FILLER METAL CAPABLE OF PROVIDING A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 40 FT-LB AT 70°F BASED ON WPS HEAT INPUT ENVELOPE TESTING PRESCRIBED IN ANNEX A OF AWS D1.8/D1.8M.

19. INTERMIX OF FILLER METAL: WHEN FCAW-S FILLER METALS ARE USED IN COMBINATION WITH FILLER METALS FOR OTHER PROCESSES, INCLUDING FCAW-G, SUPPLEMENTAL CVN NOTCH TOUGHNESS TESTING SHALL BE CONDUCTED IN ACCORDANCE WITH ONE OR MORE OF THE FOLLOWING:

A. TESTS AS DESCRIBED IN ANNEX B OF AWS D1.8/D1.8M.

B. PQR TESTS THAT CONTAIN INTERMIX WELD METAL, WHEREIN CVN TEST SPECIMENS HAVE BEEN TAKEN FROM THE INTERMIX ZONE.

20. WELDING OF SHEET METAL AND METAL STUDS SHALL BE IN ACCORDANCE WITH AWS D1.3/ D1.3M.

21. MOVEMENT CONNECTIONS SHALL BE JOINED USING BOLTS WITH NUT AND JAM NUT. SNUG TIGHTEN FIRST NUT, AND THEN BACK OFF 1/4 TURN. FULLY TIGHTEN JAM NUT. SLOTTED HOLES SHALL BE MILLED SMOOTH. WHEN A SLOTTED HOLE OCCURS IN THE OUTER PLY, PROVIDE AN ASTM F436 WASHER OR A 5/16" PLATE WASHER TO COVER THE HOLE.

22. SLIDE BEARINGS SHALL BE CON-SLIDE TYPE CSA AS MANUFACTURED BY CON-SERV INC. OR APPROVED EQUAL. SLIDE BEARINGS SHALL BE ATTACHED TO THE STRUCTURE MEMBER WITH 1/8"x1" LONG FILLET WELDS AT 6" MAXIMUM SPACING. EACH SIDE SHALL HAVE AT LEAST TWO WELDS (ONE AT EACH CORNER).

23. CONTRACTOR SHALL PROVIDE FOR AN ALLOWANCE OF 5 TONS OR 2%, WHICHEVER IS GREATER, OF STRUCTURAL STEEL TO BE FABRICATED AND/OR ERECTED DURING THE PROGRESS OF WORK AS MAY BE DIRECTED BY THE ARCHITECT (STRUCTURAL ENGINEER). THE UNUSED PORTION SHALL BE CREDITED TO THE OWNER AT THE COMPLETION OF STRUCTURAL STEEL WORK.

24. THE USE OF ROLLED STEEL SECTIONS AND/OR BOLTS MANUFACTURED OUTSIDE THE UNITED STATES WILL REQUIRE VERIFICATION THAT THE PRODUCTS COMPLY WITH APPLICABLE ASTM STANDARDS. MILL CERTIFICATES WILL BE REQUIRED FOR ALL STEEL. STEEL GRADES OTHER THAN ASTM A36 WILL REQUIRE TESTING BY APPROVED LABORATORY. ALL FOREIGN BOLTS MUST BE APPROVED BY COUNTY OF LOS ANGELES BUILDING AND SAFETY PRIOR TO THEIR USE.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF ANSI/AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS", ANSI/AISC 341 "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS", AND AISI/AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", AS AMENDED BY CALIFORNIA BUILDING CODE (CBC) SECTIONS 2203, THROUGH 2206.
2. SEISMIC FORCE RESISTING SYSTEM (SFRS) IS THAT PART OF THE STRUCTURAL SYSTEM THAT HAS BEEN CONSIDERED IN THE DESIGN TO PROVIDE THE REQUIRED RESISTANCE TO THE SEISMIC FORCES PRESCRIBED IN ASCE/SEI 7.
3. STRUCTURAL STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING ASTM STANDARDS, UNLESS NOTED OTHERWISE ON DRAWINGS:

WIDE FLANGE SHAPESASTM A992/A992M

CHANNELS, ANGLES, M- & S-SHAPESASTM A36/A36M

PIPESASTM A53/A53M, GRADE B (F_y = 35 KSI)

ROUND HOLLOW STRUCTURAL SECTIONSASTM A500/A500M, GRADE C (F_y = 46 KSI)

RECTANGULAR HOLLOW STRUCTURAL SECTIONSASTM A500/A500M, GRADE C (F_y = 50 KSI)

PLATESASTM A572/A572M, GRADE 50

ANCHOR BOLTSASTM F1554, GRADE 36 (UNO)

ANCHOR BOLTS USED IN SFRSASTM F1554, GRADE 55, WELDABLE,(UNO)

UNFINISHED MACHINE BOLTSASTM A307

THREADED ROUND STOCKASTM A36/A36M

4. HIGH STRENGTH BOLTS, NUTS AND WASHERS SHALL CONFORM TO THE RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS", AS AMENDED BY CBC SECTION 2204.2. WHEN ASSEMBLED, ALL JOINT SURFACES, INCLUDING THOSE ADJACENT TO WASHERS, SHALL BE FREE OF SCALE, EXCEPT TIGHT MILL SCALE. USE STANDARD HOLES UNLESS NOTED OTHERWISE.

A. PROVIDE ASTM A325, TYPE I, SNUG-TIGHTENED (ST) BOLTS WITH THREADS INCLUDED IN SHEAR PLANE, UNLESS NOTED OTHERWISE. PROVIDE ASTM A325, TYPE I, SLIP-CRITICAL (SC) BOLTS AT CONNECTIONS IN SFRS AND WHERE SPECIFICALLY INDICATED. FAYING SURFACES FOR SLIP-CRITICAL CONNECTIONS SHALL MEET CLASS A SLIP RESISTANCE, UNLESS OTHERWISE NOTED. NUTS AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM A563 AND ASTM F436, RESPECTIVELY.

B. ASTM A325-ST BOLTS SHALL BE SNUG TIGHTENED IN ACCORDANCE WITH RCSC SPECIFICATION SECTION 8.1, UNLESS NOTED OTHERWISE. FULLY TENSION ALL ASTM A325-SC BOLTS AND ALL BOLTS REQUIRED TO BE TENSIONED BY AISC SPECIFICATION SECTION J3.1 AND RCSC SPECIFICATION SECTIONS 4.2 AND 4.3. FULLY TENSIONED BOLTS SHALL BE TIGHTENED TO THE MINIMUM TENSION USING TURN-OF-THE-NUT PRETENSIONING METHOD, CALIBRATED WRENCH PRETENSIONING METHOD, OR DIRECT-TENSION-INDICATOR PRETENSIONING METHOD USING DIRECT TENSION INDICATORS THAT MEET THE REQUIREMENTS OF ASTM F959.

C. TWIST-OFF-TYPE TENSION-CONTROL BOLTS THAT MEET THE REQUIREMENTS OF ASTM F3125/F3125M, TYPE 1, MAY BE USED IN LIEU OF ASTM A325-ST OR ASTM A325-SC BOLTS.

5. HEADED STUD ANCHORS SHALL BE NELSON TYPE S3L OR TYPE H4L FLUX-FILLED HEADED SHEAR CONNECTOR STUDS (ICC EVALUATION SERVICE REPORT ESR-2856), OR AN APPROVED EQUAL, AND SHALL BE MADE FROM COLD DRAWN, LOW CARBON STEEL CONFORMING TO ASTM A29, GRADES C1010 THROUGH C1020, WITH A MINIMUM TENSILE STRENGTH OF 65 KSI. STUD WELDING TEST AND INSPECTION SHALL CONFORM TO AWS D1.1, CLAUSE 7. ANCHOR LENGTHS NOTED ON DRAWINGS ARE AFTER WELD LENGTHS.

6. PROVIDE UPWARD CAMBER TO ALL BEAMS SPECIFIED TO HAVE CAMBER. AMOUNT MEASURES IN THE FIELD PRIOR TO ERECTION SHALL NOT DEVIATE MORE THAN ALLOWED BY AISC SPECIFICATIONS. BEAMS WITHOUT SPECIFIED CAMBER SHALL BE FABRICATED TO SO THAT ANY MINOR CAMBER DUE TO ROLLING SHALL BE UPWARD AFTER ERECTION.

7. PRIOR TO FABRICATION, SUBMIT SHOP DRAWINGS TO ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW AND, UPON REQUEST, TO GOVERNING CODE AUTHORITY. INDICATE AN ERECTION SEQUENCE OF WELDING TO MINIMIZE LOCKED-UP STRESSES OR DISTORTION FOR MOMENT-RESISTING STEEL FRAMES.

8. HOURLY FIRE RESISTIVE REQUIREMENTS FOR STRUCTURAL STEEL MEMBERS SHALL BE DETERMINED USING CBC TABLE 601. BUILDING TYPES OF CONSTRUCTION AND FIREPROOFING MATERIALS ARE AS INDICATED ON ARCHITECTURAL DRAWINGS.

9. ALL STEEL NOT ENCASED IN CONCRETE, MASONRY, OR FIREPROOFING SHALL BE SHOP PRIMED AND PAINTED PER SPECIFICATIONS, EXCEPT FOR TOP FLANGE OF BEAMS SUPPORTING METAL DECK. ANY ABRASIONS OR UNPAINTED AREAS SHALL BE TOUCHED UP AFTER ERECTION.

10. ALL STRUCTURAL STEEL AND MISCELLANEOUS METALS EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION UNLESS NOTED OTHERWISE ON ARCHITECTURAL DRAWINGS.

11. WELDING SHALL CONFORM TO LATEST EDITION OF AWS D1.1/ D1.1M, AS AMENDED IN CBC SECTION 2204.1.

A. WELDING PROCESS SHALL BE ELECTRIC ARC USING E70XX ELECTRODES. SUBMERGED ARC PROCESS (SAW) WITH AUTOMATIC WELDING MAY BE USED AS AN ALTERNATIVE.

B. WELDERS SHALL BE CERTIFIED TO CONFORM WITH AWS STANDARDS AND APPROVED BY THE GOVERNING CODE AUTHORITY.

C. SHOP WELDING, INCLUDING ULTRASONIC TESTING OF FULL PENETRATION GROOVE WELDS, SHALL BE PERFORMED ON THE PREMISES OF AN APPROVED FABRICATOR.

D. MINIMUM FILLET WELD SIZE SHALL CONFORM TO AISC SPECIFICATION TABLE J2.4. WELDS LENGTHS NOTED ON DRAWINGS ARE THE NET EFFECTIVE LENGTHS REQUIRED.

E. FIELD WELD SYMBOLS NOTED ON THE DRAWINGS SHOW ENGINEERING INTENT, BUT NO ATTEMPT HAS BEEN MADE TO CLASSIFY ALL WELDS. AT FABRICATOR'S OPTION, ANY WELD INDICATED AS A FIELD WELD MAY BE SHOP WELDED AND ANY WELD INDICATED AS A SHOP WELD MAY BE FIELD WELDED.

12. WELDS SHALL BE PREQUALIFIED PER AWS D1.1/D1.1M. NON-PREQUALIFIED WELDED JOINTS SHALL BE QUALIFIED BY TEST PER AWS D1.1/ D1.1M.

CONCRETE MASONRY UNITS

1. CONCRETE MASONRY UNITS (CMU) SHALL BE MEDIUM WEIGHT HOLLOW LOAD BEARING UNITS CONFORMING TO ASTM C90. NORMAL WEIGHT OR LIGHTWEIGHT UNITS MAY BE USED IF APPROVED BY THE ARCHITECT (STRUCTURAL ENGINEER). USE OPEN END UNITS AT VERTICAL REINFORCING AND BOND BEAM UNITS AT HORIZONTAL REINFORCING.
2. SPECIFIED COMPRESSIVE STRENGTH (f_m) OF COMPLETED CONCRETE MASONRY UNITS IS 2000 PSI UNLESS NOTED OTHERWISE. COMPLIANCE WITH SPECIFIED COMPRESSIVE STRENGTH SHALL BE BY THE UNIT STRENGTH METHOD OR BY THE PRISM TEST METHOD IN ACCORDANCE TO TMS 602 ARTICLES 1.4 B.2.b OR 1.4 B.3, RESPECTIVELY.
3. MORTAR MIX SHALL CONFORM TO THE REQUIREMENTS OF ASTM C270 FOR TYPE S MORTAR. MORTAR SAND SHALL BE FREE OF INJURIOUS AMOUNTS OF DELETERIOUS SUBSTANCES AND ORGANIC IMPURITIES AND CONFORM TO ASTM C144.
4. GROUT SHALL CONFORM TO THE REQUIREMENTS ASTM C476 FOR COARSE GROUT. GROUT SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI, OR CMU BLOCK STRENGTH, WHICHEVER IS THE GREATER, AT 28 DAYS. AGGREGATES FOR COARSE GROUT SHALL CONFORM TO ASTM C404. SELF-CONSOLIDATING GROUT IS NOT PERMITTED.
5. CEMENT SHALL BE LOW ALKALI CONFORMING TO ASTM C150, TYPE I OR TYPE II. USE OF MASONRY CEMENT IS PROHIBITED.
6. WATER MUST BE CLEAN AND POTABLE AND FREE OF DELETERIOUS AMOUNTS OF OILS, ACIDS, ALKALIES, ORGANIC MATERIALS, AND SOLUBLE SALTS SUCH AS POTASSIUM AND SODIUM SULFATES.
7. CLEANOUTS SHALL BE PROVIDED FOR ALL GROUT POUR HEIGHTS OVER FIVE FEET FOUR INCHES. WHERE REQUIRED, CLEANOUTS SHALL BE PROVIDED IN THE BOTTOM COURSE AT EVERY VERTICAL REINFORCING BUT SHALL NOT BE SPACED GREATER THAN 32 INCHES ON CENTER FOR SOLID GROUTED MASONRY. CLEANOUTS SHALL BE OF SUFFICIENT SIZE TO PERMIT REMOVAL OF DEBRIS, BUT NOT LESS THAN 3 INCHES. CLEANOUTS SHALL BE SEALED AFTER INSPECTION AND BEFORE GROUTING.
8. GROUT POUR HEIGHT SHALL NOT EXCEED THAT SHOWN IN TMS 402, TABLE 3.2.1. GROUT SHALL BE PLACED IN A CONTINUOUS POUR IN GROUT LIFTS NOT EXCEEDING FIVE FEET FOUR INCHES. BETWEEN GROUT POURS, A HORIZONTAL CONSTRUCTION JOINT SHALL BE FORMED BY STOPPING GROUT POUR 1-1/2 INCHES BELOW TOP OF MORTAR JOINT.
9. CELLS SHALL BE IN VERTICAL ALIGNMENT. ALL CELLS SHALL BE GROUTED SOLID UNLESS OTHERWISE NOTED. CONSOLIDATE BY MECHANICAL VIBRATION AND RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED.
10. SEE REINFORCING STEEL GENERAL NOTES FOR REINFORCING STEEL IN CMU.
11. WALL AND PILASTER DOWELS IN FOOTINGS SHALL MATCH SIZE, GRADE, AND SPACING OF RESPECTIVE VERTICAL REINFORCING UNLESS OTHERWISE NOTED. DOWELS SHALL BE SET TO ALIGN WITH CELLS CONTAINING REINFORCING STEEL.
12. REINFORCING STEEL SHALL BE SECURED AGAINST DISPLACEMENT PRIOR TO GROUTING USING WIRE POSITIONERS. PROVIDE A MINIMUM OF ONE BAR DIAMETER, LARGEST SIZE AGGREGATE PLUS 1/4 INCH, OR 1/2 INCH, WHICHEVER IS GREATER, GROUT BETWEEN MAIN REINFORCING STEEL AND MASONRY UNIT.
13. REINFORCING STEEL, EXCEPT JOINT REINFORCING, SHALL BE COMPLETELY EMBEDDED IN MORTAR OR GROUT AND HAVE A MINIMUM COVER, INCLUDING CMU SHELL, AS FOLLOWS:

A. CMU NOT EXPOSED TO EARTH OR WEATHER1-1/2"

B. CMU EXPOSED TO EARTH OR WEATHER:

NO. 6 AND LARGER2"

NO. 5 AND SMALLER1-1/2"

14. MINIMUM CLEARANCE BETWEEN PARALLEL REINFORCING STEEL SHALL BE ONE INCH, NOMINAL BAR DIAMETER, OR 4/3 TIMES THE MAXIMUM SIZE OF AGGREGATE, WHICHEVER IS GREATER.

15. MINIMUM CLEAR DISTANCE BETWEEN VERTICAL BARS IN COLUMN AND PILASTERS SHALL NOT BE LESS THAN 1.5 TIMES THE NOMINAL BAR DIAMETER, NOR LESS THAN 1 1/2 INCHES.

16. PROVIDE ONE INCH MINIMUM GROUT COVER AROUND ANCHOR BOLTS, REINFORCING STEEL DOWELS, AND OTHER INSERTS PENETRATING CMU SHELL. ANCHOR BOLTS SHALL BE HEADED TYPE BOLTS. ANCHOR BOLTS WITH HOOKED ENDS ARE NOT PERMITTED.

17. REFER TO ARCHITECTURAL DRAWINGS FOR SURFACE AND HEIGHT OF MASONRY UNITS, LAYING PATTERN AND JOINT TYPE. IF NOT SPECIFIED OTHERWISE ON ARCHITECTURAL DRAWINGS, USE 8"x8"x16" NATURAL GRAY PRECISION UNITS IN RUNNING BOND PATTERN WITH TOOLED CONCAVE MORTAR JOINT.

18. MORTAR JOINTS SHALL 3/8 INCH THICK (UNLESS NOTED OTHERWISE), EXCEPT THAT THICKNESS OF STARTER MORTAR JOINT PLACED OVER FOUNDATIONS SHALL NOT BE LESS THAN 1/4 INCH AND MORE THAN 3/4" INCH.

19. INCLEMENT WEATHER:

- A. WHEN AMBIENT AIR TEMPERATURE FALLS BELOW 40-DEGREE F, IMPLEMENT COLD WEATHER PROCEDURES COMPLYING WITH TMS 602 ARTICLE 1.8 C.
- B. WHEN AMBIENT AIR TEMPERATURE EXCEEDS 100-DEGREE F, OR WHEN AMBIENT AIR TEMPERATURE EXCEEDS 90-DEGREE F WITH A WIND VELOCITY GREATER THAN 8 MPH, IMPLEMENT HOT WEATHER PROCEDURES COMPLYING WITH TMS 602 ARTICLE 1.8 D.

20. UNLESS OTHERWISE NOTED, FULL ALLOWABLE STRESSES ARE USED IN DESIGN. SPECIAL INSPECTION REQUIRED FOR ERECTION OF ALL CONCRETE MASONRY UNITS.

21. HEADED BOLTS IN MASONRY SHALL CONFORM TO ASTM A449 UNLESS NOTED OTHERWISE.

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

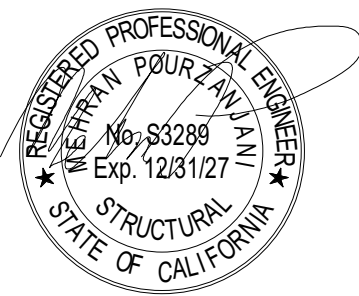
TEL 310 392 3995

SO
seal of architect

725 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.williamlloydjones.com
(213) 315-2277
Project #25534

GENERAL NOTES

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA



THE ABOVE DRAWINGS AND SPECIFICATIONS ARE THE SOLE DESIGN AND
ARRANGEMENTS REPRESENTED BY THE ARCHITECT. THE ARCHITECT SHALL BE
PROTECTED BY THE ARCHITECT'S PROFESSIONAL SEAL AND SHALL REMAIN THE
PROPERTY OF THE ARCHITECT. NO PART OF THESE DRAWINGS SHALL BE COPIED,
DISCLOSED TO OTHERS OR USED IN CONNECTION WITH ANY WORK OR PROJECT
OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY WERE PREPARED,
AND DEVELOPED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT. VERBAL
CONTACT WITH THESE DRAWINGS OR SPECIFICATIONS SHALL NOT CONSTITUTE
CONCLUSIVE EVIDENCE OF ACCEPTANCE OF THESE DRAWINGS.

WRITTEN OWNERSHIP ON THESE DRAWINGS SHALL BE THE RESPONSIBILITY OF THE ARCHITECT. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THE OFFICE MUST BE NOTIFIED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS SHOWN IN THESE DRAWINGS. SHOP DRAWINGS MUST BE SUBMITTED TO THIS OFFICE FOR APPROVAL. BEFORE PROCEEDING WITH FABRICATION.

Date	Issue Date
Drawn	
Checked	
Scale	AS NOTED
Job. No.	Project Number

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003

ADDED 5

STRUCTURAL OBSERVATION - CONTINUED

9. WHEN THERE IS A NEED TO REPLACE THE STRUCTURAL OBSERVER OF RECORD, THE OWNER SHALL:
- A. NOTIFY THE BUILDING INSPECTOR IN WRITING BEFORE THE NEXT INSPECTION
- B. CALL AN ADDITIONAL PRECONSTRUCTION MEETING, AND

STRUCTURAL OBSERVATION

- A. FURNISH THE REPLACEMENT STRUCTURAL OBSERVER WITH A COPY OF ALL PREVIOUS OBSERVATION REPORTS.
- B. THE NEW STRUCTURAL OBSERVER MUST BE DESIGNATED BY THE ENGINEER OR ARCHITECT OF RECORD.
- C. THE REPLACEMENT STRUCTURAL OBSERVER SHALL APPROVE THE CORRECTION OF THE ORIGINAL OBSERVED DEFICIENCIES UNLESS OTHERWISE APPROVED BY THE GOVERNING CODE AUTHORITY. THE POLICY OF THE GOVERNING CODE AUTHORITY SHALL BE TO CORRECT ANY PROPERLY NOTED DEFICIENCIES WITHOUT CONSIDERATION OF THEIR SOURCE.
10. THE ENGINEER OR ARCHITECT OF RECORD WILL DEVELOP ALL CHANGES RELATING TO THE STRUCTURAL SYSTEMS. THE GOVERNING CODE AUTHORITY SHALL REVIEW AND APPROVE ALL CHANGES TO THE APPROVED PLANS AND SPECIFICATIONS.
11. STRUCTURAL OBSERVATION/SIGNIFICANT CONSTRUCTION STAGE TABLE

STRUCTURAL OBSERVATION/SIGNIFICANT CONSTRUCTION STAGE TABLE		
CONST STAGE	CONST TYPE	ELEMENTS/CONNECTIONS TO BE OBSERVED
FOUNDATION	SPREAD FOOTINGS CONTINUOUS FTGS GRADE BEAMS	FIRST SIGNIFICANT SEISMIC FOUNDATION POUR REINF IN-PLACE PRIOR TO CONCRETING
APPARATUS ROOM CONC FRAME	CONC MOMENT FRAMES	REINF IN-PLACE PRIOR TO CONCRETING
STEEL	STEEL DECKING	STEEL DECK PRIOR TO ROOFING
	STEEL DECKING FRAMING	STEEL FRAMING IN-PLACE PRIOR TO DECKING
WOOD	FRAMING AND NAILING OF SHEAR WALLS AND DIAPHRAGM	PRIOR TO COVERING
COMPLETION	OVERALL STRUCTURE	ALL OBSERVED DEFICIENCIES RESOLVED GENERAL CONFORMANCE TO APPROVED DOCUMENTS

PARALLAM PARALLEL STRAND LUMBER BEAMS

1. MANUFACTURER: PARALLAM PSL 2.2E DF BY WEYERHAEUSER.
2. PROPERTIES:

JOIST/BEAM ORIENTATION

E = 2.2x10 PSI

Emin = 1.18x10 PSI

Fb = 2900 PSI

Fv = 290 PSI

Fc (pcrp) = 750 PSI

Axial

Ft = 1755 PSI

Fc = 2500 PSI
3. PROVIDE MATERIALS AND MANUFACTURE TO THE STANDARDS SET FORTH IN ICC-ESR-1387.
4. ADHESIVES: WATERPROOFING TYPE CONFORMING TO THE REQUIREMENTS OF ASTM D-2559.

TIMBER STRAND LAMINATED STRAND LUMBER BEAMS

1. MANUFACTURER: TIMBER STRAND LSL 1.55E DF BY WEYERHAEUSER.
2. PROPERTIES:

JOIST/BEAM ORIENTATION

E = 1.55x10 PSI

Emin = 1.3x10 PSI

Fb = 2325 PSI

Fv = 310 PSI

Fc (pcrp) = 900 PSI

FACE/FLAT/PLANK ORIENTATION

Fb = 1900 PSI

Fv = 150 PSI

Fc (pcrp) = 375 PSI

Axial

Ft = 1075 PSI

Fc = 1835 PSI
3. PROVIDE MATERIALS AND MANUFACTURE TO THE STANDARDS SET FORTH IN ICC-ESR-1387.
4. ADHESIVES: WATERPROOFING TYPE CONFORMING TO THE REQUIREMENTS OF ASTM D-2559.

PERFORMANCE CRITERIA FOR PV PANEL FRAMING AND ATTACHMENT DESIGN (DESIGN BUILD)

1. ALL PV PANEL FRAMING AND MOUNTING IS DESIGN-BUILD
2. DESIGN SHALL INCLUDE FRAMING MEMBERS, CONNECTIONS (INCLUDING THOSE TO BUILDING STRUCTURE) AND PROVIDING FOR LATERAL RESTRAINT OF THE PANELS. ALL DESIGN SHALL COMPLY WITH CONTRACT DOCUMENTS AND GOVERNING CODE.
3. THE TOTAL WEIGHT OF THE SOLAR PANEL INCLUDING MOUNTING ASSEMBLY AND TIES SHALL NOT EXCEED 5.0 PSF.
4. THE SOLAR PANEL FRAMING AND MOUNTING ASSEMBLY SHALL BE CONFORM TO REQUIREMENTS OF CBC 2022, AND ASCE 7-16 INCLUDING SECTION 13.6.12.
5. THE MOUNTING COMPONENTS AND PV PANELS SHALL INTERCONNECTED TO ENSURE THE ENTIRE ASSEMBLY IS MOVING TOGETHER IN A SEISMIC EVENT AS REQUIRED BY APPLICABLE CODES.
6. THE DESIGN OF THE PV PANEL FRAMING AND MOUNTING SHALL BE CONFORM TO THE DESIGN CONCEPTS AND INTENT SHOWN ON THE DESIGN DRAWINGS AND DOCUMENTS.

ROUGH CARPENTRY - CONTINUED

22. PLYWOOD SHALL BE GRADE MARKED C OR D. HORIZONTAL PLYWOOD SHALL BE LAID WITH FACE GRAIN PERPENDICULAR TO JOISTS. FLOOR AND ROOF SHEATHING SHALL HAVE A PANEL INDEX OF 32/16 UNLESS OTHERWISE NOTED.
23. NAILING OF PLYWOOD TO BE APPROVED BY THE INSPECTOR BEFORE COVERING WITH ROOF, FLOOR OR WALL MATERIALS.
24. MACHINE APPLIED (PNEUMATIC) NAILING IS SUBJECT TO A SATISFACTORY JOBSITE DEMONSTRATION AND TO THE APPROVAL OF THE GOVERNING CODE AUTHORITY AND THE ARCHITECT (STRUCTURAL ENGINEER). THE APPROVAL IS SUBJECT TO CONTINUED SATISFACTORY PERFORMANCE. MACHINE APPLIED NAILING MAY BE USED ONLY ON PLYWOOD GREATER THAN 5/16" THICK. SHINERS SHALL BE REPLACED.

A. IF NAIL HEADS PENETRATE THE OUTER PLY BY MORE THAN WOULD BE NORMAL FOR A HAND HAMMER OR IF MINIMUM EDGE DISTANCES ARE NOT MAINTAINED, THE PERFORMANCE WILL BE DEEMED UNSATISFACTORY AND MACHINE NAILING SHALL BE DISCONTINUED
25. GLULAM LUMBER: DOUGLAS FIR STRUCTURAL LUMBER COMPLYING WITH STANDARD GRADING RULES NO. 17 OF THE WEST COAST LUMBER INSPECTION BUREAU. FABRICATE GLUED-LAMINATED LUMBER IN COMPLIANCE WITH AITC STANDARD SPECIFICATIONS FOR STRUCTURAL GLUED-LAMINATED TIMBER OF SOFTWOOD SPECIES (AITC 117). ANSI/AITC A190.1 AND ASTM D3737).

A. CONDITION OF USE: DRY CONDITION OF USE WITH A MOISTURE CONTENT LESS THAN 16 PERCENT AT THE TIME OF GLUING.

B. JOINT ADHESIVE: BOND LAMINATIONS TOGETHER WITH WET USE (WATERPROOF) ADHESIVES IN COMPLIANCE WITH ASTM D2559.

C. APPEARANCE GRADE: INDUSTRIAL.

a. COMBINATIONS SYMBOLS:

SIMPLE SPAM BEAMS.....24F-V4

CANTILEVER AND CONTINUOUS BEAMS.....24F-V8

D. SHOP DRAWINGS AND CERTIFICATES OF COMPLIANCE: SUBMIT FOR EACH MEMBER IN COMPLIANCE WITH APPLICABLE CODE TO ARCHITECT (STRUCTURAL ENGINEER) AND GOVERNING CODE AUTHORITY. INDICATE GRADE, TYPE OF GLUE, COMBINATION SYMBOLS, DRY OR WET CONDITION OF USE, CAMBERS, PRESSURE TREATMENT (IF REQUIRED).

E. ALL STRUCTURAL GLUED-LAMINATED TIMBER SHALL BE CONTINUOUSLY INSPECTED DURING FABRICATION BY A GLUE FABRICATION INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE GOVERNING CODE AUTHORITY.
26.

A. HOLD DOWN CONNECTORS SHALL BE TIGHTENED TO FINGER TIGHT PLUS ONE-HALF WRENCH TURN JUST PRIOR TO COVERING THE WALL FRAMING.

B. CONNECTOR BOLTS INTO WOOD FRAMING SHALL REQUIRE STEEL PLATE WASHERS ON THE POST ON THE OPPOSITE SIDE OF THE ANCHORAGE DEVICE. PLATE SIZE SHALL BE A MINIMUM OF 0.229"x3"x3"

C. HOLD DOWN HARDWARE MUST BE SECURED IN PLACE PRIOR TO FOUNDATION INSPECTION.

STRUCTURAL OBSERVATION

1. STRUCTURAL OBSERVATION IS REQUIRED FOR THE STRUCTURAL SYSTEM IN ACCORDANCE WITH SECTION 1704.6 OF THE CALIFORNIA BUILDING CODE (CBC). STRUCTURAL OBSERVATION IS THE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM BY A REGISTERED DESIGN PROFESSIONAL FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES AND AT THE COMPLETION OF THE STRUCTURAL SYSTEM. SIGNIFICANT CONSTRUCTION STAGES ARE THE STAGES OF CONSTRUCTION IDENTIFIED BY THE ENGINEER OF RECORD AS SIGNIFICANT AND REQUIRE SITE STRUCTURAL OBSERVATION. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY SECTION 110 AND SECTION 1705 OF THE CBC.
2. THE OWNER SHALL EMPLOY A STATE OF CALIFORNIA REGISTERED CIVIL OR STRUCTURAL ENGINEER OR LICENSED ARCHITECT TO PERFORM THE STRUCTURAL OBSERVATION. THE GOVERNING CODE AUTHORITY REQUIRES THE USE OF THE ENGINEER OR ARCHITECT, OR HIS/HER DESIGNEE RESPONSIBLE FOR THE STRUCTURAL DESIGN WHO ARE INDEPENDENT OF THE CONTRACTOR.
3. THE STRUCTURAL OBSERVER SHALL PROVIDE EVIDENCE OF EMPLOYMENT BY OWNER OR THE OWNER'S REPRESENTATIVE. A LETTER FROM THE OWNER, THE OWNER'S REPRESENTATIVE, OR A COPY OF THE AGREEMENT FOR SERVICES SHALL BE SENT TO BUILDING INSPECTOR BEFORE THE FIRST SITE VISIT.
4. THE OWNER OR OWNER'S REPRESENTATIVE SHALL COORDINATE AND CALL FOR A MEETING BETWEEN ENGINEER OR ARCHITECT RESPONSIBLE FOR THE STRUCTURAL DESIGN, STRUCTURAL OBSERVER, AFFECTED SUBCONTRACTORS AND DEPUTY INSPECTORS. THE PURPOSE OF THE MEETING SHALL BE TO IDENTIFY MAJOR STRUCTURAL ELEMENTS AND CONNECTIONS THAT AFFECT VERTICAL AND LATERAL LOAD SYSTEMS OF THE STRUCTURE AND TO REVIEW SCHEDULING OF THE REQUIRED OBSERVATIONS. A RECORD OF THE MEETING SHALL BE INCLUDED IN THE FIRST OBSERVATION REPORT SUBMITTED TO THE BUILDING INSPECTOR.
5. THE STRUCTURAL OBSERVER SHALL PERFORM SITE VISITS AT THOSE STEPS IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFICIENCIES WITHOUT SUBSTANTIAL EFFORT OR UNCOVERING OF THE WORK INVOLVED. AT A MINIMUM, THE LISTED SIGNIFICANT CONSTRUCTION STAGES ON THE FOLLOWING "STRUCTURAL OBSERVATION/SIGNIFICANT CONSTRUCTION STAGE TABLE" BELOW REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL OBSERVER.
6. THE STRUCTURAL OBSERVER SHALL PREPARE A REPORT FOR EACH SIGNIFICANT STAGE OF CONSTRUCTION OBSERVED. THE ORIGINAL OF THE STRUCTURAL OBSERVATION REPORT SHALL BE SENT TO THE BUILDING INSPECTOR'S OFFICE AND SHALL BE SIGNED AND SEALED (WET STAMP) BY THE RESPONSIBLE STRUCTURAL OBSERVER. ONE COPY OF THE OBSERVATION REPORT SHALL BE ATTACHED TO THE APPROVED PLANS. THE COPY ATTACHED TO PLANS SHALL BE SIGNED AND SEALED (WET STAMP) BY THE RESPONSIBLE STRUCTURAL OBSERVER OR THEIR DESIGNEE. COPIES OF REPORT SHALL ALSO BE GIVEN TO THE OWNER, CONTRACTOR, AND DEPUTY INSPECTOR. ANY DEFICIENCIES NOTED ON THE OBSERVATION REPORT WILL BECOME THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER OR ARCHITECT OF RECORD TO VERIFY ITS COMPLETION BY THE STRUCTURAL OBSERVER.
7. A FINAL OBSERVATION REPORT MUST BE SUBMITTED WHICH SHOWS THAT ALL OBSERVED DEFICIENCIES WERE RESOLVED AND STRUCTURAL SYSTEM GENERALLY CONFORMS WITH THE APPROVED PLANS AND SPECIFICATIONS. THE GOVERNING CODE AUTHORITY WILL NOT ACCEPT THE STRUCTURAL WORK WITHOUT THIS FINAL OBSERVATION REPORT.
8. THE STRUCTURAL OBSERVER SHALL PROVIDE THE ORIGINAL STAMPED AND SIGNED STRUCTURAL OBSERVATION REPORT TO THE BUILDING INSPECTOR.

ROUGH CARPENTRY

1. FRAMING LUMBER SHALL BE DOUGLAS FIR-LARCH COMPLYING WITH DOC PS-20 WITH A 19 PERCENT MAXIMUM MOISTURE CONTENT.
2. UNLESS NOTED OTHERWISE ON THE DRAWINGS, FRAMING MEMBER GRADES SHALL BE AS FOLLOWS:

A. VERTICAL FRAMING MEMBERS

1. 2x4 STUDS, 4x4 POSTS CONSTRUCTION NO. 1

2. 2x6 STUDS AND WIDER, 4x6 POSTS AND WIDER NO. 1

3. 5x5 POSTS AND LARGER NO. 1

4. ALL OTHER VERTICAL MEMBERS NO. 2

B. HORIZONTAL FRAMING MEMBERS

1. 2" AND 3" THICKNESS NO. 1

2. HEADERS IN NON-BEARING WALLS SPANNING LESS THAN 4 FT. NO. 2

3. ALL OTHER HORIZONTAL MEMBERS NO. 1
3. PLYWOOD SHALL BE STRUCTURAL I, EXPOSURE 1, COMPLYING WITH DOC PS-1 AND PS-2 AND THE APPLICABLE STANDARDS LISTED IN CBC SECTION 2306.1. EACH SHEET OF PLYWOOD SHALL BE IDENTIFIED WITH THE APPROPRIATE TRADEMARK OF AN APPROVED TESTING AND GRADING AGENCY.
4. ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED. PROVIDE GALVANIZED OR STAINLESS STEEL FASTENERS AND HARDWARE CONNECTORS AT PRESSURE TREATED LUMBER INCLUDING (BUT NOT LIMITED TO) ANCHOR BOLTS, NAILS, WASHERS, PLATES, HANGERS, CLIPS, HOLDDOWNS, ETC.
5. NAILS SHALL BE COMMON WIRE NAILS CONFORMING TO ASTM F1667. UNLESS OTHERWISE NOTED ON THE DRAWINGS, NAILING SHALL COMPLY WITH CBC TABLE 2304.10.1, FASTENING SCHEDULE.
6. BOLTS SHALL CONFORM TO ASTM A307. BOLT HOLES SHALL BE 1/32 TO 1/16 INCH LARGER THAN THE NOMINAL BOLT DIAMETER. PROVIDE STANDARD CUT PLATE WASHERS UNDER BOLT HEADS AND NUTS AGAINST WOOD. RETIGHTEN ALL BOLTS PRIOR TO CLOSING IN.
7. LAG SCREWS SHALL CONFORM TO ANSI/ASME B18.2.1. LAG SCREWS MUST BE INSERTED IN PREDRILLED HOLES. HOLE AT SHANK PORTION TO MATCH DIAMETER OF SHANK. HOLE AT THREADED PORTION TO BE 60 TO 70 PERCENT OF THE SHANK DIAMETER AND EQUAL TO LENGTH OF THE THREADED PORTION. USE SOAP OR OTHER LUBRICANTS TO FACILITATE INSTALLATION. DRIVING WITH HAMMER IS NOT PERMITTED. PROVIDE STANDARD CUT PLATE WASHERS LAG SCREW HEADS AGAINST WOOD.
8. WOOD CONNECTORS AND HOLDDOWNS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY INC. CONNECTORS SHALL HAVE CURRENT ICC-ES EVALUATION REPORTS. CONNECTORS, HANGERS AND STRAPS SHALL BE FULLY BOLTED OR NAILED TO DEVELOP FULL STRENGTH PER MANUFACTURER'S SPECIFICATIONS.
9. ANCHOR BOLTS INTO CONCRETE OR CMU SHALL CONFORM TO ASTM F1554, GRADE 36
10. GROUT SILL PLATES IF NECESSARY TO ACHIEVE FULL BEARING. ATTACH SILL PLATES TO CONCRETE WITH 5/8" DIAMETER ANCHOR BOLTS AT 4'-0" MAXIMUM UNLESS OTHERWISE NOTED. AT CORNERS, INTERSECTIONS, DOOR OPENINGS, SILL ENDS, AND CUTS EXCEEDING 1/3 THE SILL WIDTH, PLACE AN ANCHOR BOLT NOT MORE THAN 9" FROM THE NOTCH OR SILL END. ANCHOR BOLTS SHALL BE EMBEDDED A MINIMUM OF 8" INTO CONCRETE.
11. UNLESS OTHERWISE NOTED, DOUBLE TOP PLATES ON ALL EXTERIOR AND BEARING WALLS SHALL BE LAPPED 4'-0" MINIMUM AT SPLICES AND NAILED WITH 8-16d NAILS, MINIMUM, EACH SIDE OF LAP.
12. ALL HORIZONTAL FRAMING MEMBERS SHALL BE ERECTED WITH NATURAL OR BUILT-IN CAMBER UPWARD.
13. NOTCHING OR CUTTING OF STRUCTURAL LUMBER IS NOT PERMITTED UNLESS SPECIFICALLY DETAILED OR INDICATED. OBTAIN ARCHITECT'S (STRUCTURAL ENGINEER'S) APPROVAL FOR HOLES OR NOTCHES NOT DETAILED.

A. CUTTING, NOTCHING AND BORED HOLES IN WOOD STUDS SHALL COMPLY WITH CBC SECTIONS 2308.5.9 AND 2308.5.10.

B. NOTCHING AND BORED HOLES IN WOOD JOISTS AND RAFTERS SHALL COMPLY WITH CBC SECTION 2308.7.4.
14. LATERAL SUPPORT OF JOISTS AND RAFTERS SHALL COMPLY WITH CBC SECTION 2308.4.6. JOISTS/RAFTERS SHALL BE SUPPORTED Laterally AT THE ENDS AND AT EACH SUPPORT BY SOLID BLOCKING EXCEPT WHERE THE ENDS OF THE JOISTS/RAFTERS ARE NAILED TO A HEADER OR RIM JOIST OR TO AN ADJOINING STUD OR BY OTHER APPROVED MEANS. SOLID BLOCKING SHALL NOT BE LESS THAN 2 INCHES IN THICKNESS AND THE FULL DEPTH OF THE JOIST/RAFTER. IN ADDITION, SOLID BLOCKING SHALL BE PROVIDED AS FOLLOWS:

A. FLOOR AND ROOF JOISTS SHALL BE BLOCKED AT 8'-0" OC.

B. UNDER SUPPORTED TRANSVERSE PARTITIONS PERPENDICULAR TO THE JOIST.

15. PROVIDE DOUBLE JOIST UNDER ALL SUPPORTED PARTITION RUNNING PARALLEL TO THE FLOOR FRAMING.
16. WOOD STUD WALLS SHALL COMPLY WITH THE FOLLOWING:

A. STUD WALL BRACING IN STUD WALLS NOT PLYWOOD SHEATHED SHALL COMPLY WITH CBC SECTION 2308.5.7.

B. FIREBLOCKING SHALL BE IN ACCORDANCE TO CBC SECTION 719.2.

C. STUDS TO BE SPACED AT 16" OC MAXIMUM.
17. BEAMS OR DRAG-STRUTS CONSISTING OF DOUBLE JOISTS SHALL BE LAMINATED TOGETHER WITH 16d NAILS AT 9" OC STAGGERED. BEAM OR DRAG-STRUT CONSISTING OF THREE OR MORE JOISTS SHALL BE LAMINATE TOGETHER WITH 1/2-INCH DIAMETER BOLTS AT 24" OC STAGGERED.
18. POSTS AND STUDS SHALL BEAR ON SILL PLATES UNLESS OTHERWISE NOTED.
19. ALL WALLS NOT OTHERWISE BRACED SHALL HAVE 1x6 DIAGONAL LET-IN BRACING AT 25 FEET INTERVALS. EACH BRACE SHALL COVER 3 STUD SPACES MINIMUM AND BE ATTACHED TO TOP AND BOTTOM PLATES WITH 3-8d NAILS.
20. FIREBLOCKED WALLS SHALL BE FIREBLOCKED SUCH THAT NO SPACE EXCEEDS 8 FEET IN HEIGHT. STAIR STRINGERS SHALL BE FIREBLOCKED AT EACH END AND AT MIDHEIGHT.
21. WHERE STUD WALLS JOIN CONCRETE OR MASONRY WALLS, THE END STUD SHALL BE BOLTED THERETO WITH 1/2" DIAMETER BOLTS AT TOP, BOTTOM AND MIDHEIGHT. SUCH BOLTS SHALL BE EMBEDDED INTO THE WALL NOT LESS THAN 2/3 THE THICKNESS OF THE WALL OR 8" MAXIMUM.

POST-INSTALLED ANCHORS

1. POST-INSTALLED ANCHORS INSTALLED IN NORMAL WEIGHT OR LIGHTWEIGHT CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:

A. KWIK BOLT TZ EXPANSION ANCHORS (ICC-ES REPORT ESR-4266) AS MANUFACTURED BY HILTI, INC.

B. HIT-RE 500-V3 ADHESIVE ANCHORS (ICC-ES REPORT ESR-4561) AS MANUFACTURED BY HILTI, INC. (NORMAL WEIGHT CONCRETE ONLY).

C. HDI-P TZ EXPANSION ANCHORS (ICC-ES REPORT ESR-4236) AS MANUFACTURED BY HILTI, INC.
2. POST-INSTALLED ANCHORS INSTALLED IN FULLY GROUTED CONCRETE MASONRY UNITS SHALL BE AS FOLLOWS:

A. KWIK BOLT TZ EXPANSION ANCHORS (ICC-ES REPORT ESR-4561) AS MANUFACTURED BY HILTI, INC.

B. HIT HY 270 ADHESIVE ANCHOR SYSTEM (ICC-ES REPORT ESR-4143 AS MANUFACTURED BY HILTI, INC.
3. POST-INSTALLED ANCHORS OF EQUAL QUALITY AND WITH CURRENT ICC-ES OR IAPMO UES EVALUATION REPORT MAY BE SUBSTITUTED IF APPROVED BY THE ARCHITECT (STRUCTURAL ENGINEER).
4. POST-INSTALLED ANCHORS INSTALLED IN EXTERIOR EXPOSURE OR DAMP ENVIRONMENT SHALL BE STAINLESS STEEL.
5. DOWELS ANCHORED IN CONCRETE, CONCRETE MASONRY UNITS (GROUTED OR HOLLOW), OR HOLLOW BRICK SHALL BE AS FOLLOWS: DOWELS INSTALLED IN EXTERIOR EXPOSURE OR DAMP ENVIRONMENT SHALL BE STAINLESS STEEL. LENGTHS SHALL BE AS INDICATED ON DRAWINGS.

A. DEFORMED REINFORCING BARS: ASTM A615, GRADE 60; OR ASTM A706.

B. CARBON STEEL THREADED STEEL RODS: ASTM A36.

C. STAINLESS STEEL THREADED RODS: ASTM F593, ALLOY GROUP I, TYPE 304, CONDITION CW.
6. INSTALL POST-INSTALLED ANCHORS IN STRICT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS. REINFORCING STEEL DOWELS, THREADED RODS, AND ANCHORS SHALL BE FREE OF DUST, GREASE, RUST AND OTHER MATERIALS THAT WILL IMPAIR BOND WITH CONCRETE.
7. USE ONLY NON-REBAR CUTTING DRILL BITS TO DRILL HOLES IN CONCRETE AND CONCRETE MASONRY UNITS. EXISTING REINFORCING STEEL SHALL BE POSITIVELY LOCATED BY NON-DESTRUCTIVE MEANS PRIOR TO DRILLING HOLES. DO NOT CUT OR DAMAGE EXISTING REINFORCING STEEL UNLESS APPROVED BY THE ARCHITECT (STRUCTURAL ENGINEER).
8. WHERE EXISTING CONCRETE IS DAMAGED AND/OR DRILLED HOLES ABANDONED, THE DAMAGED CONCRETE OR ABANDONED HOLES SHALL BE REPAIRED OR FILLED WITH NON-SHRINK GROUT, RESPECTIVELY. BRING EACH CONDITION TO THE ATTENTION OF THE ARCHITECT (STRUCTURAL ENGINEER) PRIOR TO IMPLEMENTING REPAIRS.
9. TEST QUANTITY SHALL BE AS NOTED, (PER CBC 1913.5.3 & 1705.4):

APPLICATION QUANTITY

STRUCTURAL 100% OF BOLTS

NON-STRUCTURAL 50% OF BOLTS PER GROUP. ALTERNATE BOLTS.
10. THE FOLLOWING CRITERIA APPLY FOR THE ACCEPTANCE OF INSTALLED ANCHORS:

HYDRAULIC RAM METHOD: THE ANCHOR SHALL HAVE NO OBSERVABLE MOVEMENT TO THE APPLICABLE TEST LOAD. TEST LOAD SHALL BE APPLIED FOR A MINIMUM OF 15 SECONDS. FOR EXPANSION ANCHORS, A PRACTICAL WAY TO DETERMINE OBSERVABLE MOVEMENT IS THAT THE WASHER UNDER THE NUT BECOMES LOOSE. THE TESTING DEVICE SHALL NOT RESTRICT THE CONCRETE SHEAR CONE TYPE FAILURE MECHANISM FROM OCCURRING. THE APPLICABLE TEST TORQUE MUST BE REACHED WITHIN ONE HALF (1/2) TURN OF THE NUT.

TORQUE WRENCH METHOD:
11. REQUIRED TEST LOADS SHALL BE DETERMINED AS THE LESSER OF TWICE THE MAXIMUM ALLOWABLE TENSION LOAD OR 80% OF THE NOMINAL YIELD STRENGTH OF THE ANCHOR ELEMENT.
12. TORQUE TESTS SHALL BE PERFORMED WITH THE INSTALLATION TORQUE SPECIFIED IN THE REPORT.

HILTI KB-TZ TORQUE TEST PER ICC REPORT (CONCRETE)			
DIAMETER	3/8"	1/2"	5/8"
TORQUE	25 FT-LB	40 FT-LB	60 FT-LB
	110 FT-LB		

HILTI KB-TZ TORQUE TEST PER ICC REPORT (CMU)			
DIAMETER	3/8"	1/2"	5/8"
TORQUE	15 FT-LB	25 FT-LB	35 FT-LB
	70 FT-LB		

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

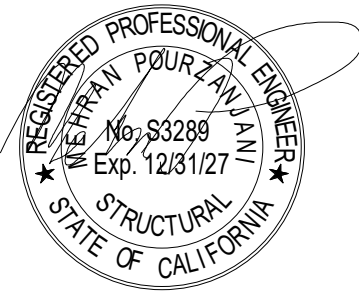
TEL 310 392 3995

SO
saif-ibrahim
structural engineers

725 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.saifibrahim.com
(213) 315-2277
Project #25534

GENERAL NOTES

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA



THE ABOVE DRAWINGS AND SPECIFICATIONS ARE THE SOLE DESIGN AND
ARRANGEMENTS REPRESENTED BY THE ARCHITECT. NO PARTS SHALL BE COPIED,
REPRODUCED, OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR
MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION
SYSTEM, WITHOUT THE WRITTEN PERMISSION OF THE ARCHITECT. VISUAL
CONTACT WITH THESE DRAWINGS OR SPECIFICATIONS SHALL CONSTITUTE
CONCLUSIVE EVIDENCE OF ACCEPTANCE OF THESE INSTRUCTIONS.
WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL BE THE GOVERNANCE OVER
SCALED DIMENSIONS. CONTRACTORS SHALL BE RESPONSIBLE FOR VERIFYING
FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THE OFFICE MUST
BE NOTIFIED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS
SHOWN IN THESE DRAWINGS. BEFORE PROCEEDING WITH CONSTRUCTION,
CONTRACTORS SHALL SUBMIT A REQUEST FOR CLARIFICATION TO THE
OFFICE FOR APPROVAL. BEFORE PROCEEDING WITH CONSTRUCTION,
CONTRACTORS SHALL SUBMIT A REQUEST FOR CLARIFICATION TO THE
OFFICE FOR APPROVAL.

Date	Issue Date
Drawn	
Checked	
Scale	AS NOTED
Job. No.	Project Number

SO04

ADDENDUM 5

STATEMENT OF SPECIAL INSPECTIONS (Cont'd)

8. APPROVED AGENCY SHALL PERFORM SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE IN ACCORDANCE WITH CBC SECTION 1705.12 FOR THE FOLLOWING WORK:

A. INSPECTION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF AISC 341 (CHAPTER J) AND TABLE 7.1.

1. NONDESTRUCTIVE TESTING (NDT) OF WELDED JOINTS SHALL BE IN ACCORDANCE WITH AISC 341 (CHAPTER J.6), AWS D1.8/D1.8M AS FOLLOWS:

a. K-AREA NDT: WHERE WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, THE WEB SHALL BE TESTED FOR CRACKS USING MT. THE MT INSPECTION AREA SHALL INCLUDE THE K-AREA BASE METAL WITHIN 3 INCHES OF THE WELD. THE MT SHALL BE PERFORMED NO SOONER THAN 48 HOURS FOLLOWING COMPLETION OF THE WELDING.

b. CJP GROOVE WELD NDT: UT SHALL BE PERFORMED ON 100 PERCENT OF CJP GROOVE WELDS IN MATERIALS 5/16 INCH OR GREATER. UT IN MATERIAL LESS THAN 5/16 INCH THICK IS NOT REQUIRED. MT SHALL BE PERFORMED ON 25 PERCENT OF ALL BEAM-TO-COLUMN CJP GROOVE WELDS.

c. BASE METAL NDT FOR LAMELLAR TEARING AND LAMINATIONS: AFTER JOINT COMPLETION, BASE METAL THICKER THAN 1-1/2 INCHES LOADED IN TENSION IN THE THROUGH THICKNESS DIRECTION IN TEE AND CORNER JOINTS, WHERE THE CONNECTED MATERIAL IS GREATER THAN 3/4 INCH AND CONTAINS CJP GROOVE WELDS, SHALL BE UT FOR DISCONTINUITIES BEHIND AND ADJACENT TO THE FUSION LINE OF SUCH WELDS. ANY BASE METAL DISCONTINUITIES FOUND WITHIN 1/4" OF THE STEEL SURFACE SHALL BE ACCEPTED OR REJECTED ON THE BASIS OF CRITERIA OF AWS D1.1 TABLE 6.2, WHERE T IS THE THICKNESS OF THE PART SUBJECTED TO THE THROUGH-THICKNESS STRAIN.

d. WELD TAB REMOVAL SITES: MT SHALL BE PERFORMED ON THE END OF WELDS FROM WHICH THE WELD TABS HAVE BEEN REMOVED, EXCEPT FOR CONTINUITY PLATE WELD TABS.

e. REDUCTION OF PERCENTAGE OF UT: THE REDUCTION OF PERCENTAGE OF UT IS PERMITTED TO BE REDUCED IN ACCORDANCE WITH NOTE 7.A.1.f ABOVE, EXCEPT NO REDUCTION IS PERMITTED FOR DEMAND CRITICAL WELDS.

f. REDUCTION OF PERCENTAGE OF MT: THE AMOUNT OF MT ON CJP GROOVE WELDS IS PERMITTED TO BE REDUCED IF APPROVED BY THE ENGINEER OF RECORD AND THE GOVERNING CODE AUTHORITY. THE MT RATE FOR AN INDIVIDUAL WELDER OR WELDING OPERATOR MAY BE REDUCED TO 10 PERCENT, PROVIDED THE REJECT RATE IS DEMONSTRATED TO BE 5 PERCENT OR LESS OF THE WELDS TESTED FOR THE WELDER OR WELDING OPERATOR. A SAMPLING OF AT LEAST 20 COMPLETED WELDS FOR A JOB SHALL BE MADE FOR SUCH REDUCTION EVALUATION. REJECT RATE IS THE NUMBER OF WELDS CONTAINING REJECTABLE DEFECTS DIVIDED BY THE NUMBER OF WELDS COMPLETED. THIS REDUCTION IS NOT PERMITTED ON WELDS IN THE K-AREA, AT REPAIR SITES, WELD TAB AND BACKING REMOVAL SITES AND ACCESS HOLES.

9. DEFINITIONS

A. CONTINUOUS SPECIAL INSPECTION: SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.

B. PERIODIC SPECIAL INSPECTION: SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED.

C. OBSERVE (O): APPROVED AGENCY SHALL OBSERVE THESE FUNCTIONS ON A RANDOM, DAILY BASIS. OPERATIONS NEED NOT BE DELAYED PENDING OBSERVATIONS.

D. PERFORM (P): THESE INSPECTIONS SHALL BE PERFORMED PRIOR TO THE FINAL ACCEPTANCE OF THE ITEM.

E. DOCUMENT (D): APPROVED AGENCY SHALL PREPARE REPORTS INDICATING THAT THE WORK HAS BEEN PERFORMED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. FOR SHOP FABRICATION, THE REPORT SHALL INDICATE THE PIECE MARK OF THE PIECE INSPECTED. FOR FIELD WORK, THE REPORT SHALL INDICATE THE REFERENCE GRID LINES AND FLOOR OR ELEVATION INSPECTED. WORK NOT IN COMPLIANCE WITH THE CONTRACT DOCUMENTS AND WHETHER THE NONCOMPLIANCE HAS BEEN SATISFACTORILY REPAIRED SHALL BE NOTED IN THE INSPECTION REPORT.

QUALITY ASSURANCE

1. TESTING LABORATORY: RETAINED BY OWNER AND SATISFACTORY TO ARCHITECT (STRUCTURAL ENGINEER) AND APPLICABLE CODE.

2. MATERIAL CERTIFICATION: SUBMIT LABORATORY TEST REPORTS CERTIFYING MATERIALS ARE OF IDENTIFIABLE TESTED STOCK COMPLYING WITH PROJECT SPECIFICATIONS TO OWNER, TESTING LABORATORY, ARCHITECT (STRUCTURAL ENGINEER) AND, UPON REQUEST, TO GOVERNING CODE AUTHORITY. IF LABORATORY TEST REPORTS CANNOT BE MADE AVAILABLE, TESTING LABORATORY WILL PERFORM TESTS AS DIRECTED BY ARCHITECT (STRUCTURAL ENGINEER). CONTRACTOR SHALL PAY TESTING LABORATORY FOR COSTS RELATED TO TESTS AND INSPECTIONS OF UNIDENTIFIABLE MATERIALS. MATERIALS FURNISHED WITHOUT LABORATORY TEST REPORTS, MATERIALS ROUND DEFICIENT AFTER INITIAL TESTS AND INSPECTIONS, AND/OR MATERIALS REPLACING DEFICIENT MATERIALS.

3. TESTS AND INSPECTIONS REPORTS: TESTING LABORATORY WILL SUBMIT REPORT STATING COMPLIANCE OR NONCOMPLIANCE WITH CONTRACT DOCUMENTS TO OWNER, CONTRACTOR, ARCHITECT (STRUCTURAL ENGINEER) AND, UPON REQUEST, TO GOVERNING CODE AUTHORITY. SEE SPECIFICATIONS FOR ADDITIONAL TEST AND INSPECTION REQUIREMENTS.

4. SEE "STATEMENT OF SPECIAL INSPECTIONS" NOTES ON SHEET S005 FOR ADDITIONAL INFORMATION.

STATEMENT OF SPECIAL INSPECTIONS

1. AN APPROVED AGENCY, RETAINED BY OWNER AND SATISFACTORY TO ARCHITECT (STRUCTURAL ENGINEER) AND GOVERNING CODE AUTHORITY, SHALL PERFORM SPECIAL INSPECTIONS AND TESTS REQUIRED BY THIS CONTRACT AND APPLICABLE CODE. THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS REQUIRED BY CBC SECTION 110. AN APPROVED AGENCY IS AN ESTABLISHED AND RECOGNIZED AGENCY REGULARLY ENGAGED IN CONDUCTING TESTS AND/OR FURNISHING INSPECTION SERVICES, WHERE SUCH AGENCY HAS BEEN APPROVED BY THE GOVERNING CODE AUTHORITY.

2. APPROVED AGENCY SHALL KEEP RECORDS OF ALL SPECIAL INSPECTIONS AND TESTS AND SHALL FURNISH REPORTS OF SPECIAL INSPECTIONS AND TESTS TO THE GOVERNING CODE AUTHORITY AND THE ARCHITECT (STRUCTURAL ENGINEER). REPORTS SHALL INDICATE WHETHER THE WORK INSPECTED OR TESTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE GOVERNING CODE AUTHORITY AND THE ARCHITECT (STRUCTURAL ENGINEER) PRIOR TO THE COMPLETION OF THAT PHASE OF WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND TESTS, AND CORRECTION OF DISCREPANCIES, SHALL BE SUBMITTED UPON COMPLETION OF THAT PHASE OF WORK.

3. WHERE FABRICATION OF STRUCTURAL LOAD-BEARING OR LATERAL LOAD-RESISTING MEMBERS OR ASSEMBLIES IS BEING CONDUCTED ON THE PREMISES OF A FABRICATOR'S SHOP, SPECIAL INSPECTION OF THE FABRICATED ITEMS SHALL BE PERFORMED DURING FABRICATION. SPECIAL INSPECTIONS DURING FABRICATION ARE NOT REQUIRED WHERE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED BY THE GOVERNING CODE AUTHORITY TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. FABRICATOR APPROVAL SHALL BE IN ACCORDANCE TO CBC SECTION 1704.2.5.1.

4. EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A MAIN WIND- OR SEISMIC FORCE-RESISTINGNSYSTEM, DESIGNATED SEISMIC SYSTEM, OR A WIND- OR SEISMIC FORCE-RESISTING COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTION SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE GOVERNING CODE AUTHORITY AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS OF INSPECTION.

5. CONTRACTOR SHALL SUBMIT MATERIAL CERTIFICATION OR LABORATORY TEST REPORTS CERTIFYING MATERIALS ARE OF IDENTIFIABLE TESTED STOCK, COMPLYING WITH PROJECT SPECIFICATIONS, TO OWNER, APPROVED AGENCY, ARCHITECT (STRUCTURAL ENGINEER) AND, UPON REQUEST, TO GOVERNING CODE AUTHORITY. IF LABORATORY TEST REPORTS CANNOT BE MADE AVAILABLE, APPROVED AGENCY WILL PERFORM TESTS AS DIRECTED BY ARCHITECT (STRUCTURAL ENGINEER). CONTRACTOR SHALL PAY FOR COSTS RELATED TO TESTS AND INSPECTIONS OF UNIDENTIFIABLE MATERIALS. MATERIALS FURNISHED WITHOUT LABORATORY TEST REPORTS, MATERIALS FOUND DEFICIENT AFTER INITIAL TESTS AND INSPECTIONS, AND/OR MATERIALS REPLACING DEFICIENT MATERIALS.

6. APPROVED AGENCY SHALL SUBMIT MATERIAL TEST REPORTS INDICATING WHETHER TESTED MATERIALS ARE IN COMPLIANCE OR NONCOMPLIANCE WITH CONTRACT DOCUMENTS TO OWNER, CONTRACTOR, ARCHITECT (STRUCTURAL ENGINEER) AND, UPON REQUEST, TO GOVERNING CODE AUTHORITY.

7. APPROVED AGENCY SHALL PERFORM SPECIAL INSPECTIONS IN ACCORDANCE WITH CBC SECTION 1705 AND WITH THIS SHEET FOR THE FOLLOWING WORK. SEE PROJECT SPECIFICATIONS FOR ADDITIONAL TEST AND INSPECTION REQUIREMENTS.

A. STEEL CONSTRUCTION: SPECIAL INSPECTIONS AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES, AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIRMENTS OF AISC 360 (CHAPTER N):

1. INSPECTION OF WELDING: OBSERVATION OF WELDING OPERATIONS AND VISUAL INSPECTION OF IN-PROGRESS AND COMPLETED WELDS SHALL BE THE PRIMARY METHOD TO CONFIRM THAT MATERIALS, PROCEDURES AND WORKMANSHIP ARE IN CONFORMANCE WITH CONSTRUCTION DOCUMENTS. ALL PROVISIONS OF AWS D1.1/D1.1M AND TABLE 1.1 SHALL APPLY.

a. NONDESTRUCTIVE TESTING (NDT) OF WELDED JOINTS SHALL BE IN ACCORDANCE WITH AISC 360 (CHAPTER N5.5) AS FOLLOWS.

b. PROCEDURES: ULTRASONIC TESTING (UT), MAGNETIC PARTICLE TESTING (MT), PENETRANT TESTING (PT) AND RADIOGRAPHIC TESTING (RT), WHERE REQUIRED, SHALL BE PERFORMED IN ACCORDANCE WITH AWS D1.1/D1.1M CLAUSE 6. ACCEPTANCE CRITERIA SHALL BE IN ACCORDANCE WITH AWS D1.101.1M (TABLE 6.2), UNLESS OTHERWISE DESIGNATED IN THE CONTRACT DOCUMENTS.

c. CJP GROOVE WELD NDT: FOR STRUCTURES IN RISK CATEGORY III OR IV, UT SHALL BE PERFORMED ON ALL CJP GROOVE WELDS SUBJECT TO TRANSVERSELY APPLIED TENSION LOADING IN BUTT, T- AND CORNER JOINTS, IN MATERIALS 5/16" THICK OR GREATER. FOR STRUCTURES IN RISK CATEGORY II, UT SHALL BE PERFORMED ON 10% OF CJP GROOVE WELDS SUBJECT TO TRANSVERSELY APPLIED TENSION LOADING IN BUTT, T- AND CORNER JOINTS, IN MATERIALS 5/16" THICK OR GREATER.

d. ACCESS HOLE NDT: THERMALLY CUT SURFACES OF ACCESS HOLES SHALL BE TESTED USING MT OR PT, WHEN THE FLANGE THICKNESS EXCEEDS 2" FOR ROLLED SHAPES, OR WHEN THE WEB THICKNESS EXCEEDS 2" FOR BUILT-UP SHAPES. ANY CRACK SHALL BE DEEMED UNACCEPTABLE REGARDLESS OF SIZE OR LOCATION.

e. WELDED JOINTS SUBJECT TO FATIGUE: WHEN REQUIRED BY AISC 360, APPENDIX 3, TABLE A-3.1, WELDED JOINTS REQUIRING WELD SOUNDING BY RT OR UT INSPECTION SHALL BE TESTED AS PRESCRIBED IN SECTIONS 6.12.2 OR 6.13.2 OF AWS D1.101.1M. REDUCTION IN THE RATE OF UT IS PROHIBITED.

f. REDUCTION OF RATE OF UT: THE RATE OF UT IS PERMITTED TO BE REDUCED IF APPROVED BY THE ENGINEER OF RECORD AND THE GOVERNING CODE AUTHORITY. WHERE THE INITIAL RATE IS 100%, THE NOT RATE FOR AN INDIVIDUAL WELDER OR WELDING OPERATOR IS PERMITTED TO BE REDUCED TO 25%, PROVIDED THE REJECT RATE (THE NUMBER OF WELDS CONTAINING UNACCEPTABLE DEFECTS DIVIDED BY THE NUMBER OF WELDS COMPLETED) IS DEMONSTRATED TO BE 5% OR LESS OF THE WELDS TESTED FOR THE WELDER OR WELDING OPERATOR. A SAMPLING OF AT LEAST 40 COMPLETED WELDS FOR A JOB SHALL BE MADE FOR SUCH REDUCTION EVALUATION. FOR EVALUATING THE REJECT RATE OF CONTINUOUS WELDS OVER 3 FEET IN LENGTH WHERE THE EFFECTIVE THROAT THICKNESS IS 1 INCH OR LESS, EACH 12-INCH INCREMENT OR FRACTION THEREOF SHALL BE CONSIDERED AS ONE WELD. FOR EVALUATING THE REJECT RATE ON CONTINUOUS WELDS OVER 3 FEET IN LENGTH WHERE THE EFFECTIVE THROAT THICKNESS IS GREATER THAN 1 INCH, EACH 6-INCH OF LENGTH OR FRACTION THEREOF SHALL BE CONSIDERED ONE WELD.

g. INCREASE IN RATE OF UT: FOR STRUCTURES IN RISK CATEGORY II, WHERE THE INITIAL RATE FOR UT IS 10%, THE NOT RATE FOR AN INDIVIDUAL WELDER OR WELDING OPERATOR SHALL BE INCREASED TO 100% SHOULD THE REJECT RATE (THE NUMBER OF WELDS CONTAINING UNACCEPTABLE DEFECTS DIVIDED BY THE NUMBER OF WELDS COMPLETED) EXCEEDS 5% OF THE WELDS TESTED FOR THE WELDER OR WELDING OPERATOR. A SAMPLING OF AT LEAST 20 COMPLETED WELDS FOR A JOB SHALL BE MADE PRIOR TO IMPLEMENTING SUCH AN INCREASE. WHEN THE REJECT RATE FOR THE WELDER OF WELDING OPERATOR, AFTER A SAMPLING OF AT LEAST 40 COMPLETED WELDS, HAS FALLEN TO 5% OR LESS, THE RATE OF UT SHALL BE RETURNED TO 10%. FOR EVALUATING THE REJECT RATE OF CONTINUOUS WELDS OVER 3 FEET IN LENGTH WHERE THE EFFECTIVE THROAT THICKNESS IS 1 INCH OR LESS, EACH 12-INCH INCREMENT OR FRACTION THEREOF SHALL BE CONSIDERED AS ONE WELD. FOR EVALUATING THE REJECT RATE ON CONTINUOUS WELDS OVER 3 FEET IN LENGTH WHERE THE EFFECTIVE THROAT THICKNESS IS GREATER THAN 1 INCH, EACH 6-INCH OF LENGTH OR FRACTION THEREOF SHALL BE CONSIDERED ONE WELD.

h. DOCUMENTATION: ALL NOT PERFORMED SHALL BE DOCUMENTED. FOR SHOP ABRICATION, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY PIECE MARK AND LOCATION IN THE PIECE. FOR FIELD WORK, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY LOCATION IN THE STRUCTURE, PIECE MARK, AND THE LOCATION IN THE PIECE.

2. OTHER STRUCTURAL STEEL INSPECTION TASKS:

a. APPROVED AGENCY SHALL BE ON THE PREMISES DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH CONSTRUCTION DOCUMENTS. AS A MINIMUM, THE DIAMETER, GRADE, TYPE AND LENGTH OF ANCHOR RODS OR EMBEDDED ITEMS, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO CONCRETE, SHALL BE VERIFIED PRIOR TO PLACEMENT OF CONCRETE.

b. APPROVED AGENCY SHALL INSPECT THE PREFABRICATED OR ERECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATION AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.

3. COLD-FORMED STEEL DECK: SPECIAL INSPECTIONS AND QUALIFICATIONS OF WELDING SPECIAL INSPECTORS FOR COLD-FORMED STEEL FLOOR AND ROOF DECKS SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF SDI Q4/QC.

B. CONCRETE/SHOTCRETE CONSTRUCTION: SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE TO CBC SECTION 1705.3 AND TABLE 2.

a. SPECIAL INSPECTIONS OF WELDING AND QUALIFICATIONS OF SPECIAL INSPECTORS FOR REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF AWS D1.4 FOR SPECIALS INSPECTION AND OF AWS D1.4 FOR SPECIAL INSPECTOR QUALIFICATION.

b. MATERIAL TESTS: IN THE ABSENCE OF SUFFICIENT DATA OR DOCUMENTATION PROVIDING EVIDENCE OF CONFORMANCE TO QUALITY STANDARDS FOR MATERIALS IN CHAPTERS 19 AND 20 OF AC 318, THE GOVERNING AGENCY SHALL REQUIRE TESTING OF MATERIALS IN ACCORDANCE WITH THE APPROPRIATE STANDARDS AND CRITERIA FOR THE MATERIAL IN CHAPTERS 19 AND 20 OF AC 318.

C. STRUCTURAL MASONRY CONSTRUCTION: SPECIAL INSPECTIONS AND TESTS OF MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE TO TMS 402/602 AND THE FOLLOWINGS:

1. FOR MASONRY IN RISK CATEGORY I, II, OR III STRUCTURES SEE TABLE 3.1 - LEVEL 2 QUALITY ASSURANCE

2. FOR MASONRY IN RISK CATEGORY IV STRUCTURES SEE TABLE 3.2 - LEVEL 3 QUALITY ASSURANCE

3. SELF-CONSOLIDATING GROUT IS NOT PERMITTED WITHOUT THE EXPRESS APPROVAL OF THE ARCHITECT (STRUCTURAL ENGINEER).

D. WOOD CONSTRUCTION: SPECIAL INSPECTION OF WOOD CONSTRUCTION SHALL BE IN ACCORDANCE TO THE FOLLOWING:

1. FABRICATION PROCESS OF PREFABRICATED WOOD STRUCTURAL ELEMENTS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH NOTE 3 HEREINABOVE.

E. SOILS: SPECIAL INSPECTIONS AND TESTS OF EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND LOAD-BEARING REQUIREMENTS SHALL BE PERFORMED IN ACCORDANCE WITH TABLE 4. THE APPROVED GEOTECHNICAL REPORT AND THE CONSTRUCTION DOCUMENTS SHALL BE USED TO DETERMINE COMPLIANCE.

1. DURING FILL PLACEMENT THE APPROVED AGENCY SHALL VERIFY THAT PROPER MATERIALS AND PROCEDURES ARE USED IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT.

F. CAST-IN-PLACE DEEP FOUNDATIONS: SPECIAL INSPECTIONS AND TESTS SHALL BE PERFORMED DURING THE INSTALLATION CAST-IN-PLACE DEEP FOUNDATION ELEMENTS DEEP FOUNDATION ELEMENTS IN ACCORDANCE WITH THE TABLE 6. THE APPROVED GEOTECHNICAL REPORT AND THE CONSTRUCTION DOCUMENTS SHALL BE USED TO DETERMINE COMPLIANCE.

PERFORMANCE CRITERIA FOR PV PANEL FRAMING AND ATTACHMENT DESIGN (DESIGN BUILD)

7. THE SOLAR PANEL FRAMING AND MOUNTING ENGINEER SHALL PROVIDE ALL TESTING, ANALYSIS, DATA, AND CALCULATIONS REQUIRED BY EOR, THE JURISDICTION FOR THE JUSTIFICATION AND PERMITTING OF THEIR SYSTEM AND THE COMPONENTS USED FOR THE PROJECT.

8. THE PANELS SHALL BE LOCATED AS SHOWN ON THE DESIGN DRAWINGS.

9. AT CONNECTIONS TO STRUCTURE, PROVIDE STABILIZING ELEMENTS TO RESTRAIN AND PROTECT STRUCTURAL MEMBERS AGAINST INDUCED TWISTING, OR WARPING AS A RESULT OF THE CONNECTION. PROVIDE MATERIAL AND INSTALL STABILIZING ELEMENTS AT NO ADDITIONAL COST TO OWNER. CONNECTIONS SHALL BE TO A FRAMING MEMBER, CONNECTION TO THE BARE DECK IS NOT ALLOWED.

10. SUBMIT SHOP DRAWINGS AND STRUCTURAL CALCULATIONS AND THE REQUIRED TEST DATA SIGNED BY AND BEARING THE SEAL OF A REGISTERED STRUCTURAL ENGINEER IN CALIFORNIA TO ARCHITECT OR STRUCTURAL ENGINEER FOR REVIEW AND TO GOVERNING AUTHORITY FOR APPROVAL PRIOR TO START OF WORK. CALCULATIONS AND SHOP DRAWINGS SHALL BE COMPLETE INCLUDING FRAMING, CONNECTIONS AND ALL REQUIRED DETAILS AND TEST DATA.

PREFABRICATED WOOD TRUSSES DESIGN-BUILT, DEFERRED APPROVAL)

1. DESIGN RESPONSIBILITY: CONTRACTOR TO DESIGN AND OBTAIN SEPARATE GOVERNING CODE AUTHORITY APPROVAL FOR MATERIALS OF THIS SECTION PRIOR TO FABRICATION WITH ICC.

2. MANUFACTURER: HENSONTRUSS INC./TRUTRUS.

3. FABRICATE TRUSSES WITH STRUCTURAL WOOD WEBS AND CHORDS AND METAL PLATE CONNECTIONS.

4. PROVIDE DOUGLAS FIR STRUCTURAL WOOD WEBS AND CHORDS OF STRESS GRADED LUMBER COMPLYING WITH STANDARD GRADING RULES NO. 16 OF WEST COAST LUMBER INSPECTION BUREAU. PROVIDE AIR DRY LUMBER WITH A 19% MAXIMUM MOISTURE CONTENT AT TIME OF FABRICATION. DESIGN STRESS VALUES SHALL BE AS ASSIGNED BY GOVERNING CODE AUTHORITY FOR STRESS GRADE USED.

5. PROVIDE METAL CONNECTOR PLATES COMPLYING WITH "DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES", TPI-85 OF TRUSS PLATE INSTITUTE AND CBC SECTION 2303.4.2. MANUFACTURE TRUSS CONNECTOR PLATES FROM ASTM A446, GRADE A, PRIME COMMERCIAL QUALITY GALVANIZED SHEET STEEL OF NO. LESS THAN 20 GAUGE THICKNESS HAVING A MINIMUM YIELD OF 33,000 PSI AND A MINIMUM ULTIMATE TENSILE STRENGTH OF 45,000 PSI. GALVANIZE WITH G60 COMMERCIAL COATING COMPLYING WITH ASTM A525 OR ELECTROLYTICALLY ZINC-COAT WITH A CLASS C COATING COMPLYING WITH ASTM A591.

6. DESIGN AND FABRICATE TRUSSES TO SAFELY SUPPORT SUPERIMPOSED DEAD, LIVE AND LATERAL LOADS; SPECIAL LOADINGS SUCH AS MECHANICAL EQUIPMENT AND TO ACCOMMODATE SPECIAL FRAMING CONDITIONS. DESIGN TRUSS LOADS BEARING ON BEAMS OR TOP PLATES OF WALLS NOT EXCEEDING ALLOWABLE BEARING AND BENDING STRESSES FOR THOSE MEMBERS.

7. DESIGN LOADS: AS INDICATED ON PLANS

a. ALL TRUSSES SHALL BE DESIGNED TO CARRY THE FOLLOWING GRAVITY LOADS

ROOF NOTE: ALL DEAD LOADS MUST BE INCREASED TO ACCOUNT FOR ROOF SLOPE PER ARCH.

b. ALL TRUSSES SHALL BE DESIGNED FOR THE COMPLETE LOAD PATH FOR TRANSMITTING WIND & SEISMIC FORCES. BETWEEN LATERAL FORCE RESISTING ELEMENTS (i.e. CONNECTIONS BETWEEN DIAPHRAGMS & SHEAR WALLS.

c. CONSIDERATION SHALL BE GIVEN IN THE DESIGN OF THE TRUSSES TO LOADING IMPARTED BY TRUSSES WALL ANCHORAGE & ATTACHMENTS. SEE PLANS FOR ADDITIONAL LOAD CRITERIA.

8. LIMIT DEFLECTIONS TO THE FOLLOWING:

DEAD LOAD PLUS LIVE LOAD(L/240)

LIVE LOAD(L/360)

9. SUBMIT SHOP DRAWINGS AND STRUCTURAL CALCULATIONS SIGNED BY AND BEARING SEAL OF A REGISTERED CIVIL OR STRUCTURAL ENGINEER TO ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW AND GOVERNING CODE AUTHORITY FOR APPROVAL. INDICATE TRUSS CONFIGURATIONS; DESIGN LOADING AND ALLOWABLE STRESS INCREASE; SIZE, SPECIES AND STRESS GRADE OF LUMBER; CAMBERS; BRIDGING AND BRACING; JOINT DETAILS AND DETAILS AT TRUSS SUPPORTS.

10. OBTAIN GOVERNING CODE AUTHORITY APPROVAL OF CALCULATIONS AND SHOP DRAWINGS PRIOR TO FABRICATION.

11. BEAR ANCHOR PLATES OR BEARING CLIPS FULLY ON SUPPORTS WITH NO MORE THAN 1/4-INCH OVERHANG.

12. KEEP TRUSSES UPRIGHT AT ALL TIMES. PLUMB TRUSSES AND HOLD CHORDS IN A STRAIGHT LINE FROM THE TIME TRUSSES ARE SET IN PLACE UNTIL SHEATHING IS APPLIED.

13. PROVIDE CROSS BRIDGING, BRACING, OR BOTH AS REQUIRED TO ADEQUATELY BRACE TRUSSES. INSTALL BRIDGING AS ERECTION PROCEEDS AND INSTALL TEMPORARY BRACING TO MAINTAIN ALIGNMENT AND PREVENT LATERAL MOVEMENT. DESIGN AND PROVIDE TRUSS BRACING COMPLYING WITH THE TRUSS PLATE INSTITUTE RECOMMENDATIONS.

14. SECURELY FASTEN SHEATHING TO TOP CHORD. STAGGER NAILING TO AVOID SPLITTING AND TO ENSURE NAILING INTO EACH CHORD MEMBER.

15. DO NOT DRILL OR CUT WEB OR CHORD MEMBERS.

GENERAL NOTES

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

THE ABOVE DRAWINGS AND SPECIFICATIONS ARE THE SOLE DESIGN AND ARRANGEMENTS REPRESENTED BY THE ARCHITECT AND ENGINEER. THEY REMAIN THE PROPERTY OF THE ARCHITECT AND ENGINEER. NO PARTS OF THESE DRAWINGS OR SPECIFICATIONS SHALL BE COPIED, DISCLOSED TO OTHERS OR USED IN CONNECTION WITH ANY WORK OR PROJECT OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY WERE PREPARED, AND DEVELOPED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT. VISUAL CONTACT WITH THESE DRAWINGS OR SPECIFICATIONS SHALL BE PROHIBITED. THESE DRAWINGS SHALL BE KEPT IN THE ARCHITECT'S OFFICE AND SHALL BE MAINTAINED IN ACCORDANCE WITH THE ARCHITECT'S POLICY. THESE DRAWINGS SHALL BE KEPT IN THE ARCHITECT'S OFFICE AND SHALL BE MAINTAINED IN ACCORDANCE WITH THE ARCHITECT'S POLICY. THESE DRAWINGS SHALL BE KEPT IN THE ARCHITECT'S OFFICE AND SHALL BE MAINTAINED IN ACCORDANCE WITH THE ARCHITECT'S POLICY.

Date	Issue Date
Drawn	
Checked	
Scale	AS NOTED
Job No.	Project Number

S005

ADDENDUM 5

WILLIAM LOYD JONES
ARCHITECT

SO
saif-bouquet
structural engineers

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

PLAN CHECK SUBMITTAL - October 31 2025

TABLE 3.2 -LEVEL 3 QUALITY ASSURANCE (TMS 402 TMS 602 TABLE 4)		
MINIMUM TEST		
VERIFICATION OF <i>f</i> m IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.4 B PRIOR TO CONSTRUCTION, AND FOR EVERY 5,000 SQ FT. (465 Aq.M) DURING CONSTRUCTION		
VERIFICATION OF PROPERTIES OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, AND GROUT OTHER THAN SELF-CONSOLIDATING GROUT, AS DELIVERED TO THE PROJECT SITE		
INSPECTION TASKS	CONTINUOUS	PERIODIC
1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE		
a. PROPORTIONS OF SITE-PREPARED MORTAR		X
b. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES		X
c. GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES		X
d. PRESTRESSING TECHNIQUE		X
e. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	X	X
f. SAMPLE PANEL CONSTRUCTION	X	
2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:		
a. GROUT SPACE	X	
b. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGE		X
c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS	X	
d. PROPERTIES OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS		X
3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION:		
a. MATERIALS AND PROCEDURES WITH APPROVED SUBMITTALS		X
b. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION		X
c. SIZE AND LOCATION OF STRUCTURAL MEMBERS		X
d. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	X	
e. WELDING OF REINFORCEMENT	X	
f. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40 ° F (4.4°C)) OR HOT WEATHER (TEMPERATURE ABOVE 90° F (32.2°C))		X
g. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	X	
h. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	X	
i. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS.	X	
4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	X	
a. REQUIRED FOR THE FIRST 5,000 SFT OF AAC MASONRY		
b. REQUIRED AFTER THE FIRST 5,000 SFT OF AAC MASONRY		

TABLE 12.2 (AISC 360 TABLE N5.6-2)	INSPECTION TASKS DURING BOLTING	TASK	DOC
1. FASTENER ASSEMBLIES, PLACED IN ALL HOLES AND WASHERS AND NUTS (IF REQUIRED) ARE POSITIONED AS REQUIRED		O	-
2. JOINT BROUGHT TO THE SNUG-TIGHT CONDITION, PRIOR TO THE PRETENSIONING OPERATION		O	-
3. FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING		O	-
4. FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES		O	-

TABLE 12.3 (AISC 360 TABLE N5.6-3)	INSPECTION TASKS AFTER BOLTING	TASK	DOC
1. DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS		P	-

TABLE 2 (CBC TABLE 1705.3) REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION		
INSPECTION TASKS	CONTINUOUS	PERIODIC
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT	-	X
2. REINFORCING BAR WELDING:		
a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706;	-	X
b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND	-	X
c. INSPECT ALL OTHER WELDS.	X	-
3. INSPECT ANCHORS CAST IN CONCRETE.	-	X
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS ^a :		
a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	X	-
b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.	-	X
5. VERIFY USE OF REQUIRED DESIGN MIX.	-	X
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TEST, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X	-
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X	-
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	-	X
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	-	X
11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	-	X
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	X

TABLE 4 (CBC TABLE 1705.6) REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS		
INSPECTION TASK	CONTINUOUS	PERIODIC
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	-
5. PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	X

TABLE 3 (CBC SECTION 1705.4) REQUIRED SPECIAL INSPECTIONS AND TESTS OF MASONRY CONSTRUCTION		
TABLE 3.1-LEVEL 2 QUALITY ASSURANCE (TMS TMS 602 TABLE 4)		
MINIMUM TEST		
VERIFICATION OF <i>f</i> m IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.4 B PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED THIS CODE		
INSPECTION TASKS	CONTINUOUS	PERIODIC
1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE		
a. PROPORTIONS OF SITE-PREPARED MORTAR		X
b. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES		X
c. GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES		X
d. PRESTRESSING TECHNIQUE		X
e. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	X	X
f. SAMPLE PANEL CONSTRUCTION		X
2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:		
a. GROUT SPACE		X
b. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGE		X
c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS		X
d. PROPERTIES OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS		X
3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION:		
a. MATERIALS AND PROCEDURES WITH APPROVED SUBMITTALS		X
b. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION.		X
c. SIZE AND LOCATION OF STRUCTURAL MEMBERS		X
d. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION		X
e. WELDING OF REINFORCEMENT	X	
f. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40 ° F (4.4°C)) OR HOT WEATHER (TEMPERATURE ABOVE 90° F (32.2°C))		X
g. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	X	
h. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	X	X
i. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS.		X
4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS		X
a. REQUIRED FOR THE FIRST 5,000 SFT OF AAC MASONRY		
b. REQUIRED AFTER THE FIRST 5,000 SFT OF AAC MASONRY		

TABLE 1 (CBC SECTION 1705.2) REQUIRED SPECIAL INSPECTION OF STRUCTURAL STEEL CONSTRUCTION			
TABLE 1.1 INSPECTION OF WELDING			
TABLE 1.1.1 (AISC 360 TABLE N5. 4-1)	INSPECTION TASKS PRIOR TO WELDING	TASK	DOC
1. WELDING PROCEDURES SPECIFICATIONS (WPSs) AVAILABLE		O	-
2. WELDING PROCEDURES SPECIFICATIONS (WPSs) AVAILABLE		P	-
3. MAUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE		P	-
4. MATERIAL IDENTIFICATION (TYPE, GRADE)		O	-
5. WELDER IDENTIFICATION SYSTEM		O	-
6. FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)			
A. JOINT PREPARATION			
B. DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)			
C. CLEANLINESS (CONDITION OF STEEL SURFACES)	O		-
D. TACKING (TACK WELD QUALITY AND LOCATION)			
E. BACKING TYPE AND FIT (IF APPLICABLE)			
7. FIT-UP OF C/P GROOVE WELDS OF HSS T-, Y- AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY)			
• JOINT PREPARATIONS			
• DIMENSIONS (ALIGNMENT, ROOT FACE, BEVEL)			
• CLEANLINESS (CONDITION OF STEEL SURFACES)	-		P
• TACKING (TACK WELD QUALITY AND LOCATION)			
8. CONFIGURATION AND FINISH OF ACCESS HOLES		O	-
9. FIT-UP OF FILLET WELDS			
A. DIMENSIONS (ALIGNMENT, GAPS AT ROOT)			
B. CLEANLINESS (CONDITION OF STEEL SURFACE)	O		-
C. TACKING (TACK WELD QUALITY AND LOCATION)			
THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.			
TABLE 1.1.2 (AISC 360 TABLE N5. 4-2)	INSPECTION TASK DURING WELDING	TASK	DOC
1. USE OF QUALIFIED WELDERS		O	-
2. CONTROL AND HANDLING OF WELDING CONSUMABLES			
A. PACKAGING		O	-
B. EXPOSURE CONTROL			
3. NO WELDING OVER CRACKED TACK WELDS		O	-
4. ENVIRONMENTAL CONDITIONS			
A. WIND SPEED WITHIN LIMITS		O	-
B. PRECIPITATION AND TEMPERATURE			
5. WPS FOLLOWED			
A. SETTINGS ON WELDING EQUIPMENT			
B. TRAVEL SPEED			
C. SELECTED WELDING MATERIALS		O	-
D. SHIELDING GAS TYPE/FLOW RATE			
E. PREHEAT APPLIED			
F. INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.)			
G. PROPER POSITION (F, V, H, OH)			
6. WELDING TECHNIQUES			
A. INTERPASS AND FINAL CLEANING		O	-
B. EACH PASS WITHIN PROFILE LIMITATIONS			
C. EACH PASS MEETS QUALITY REQUIREMENTS			
7. PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS		-	P
TABLE 1.1.3 (AISC 360 TABLE N5. 4-3)	INSPECTION TASKS AFTER WELDING	TASK	DOC
1. WELDS CLEANED		O	-
2. SIZE, LENGTH AND LOCATION OF WELDS		P	-
3. WELDS MEET VISUAL ACCEPTANCE CRITERIA			
A. CRACK PROHIBITION			
B. WELDBASE-METAL FUSION			
C. CRATER CROSS SECTION			
D. WELD PROFILES		P	-
E. WELD SIZE			
F. UNDERCUT			
G. POROSITY			
4. ARC STRIKES		P	-
5. <i>k</i> -AREA ^(a)		P	-
6. WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES ^(b)		P	-
7. BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)		P	-
8. REPAIR ACTIVITIES		P	-
9. DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER		P	-
10. NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR		O	O
^(a) WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE <i>k</i> -AREA, VISUALLY INSPECT THE WEB <i>k</i> -AREA FOR CRACKS WITHIN 3 IN. OF WELD			
^(b) AFTER ROLLED HEAVY SHAPES (SEE SECTION A3.1c) AND BUILT-UP HEAVY SHAPES (SEE A3.1d) ARE WELDED VISUALLY INSPECTION THE WELD ACCESS HOLE FOR CRACKS.			

TABLE 1.2 INSPECTION OF BOLTING			
TABLE 1.2.1 (AISC 360 TABLE N5.6-1)	INSPECTION TASKS PRIOR TO BOLTING	TASK	DOC
1. MANUFACTURE'S CERTIFICATION AVAILABLE FOR FASTENER MATERIALS		P	-
2. FASTENERS MARKED IN ACCORDANACE WITH ASTM REQUIREMENTS		O	-
3. PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF HEADS ARE TO BE EXCLUDED FROM SHEAR PLANE)		O	-
4. PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL		O	-
5. CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE PAYING SURFACE CONDITION AND HOLE PREPARATION , IF SPECIFIED, MEET APPLICABLE REQUIREMENTS		O	-
6. PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED		O	-
7. PROTECTED STORAGE PROVIDED FOR BOLTS, NUTS AND OTHER FASTENER COMPONENTS		O	-

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90230
90230
90230

TEL 310 392 3995

SO
saiful-bouquet
structural engineers

725 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

STATEMENT OF SPECIAL
INSPECTIONS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S3289
Exp. 12/31/27
STATE OF CALIFORNIA
STRUCTURAL

ADDENDUM 5

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SO006

ADDENDUM 5

ABBREVIATIONS	
#	POUNDS, NUMBER
&	AND
<	LESS THAN
>	GREATER THAN
@	AT
°	DEGREE
±	PLUS OR MINUS
≤	LESS THAN OR EQUAL TO
≥	GREATER THAN OR EQUAL TO

A	
AA	ADHESIVE ANCHOR
AB	ANCHOR BOLT(S)
ABV	ABOVE
ADDL	ADDITIONAL
ADDN	ADDITION
ADJ	ADJACENT, ADJUSTABLE
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
ALT	ALTERNATE
ANCH	ANCHOR
APPROX	APPROXIMATE
AR	ALL AROUND
ARCH	ARCHITECTURAL

B	
BAL	BALANCE
BC	BOTTOM CHORD
BE	BOUNDARY ELEMENT
BEL	BELOW
BLDG	BUILDING
BLKG	BLOCKING
BLL	BOTTOM LOWER LAYER
BM	BEAM
BN	BOUNDARY NAILING
BO	BOTTOM OF
BOBP	BOTTOM OF BASE PLATE
BOS	BOTTOM OF STEEL
BOT	BOTTOM
BP	BASE PLATE
BPL	BEARING PLATE
BRB	BUCKLING-RESTRAINED BRACE
BRBF	BUCKLING-RESTRAINED BRACED FRAME
BRCG	BRACING
BRDG	BRIDGING
BRG	BEARING
BS	BOTH SIDES
BSMT	BASEMENT
BTWN	BETWEEN
BU	BUILT-UP
BUL	BOTTOM UPPER LAYER
BYD	BEYOND

C	
CA	CAMBER
CA	COLUMN ABOVE
CANT	CANTILEVER
CB	COLUMN BELOW
CC	CENTER TO CENTER
CF	CUBIC FEET
CHKD	CHECKERED
CIP	CAST-IN-PLACE
CJ	CONSTRUCTION JOINT
CJP	COMPLETE JOINT PENETRATION
CL	CENETERLINE
CLG	CEILING
CLR	CLEAR
CLSM	CONTROL LOW STRENGTH MATERIAL
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUOUS, CONTINUITY
CONTR	CONTRACTOR
COORD	COORDINATE, COORDINATES
CTR	CENTER
CTRL JT	CONTROL JOINT
CVN	CHARPY V-NOTCH
CY	CUBIC YARD
CYL	CYLINDER

ABBREVIATIONS	
D	
DBA	DEFORMED BAR ANCHOR
DBL	DOUBLE
DBLR	DOUBLER
DEG	DEGREE
DEGF	DEGREE FAHRENHEIT
DEPR	DEPRESS, DEPRESSED, DEPRESSION
DET	DETAIL
DF-L	DOUGLAS FIR - LARCH
DIA	DIAMETER
DIAG	DIAGONAL
DIAPH	DIAPHRAGM
DIM	DIMENSION
DL	DEAD LOAD
DN	DOWN
DO	DITTO
DWG	DRAWING
DWL	DOWEL

E	
(E)	EXISTING
EA	EACH
EB	EXPANSION (ANCHOR) BOLT
EBF	ECCENTRICALLY BRACED FRAME
EF	EACH FACE
EFF	EFFECTIVE
EJ	EXANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
EMBD	EMBEDMENT, EMBED
EN	EDGE NAILING
ENGR	ENGINEER
EOR	ENGINEER OF RECORD
EOS	EDGE OF SLAB
EPL	EMBEDDED PLATE
EQ	EQUAL
EQUIP	EQUIPMENT
ES	EACH SIDE
ESC	ESCALATOR
EW	EACH WAY
EWTB	EACH WAY TOP AND BOTTOM
EXC	EXCAVATE
EXP	EXPANSION
EXT	EXTERIOR

F	
F	FAHRENHEIT
FAB	FABRICATE, FABRICATION
FCAW	FLUX CORED ARC WELDING
FDN	FOUNDATION
FF	FAR FACE
FIN	FINISH
FJ	FLOOR JOIST
FLG	FLANGE
FLR	FLOOR
FN	FIELD NAILING
FO	FACE OF
FOC	FACE OF CONCRETE
FOF	FACE OF FINISH
FOGB	FACE OF GYPSUM BOARD
FOS	FACE OF STUD
FOW	FACE OF WALL
FP	FIREPROOF, FIREPROOFING
FRMG	FRAMING
FS	FAR SIDE
FT	FOOT, FEET, FLUSH TOP
FTG	FOOTING
FUT	FUTURE

G	
GA	GAGE, GAUGE
GALV	GALVANIZED
GEN	GENERAL
GFRC	GLASS FIBER REINFORCED CONCRETE
GLB	GLUE-LAMINATED BEAM
GMAW	GAS METAL ARC WELDING
GOL	GAGE OF ANGLE
GR	GRADE
GRTG	GRATING
GT	GROUND

ABBREVIATIONS	
H	
HAZ	HEATED AFFECTED ZONE
HCA	HEADED CONCRETE ANCHOR
HDB	HEADED DEFORMED BAR
HDG	HOT DIPPED GALVANIZED
HDR	HEADER
HGR	HANGER
HI, (HI)	HIGH
HORIZ, (H)	HORIZONTAL
HP	HIGH POINT
HR	HANDRAIL
HS	HIGH STRENGTH
HSB	HIGH STRENGTH BOLT
HT	HEIGHT

I	
I.F	INSIDE FACE
ICC-ES	INTERNATIONAL CODE COUNCIL EVALUATION SERVICE
ID	INSIDE DIAMETER
IE	INVERT ELEVATION
IMF	INTERMEDIATE MOMENT FRAME
IN	INCH
INFO	INFORMATION
INSP	INSPECTION, INSPECTOR
INSU	INSULATING
INT	INTERIOR
INTER	INTERMEDIATE
IRMSW	INTERMEDIATE REINFORCED MASONRY SHEAR WALL

J	
JST	JOIST
JT	JOINT

K	
K	KIP (KILOPOUND)/(1000 POUNDS)
KSF	KIP PER SQUARE FOOT
KSI	KIP PER SQUARE INCH

L	
LAM	LAMINATED
LB	LAG BOLT, POUND
LG	LONG
LL	LIVE LOAD
LLBB	LONG LEG BACK TO BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LNDG	LANDING
LNTL	LINTEL
LO, (LO)	LOW
LONGIT	LONGITUDINAL
LP	LOW POINT
LSH	LONG SLOTTED HOLE
LWT	LIGHTWEIGHT
LVL	LAMINATED VENEER LUMBER
LWC	LIGHTWEIGHT CONCRETE

M	
MAX	MAXIMUM
MB	MACHINE BOLT
MC	MOMENT CONNECTION
MECH	MECHANICAL
MEMB	MEMBER, MEMBRANE
MEZZ	MEZZANINE
MFR	MANUFACTURE(R)
MIN	MINIMUM
MISC	MISCELLANEOUS
MOV	MOVABLE
MR	MILD REINFORCED, MILD REINFORCING
MT	MAGNETIC PARTICLE TESTING
MTL	METAL
MWFRS	MAIN WIND-FORCE RESISTING SYSTEM

N	
(N)	NEW
NDT	NON-DESTRUCTIVE TESTING
NF	NEAR FACE
NIC	NOT IN CONTRACT
NIP	NOT IN PERMIT
NO	NUMBER, NORTH
NOM	NOMINAL
NS	NEAR SIDE
NTS	NOT TO SCALE
NWC	NORMAL WEIGHT CONCRETE

ABBREVIATIONS	
O	
O.F	OUTSIDE FACE
O/	OVER
OC	ON CENTER
OCBF	ORDINARY CONCENTRICALLY BRACED FRAME
OD	OUTSIDE DIAMETER
OH	OPPOSITE HAND
OMF	ORDINARY MOMENT FRAME
OPNG	OPENING
OPP HD	OPPOSITE HAND
ORCSW	ORDINARY REINFORCED CONCRETE SHEAR WALL
ORMSW	ORDINARY REINFORCED MASONRY SHEAR WALL
OSB	ORIENTED STRAND BOARD
OVS	OVERSIZED
OZ	OUNCE

P	
P/C	PRECAST
PAF	POWDER ACTUATED FASTENER
PAR	PARALLEL
PC	PIECE, PILECAP
PCF	POUNDS PER CUBIC FOOT
PERP	PERPENDICULAR
PJ	POUR JOINT
PJP	PARTIAL JOINT PENETRATION
PL	PLATE
PLATF	PLATFORM
PLCS	PLACES
PLF	POUNDS PER LINEAR FOOT
PLMB	PLUMBING
PLWD	PLYWOOD
POT	POINT OF TANGENCY
PQR	PROCEDURE QUALIFICATION RECORD
PREFAB	PREFABRICATED
PRKG	PARKING
PROJ	PROJECTION
PS	PRESTRESS(ED)
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PSL	PARALLEL STRAND LUMBER
PT	POST-TENSION(ED), LIQUID PENETRANT TESTING
PTDF	PRESSURE TREATED DOUGLAS FIR
PWJ	PLYWOOD WEB JOIST

R	
PAF	POWER ACTUATED FASTENER
R	RADIUS, RISER
RAD	RADIANS
RBS	REDUCED BEAM SECTION
REF	REFERENCE
REINF	REINFORCING
REMV	REMOVABLE, REMOVE
REQD	REQUIRED
RET	RETURN
RF	ROOF
RJ	ROOF JOIST
ROTN	ROTATION
RT	RADIOGRAPHIC TESTING
RTNG	RETAINING

S	
SA	SCREW ANCHOR
SAD	SEE ARCHITECTURAL DRAWING(S)
SAW	SUBMERDGED ARC WELDING
SCBF	SPECIAL CONCENTRICALLY BRACED FRAME
SCHED	SCHEDULE
SCL	STRUCTURAL COMPOSITE LUMBER
SECT	SECTION
SEOR	STRUCTURAL ENGINEER OF RECORD
SEP	SEPARATION
SF	SQUARE FEET
SHT	SHEET
SHTHG	SHEATHING
SIM	SIMILAR
SL	SLOPE
SLBB	SHORT LEG BACK TO BACK
SLRS	SEISMIC LOAD RESISTING SYSTEM
SLV	SLEEVE

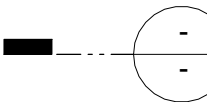

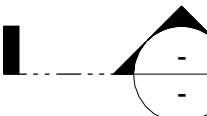
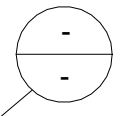
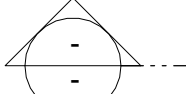
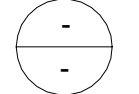

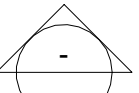
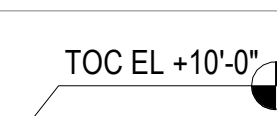
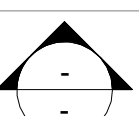
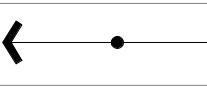
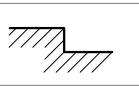
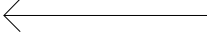

ABBREVIATIONS	
SMAW	SHIELDED METAL ARC WELDING
SMF	SPECIAL MOMENT FRAME
SMS	SHEET METAL SCREW
SO	SOUTH
SOF	SOFFIT
SOG	SLAB-ON-GRADE
SOMD	SLAB ON METAL DECK
SPEC	SPECIFICATIONS, SPECIAL
SPSW	SPECIAL PLATE SHEAR WALL
SQ	SQUARE
SRCSW	SPECIAL REINFORCED CONCRETE SHEAR WALL
SRMSW	SPECIAL REINFORCED MASONRY SHEAR WALL
SS	STAINLESS STEEL
SSH	SHORT SLOTTED HOLE
STA	STATION
STAG	STAGGER
STD	STANDARD
STIF	STIFFENER
STIR	STIRRUP
STL	STEEL
STMF	SPECIAL TRUSS MOMENT FRAME
STRUCT	STRUCTURAL
SW	STUD WELDING
SWBC	SHEAR WALL BOUNDARY COLUMN
SYMM	SYMMETRY

T	
T	TREAD, TOP
T&B	TOP AND BOTTOM
T.O	TOP OF
T/	TOP OF
TAR	TYPICAL ALL AROUND
TC	TOP CHORD
TEMP	TEMPORARY, TEMPERATURE
THD	THREAD
THK	THICK, THICKNESS
THRU	THROUGH
TLL	TOP LOWER LAYER
TOBS	TOP OF BUILT-UP SLAB
TOC	TOP OF CONCRETE
TOD	TOP OF STEEL DECK
TOF	TOP OF FOOTING
TOG	TOP OF GRATING
TOPC	TOP OF PILE CAP
TOS	TOP OF STEEL
TOW	TOP OF WALL
TUL	TOP UPPER LAYER
TYP	TYPICAL

U	
UNO	UNLESS NOTED OTHERWISE
UT	ULTRASONIC TESTING

V	
VERT, (V)	VERTICAL
VIF	VERIFY IN FIELD

W	
W/	WITH
W/O	WITHOUT
WD	WOOD
WF	WIDE FLANGE
WL	WORK LINE
WP	WORK POINT
WPS	WELD PROCEDURE SPECIFICATIONS
WSP	WOOD STRUCTURAL PANEL
WT	WEIGHT
WWR	WELDED WIRE REINFORCEMENT

	DETAIL SECTION		DETAIL CALLOUT OR PARTIAL PLAN CALLOUT
	FULL HEIGHT WALL SECTION		DETAIL CALLOUT
	PARTIAL HEIGHT BUILDING SECTION		DETAIL CALLOUT
	FULL HEIGHT BUILDING SECTION		WALL FRAME ELEVATION
	SPOT ELEVATION		BUILDING ELEVATION
	EXTENT ARROWS		DIFFERENCE IN ELEVATION
	SLOPE ARROW		MATCHLINE

WILLIAM LOYD JONES

ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

so

saiful-bouquet

structural engineers

726 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

GENERAL NOTES

ABBREVIATIONS

FIRE STATION 46

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Date

Issue Date

Drawn

Checked

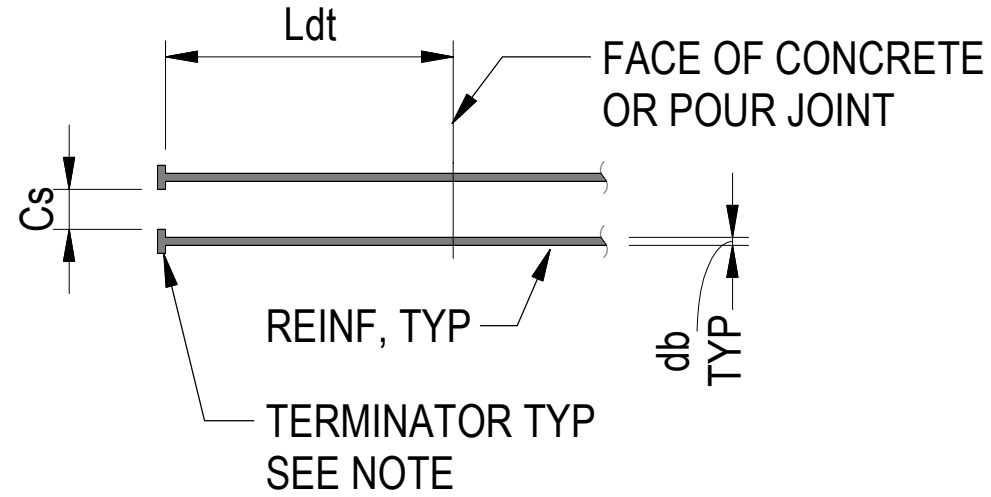
Scale

AS NOTED

Job. No.

Project Number

S008



NOTES:

1. CONCRETE SHALL BE NORMAL WEIGHT
2. HEADED ANCHORS SHALL BE:
 - A. LENTON TERMINATORS TYPE D6 (ICC ESR-3967)
 - B. HRC555 TERMINATORS (ICC ESR-2935)
3. ALL HEADED ANCHORS SHALL MEET THE REQUIREMENTS OF HA HEADS IN ASTM A970.
4. MULTIPLY THE DEVELOPMENT LENGTH L_{dt} IN SCHEDULE BY 1.33 FOR REBAR YIELD STRENGTH OF 80 KSI.
5. WHERE SIDE COVER (NORMAL TO PLANE OF HOOK) IS LESS THAN $2\frac{1}{2}$ " FOR BARS TERMINATING INSIDE A COLUMN, OR WHERE SIDE COVER NORMAL TO THE PLANE OF HOOK IS LESS THAN 6DB , L_{dt} MUST BE INCREASED BY 25%.
6. FOR EPOXY COATED BARS INCREASE DEVELOPMENT LENGTH BY 20%.

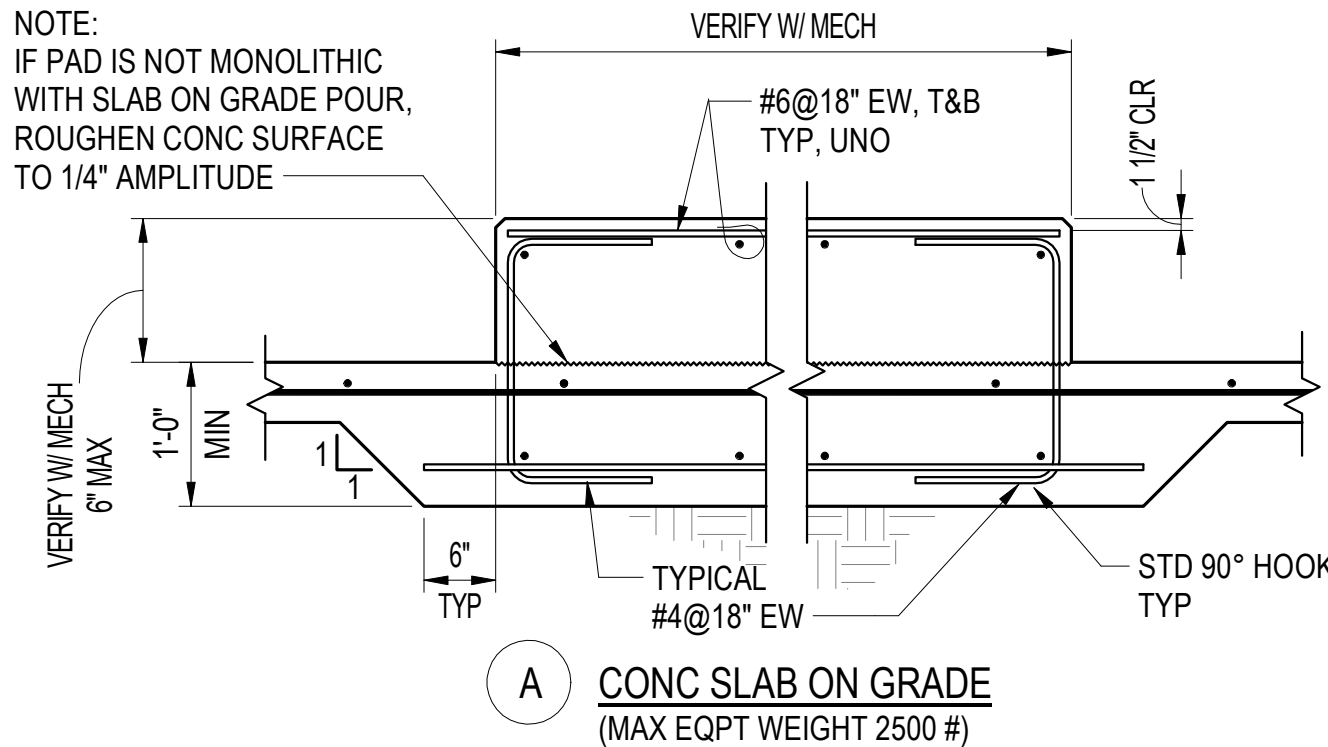
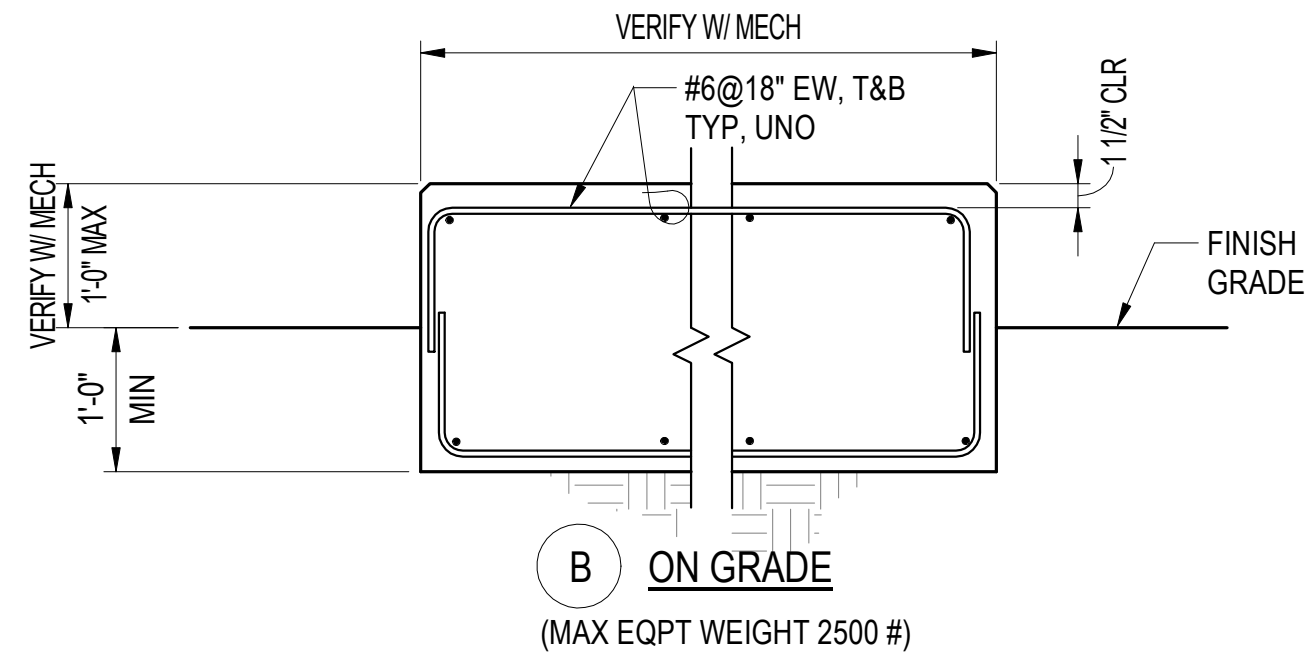
STRAIGHT DEVELOPMENT LENGTH SCHEDULE FOR TERMINATORS (L_{dt} , IN.)

NORMAL WEIGHT CONCRETE (f_c PSI)		3000 PSI		4000 PSI		5000 PSI	
BAR SIZE	BAR DIAMETER (db)	STANDARD	SEISMIC	STANDARD	SEISMIC	STANDARD	SEISMIC
#4	0.500	7	9	7	8	6	8
#5	0.625	10	12	9	11	9	11
#6	0.750	13	16	12	15	11	14
#7	0.875	16	20	15	18	14	18
#8	1.000	19	24	18	22	17	22
#9	1.128	23	28	22	27	21	26
#10	1.270	27	34	26	32	25	31
#11	1.410	32	40	30	37	29	36

HEADED BAR DEVELOPMENT LENGTH SCHEDULE

SCALE: NTS

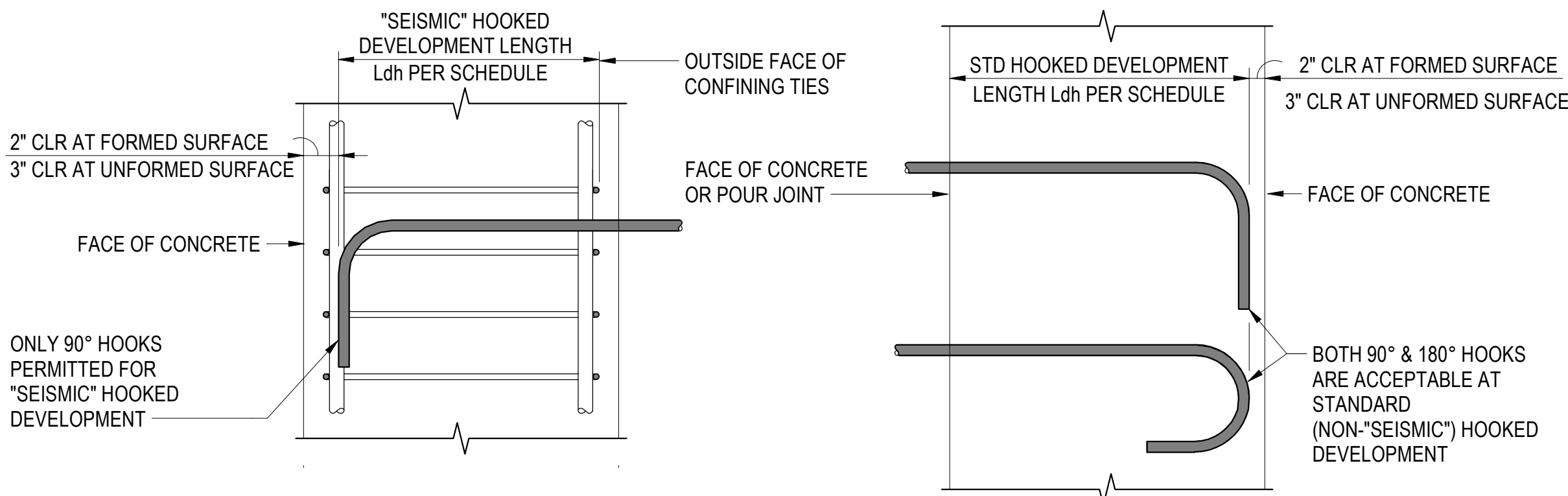
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TYPICAL MECHANICAL PAD AT SLAB ON GRADE DETAIL

SCALE: NTS

2



"SEISMIC" HOOKED DEVELOPMENT

STANDARD (NON-"SEISMIC") HOOKED DEVELOPMENT

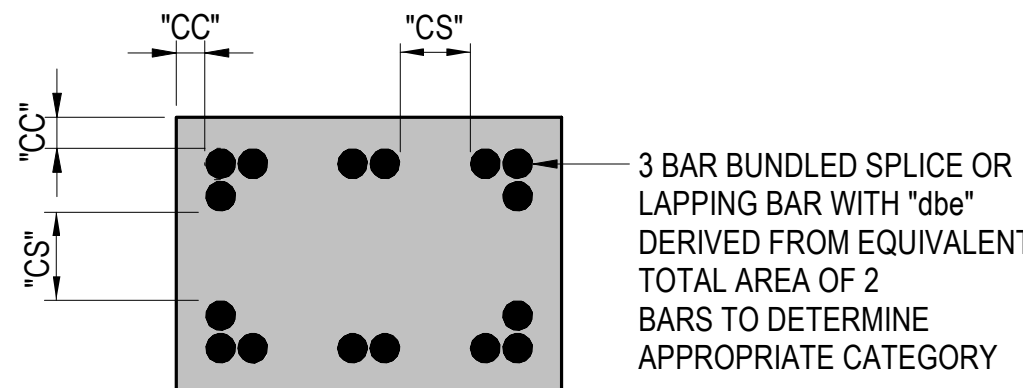
NOTES:

- A. ALL HOOKED BARS SHALL EXTEND TO THE FAR FACE OF CONCRETE, AS MUCH AS PRACTICAL, WITH 2" MINIMUM END COVER AND WITH DEVELOPMENT NOT LESS THAN LENGTHS INDICATED IN SCHEDULE INCLUDING APPROPRIATE MULTIPLIERS.
- B. FOR HOOKS TERMINATING INSIDE COLUMN CORE PROVIDE $2\frac{1}{2}$ " MINIMUM CONCRETE SIDE COVER. FOR HOOKS TERMINATING OUTSIDE COLUMN CORE PROVIDE $6db$ SIDE COVER NORMAL TO THE PLANE OF HOOK.
- C. "SEISMIC" HOOKED DEVELOPMENT LENGTH L_{dh} IN SCHEDULE INCLUDING APPROPRIATE MULTIPLIERS APPLY TO BARS W/ STD 90° HOOKS LOCATED WITHIN A CONFINED CORE OF SPECIAL MOMENT FRAME COLUMNS. 180° HOOKS ARE NOT PERMITTED FOR "SEISMIC" HOOKED DEVELOPMENT.
- D. INCREASE "STANDARD" HOOKS BY $1.6x$ WHEN HOOKS DO NOT TERMINATE IN COLUMNS OR WHEN CONFINEMENT TIES ARE ABSENT AND THE SPACING BETWEEN HOOKED BARS IS LESS THAN $6db$ ($CS < 6db$).
- E. INCREASE "STANDARD" HOOK BY $1.25x$ WHEN SIDE COVER IS LESS THAN $6db$.

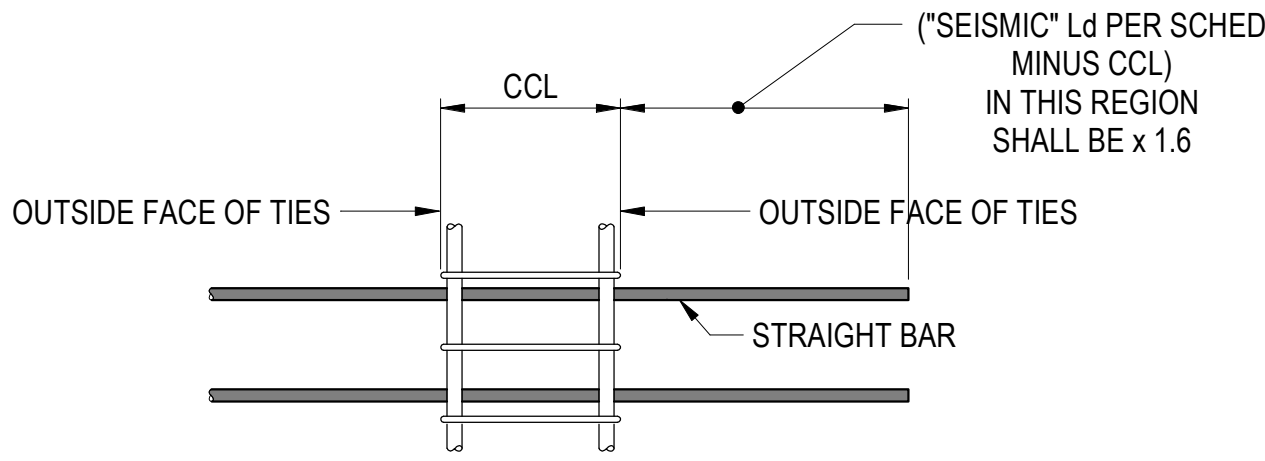
HOOKED DEVELOPMENT L_{dh}

STRAIGHT DEVELOPMENT LENGTH SCHEDULE (Ld) IN INCHES (APPLICABLE TO REBAR W/ 60 KSI YIELD STRENGTH)																									
NORMAL WEIGHT CONCRETE (fc PSI)		3000 PSI								4000 PSI & 4500 PSI								5000 PSI							
CATEGORY		1		2		3		SEISMIC (SEE NOTE 6)		1		2		3		SEISMIC (SEE NOTE 6)		1		2		3		SEISMIC (SEE NOTE 6)	
BAR SIZE	BAR DIAMETER (db)	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS	TOP	OTHERS
#4	0.500	18	14	29	22	43	33	28	22	15	12	25	19	37	29	24	19	14	12	23	17	34	26	22	17
#5	0.625	22	17	36	28	54	42	35	27	19	15	31	24	47	36	30	23	17	13	28	22	42	32	27	21
#6	0.750	26	20	43	33	65	50	42	32	23	18	37	29	56	43	36	28	20	16	34	26	50	39	32	25
#7	0.875	38	29	63	48	94	72	48	37	33	25	54	42	81	63	42	32	29	23	49	38	73	56	38	29
#8	1.000	43	33	72	55	107	83	55	43	37	29	62	48	93	72	48	37	34	26	56	43	83	64	43	33
#9	1.128	49	38	81	62	121	93	62	48	42	33	70	54	105	81	54	42	38	29	63	48	94	72	48	37
#10	1.270	55	42	91	70	136	105	70	54	47	37	79	61	118	91	61	47	43	33	71	54	106	81	54	42
#11	1.410	61	47	101	78	151	116	78	60	53	41	87	67	131	101	67	52	47	36	78	60	117	90	60	47
#14	1.693	73	56	121	93	181	140	-	-	63	49	105	81	157	121	-	-	57	44	94	72	141	108	-	-
#18	2.257	97	75	161	124	242	186	-	-	84	65	140	108	209	161	-	-	75	58	125	96	187	144	-	-

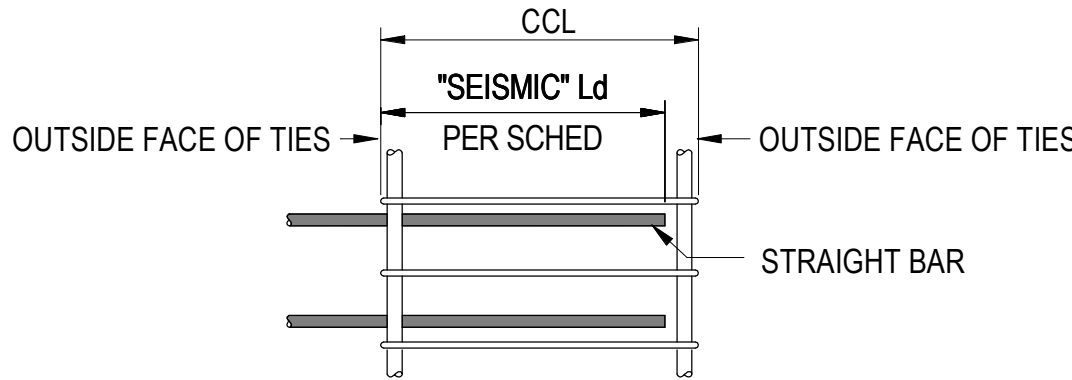
HOOKED DEVELOPMENT LENGTH SCHEDULE (L_{dh}) IN INCHES (APPLICABLE TO REBAR W/ 60 KSI YIELD STRENGTH)							
NORMAL WEIGHT CONCRETE (f_c PSI)		3000 PSI		4000 PSI & 4500 PSI		5000 PSI	
BAR SIZE	BAR DIAMETER (db)	STANDARD	SEISMIC (SEE NOTE 7)	STANDARD	SEISMIC (SEE NOTE 7)	STANDARD	SEISMIC (SEE NOTE 7)
#4	0.500	6	9	6	8	6	7
#5	0.625	8	11	8	10	8	9
#6	0.750	11	13	10	11	10	10
#7	0.875	14	15	13	13	12	12
#8	1.000	16	17	15	15	15	14
#9	1.128	20	20	18	17	18	15
#10	1.270	23	22	22	19	21	17
#11	1.410	27	24	26	21	25	19
#14	1.693	36	-	33	-	32	-
#18	2.257	55	-	51	-	49	-



TYPICAL BAR CONCRETE COVER & CLEAR SPACING DIAGRAM



CONDITION WHERE "SEISMIC" L_d IS GREATER THAN CONFINED CORE LENGTH CCL



CONDITION WHERE "SEISMIC" L_d FITS WITHIN CONFINED CORE

NOTE:

"CCL" INDICATES CONFINED CORE LENGTH OF SPECIAL MOMENT FRAME COLUMNS.

STRAIGHT "SEISMIC" DEVELOPMENT L_d AT CONFINED CORES OF SPECIAL MOMENT FRAME COLUMNS

REINFORCING DEVELOPMENT NOTES:

1. SCHEDULED DEVELOPMENT LENGTHS ARE IN ACCORDANCE WITH ACI 318-19 AND APPLY TO REBAR $F_y=60$ KSI. LENGTHS ARE FROM CHAPTER 25 (NON-SEISMIC ELEMENTS) AND CHAPTER 18 (SEISMIC ELEMENTS).

CATEGORY	DESCRIPTION
1	$2db \leq CC$ AND $4db \leq CS$
2	$[db \leq CC < 2db \text{ \& } 2db \leq CS]$ OR $[db \leq CC \text{ \& } 2db \leq CS < 4db]$
3	$1/2db \leq CC < db$ OR $db \leq CS < 2db$
SEISMIC	SEE NOTES 6 & 7

CC INDICATES CONCRETE COVER, CS INDICATES BAR CLEAR SPACING.

3. IF $CC < 1/2db$ OR $CS < db$ CONTACT SEOR FOR REQUIRED DEVELOPMENT LENGTH.
4. TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 12" OF FRESH CONCRETE POURED BELOW BARS.
5. FOR BUNDLED BARS, AN EFFECTIVE BAR DIAMETER (db_e) SHALL BE USED FOR DETERMINING COVER AND SPACING LIMITATIONS:
 - a. FOR 2 BAR BUNDLE $db_e = 1.414db$
 - b. FOR 3 BAR BUNDLE $db_e = 1.732db$
 - c. FOR 4 BAR BUNDLE $db_e = 2.000db$
6. "SEISMIC" STRAIGHT DEVELOPMENT LENGTH L_d IN SCHEDULE APPLIES TO STRAIGHT BARS LOCATED ENTIRELY IN THE CONFINED CORES.
7. "SEISMIC" HOOKED DEVELOPMENT LENGTH L_{dh} IN SCHEDULE APPLIES TO BARS W/ STD 90° HOOKS LOCATED WITHIN A CONFINED CORE.
8. APPLY THE FOLLOWING MULTIPLIERS TO SCHEDULED DEVELOPMENT LENGTHS FOR EACH INSTANCE BELOW WHICH APPLIES:

- a. FOR REBAR YIELD STRENGTHS OTHER THAN 60 KSI, MULTIPLY DEVELOPMENT LENGTHS L_d & L_{dh} IN SCHEDULE BY 1.6 OR 2.2 FOR REBAR STRENGTHS OF 80 KSI OR 100 KSI RESPECTIVELY.
- b. DEVELOP ALL LONGITUDINAL BARS IN SHEAR WALLS LOCATED IN THE FOUNDATIONS OR WITHIN A HEIGHT EQUAL TO THE LENGTH OF THE WALL ABOVE AND BELOW THE BASE OF THE WALL BY MULTIPLYING DEVELOPMENT LENGTHS L_d & L_{dh} IN SCHEDULE BY 1.25 UNO ON STRUCTURAL DRAWINGS. BASE OF THE SHEAR WALL SHALL BE CONSIDERED TO BE AT THE FOUNDATION LEVEL, AT ANY PODIUM LEVEL, OR ANYWHERE WHERE THE WALL HAS A SET BACK MORE THAN 10% OF THE WALL LENGTH. SEE DETAIL 1/55.10 FOR ADDITIONAL REQUIREMENTS.
- c. FOR 3-BAR BUNDLES, MULTIPLY DEVELOPMENT LENGTHS L_d & L_{dh} IN SCHEDULE BY 1.20. FOR 4-BAR BUNDLES, MULTIPLY DEVELOPMENT LENGTHS L_d & L_{dh} IN SCHEDULE BY 1.33.
- d. FOR LIGHTWEIGHT CONCRETE, MULTIPLY STRAIGHT DEVELOPMENT LENGTH L_d IN SCHEDULE BY 1.33. FOR LIGHTWEIGHT CONCRETE, MULTIPLY HOOKED DEVELOPMENT LENGTH L_{dh} IN SCHEDULE BY 1.33 (1.25 FOR "SEISMIC" CONDITIONS AS DEFINED IN NOTE 7).
- e. FOR EPOXY COATED BARS WITH $CC < 3db$ OR $CS < 6db$, MULTIPLY STRAIGHT DEVELOPMENT LENGTH L_d IN SCHEDULE BY 1.50. FOR STRAIGHT DEVELOPMENT FOR OTHER EPOXY COATED BARS, MULTIPLY STRAIGHT DEVELOPMENT LENGTH L_d IN SCHEDULE BY 1.20. FOR EPOXY COATED BARS, MULTIPLY HOOKED DEVELOPMENT LENGTH L_{dh} IN SCHEDULE BY 1.20.
- f. DEVELOP ALL DIAGONAL COUPLING BEAM REBARS BY MULTIPLYING DEVELOPMENT LENGTHS L_d AND L_{dh} IN SCHEDULE BY 1.25.
- g. FOR CONCRETE STRENGTH IN BETWEEN STRENGTHS INDICATED IN THE SCHEDULE, USE DEVELOPMENT LENGTH FOR THE LOWER CONCRETE STRENGTH.

TYPICAL REINFORCING STRAIGHT AND HOOKED DEVELOPMENT LENGTH SCHEDULE

SCALE: NTS

1

WILLIAM LOYD JONES ARCHITECT
9415 culver boulevard
culver city, california
9 0 2 3 2
TEL 310 392 3995

SO
structural engineers
726 S. Figueroa St.,
37th Floor
Los Angeles, CA 90017
(213) 315-2277
Project #25534

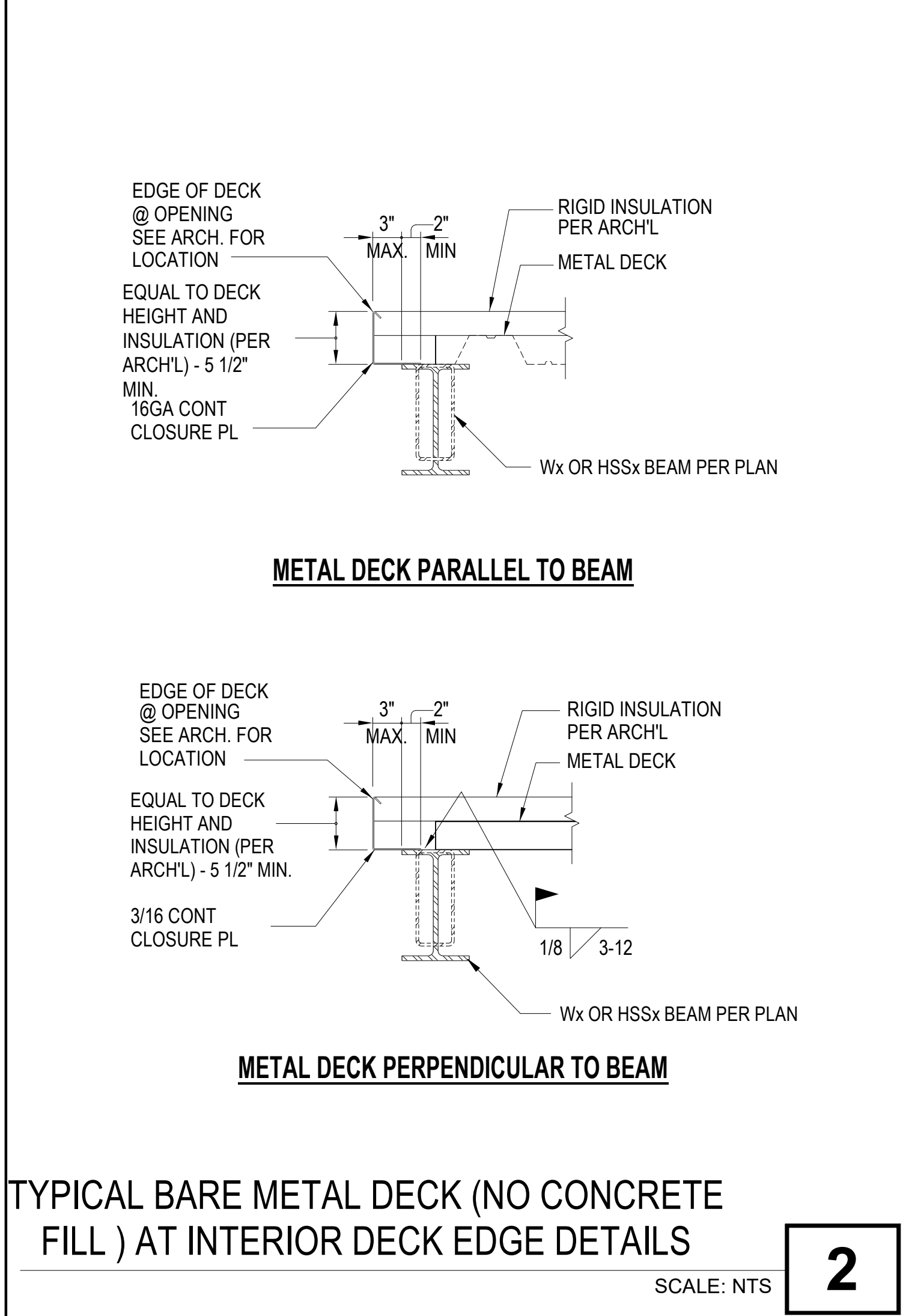
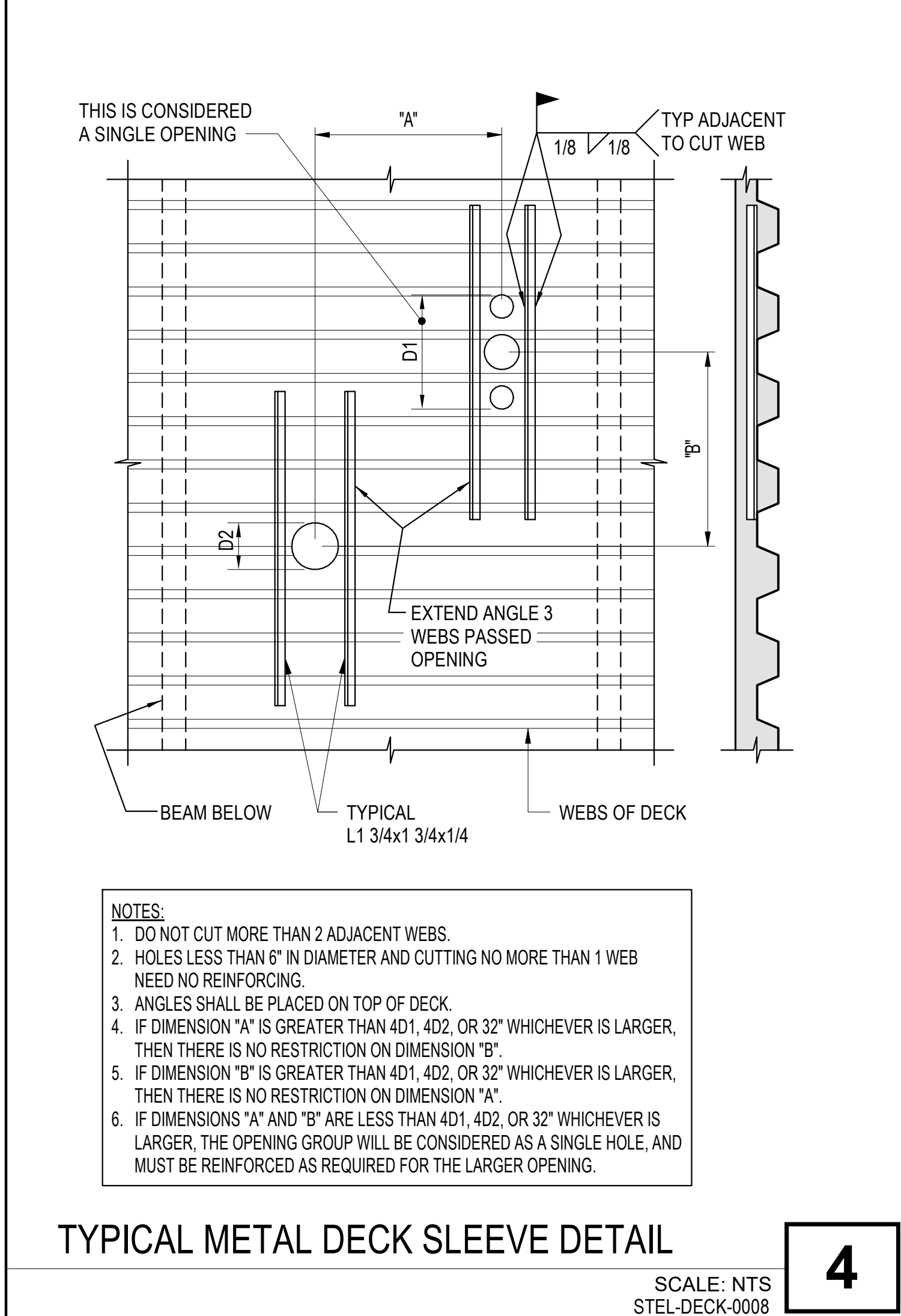
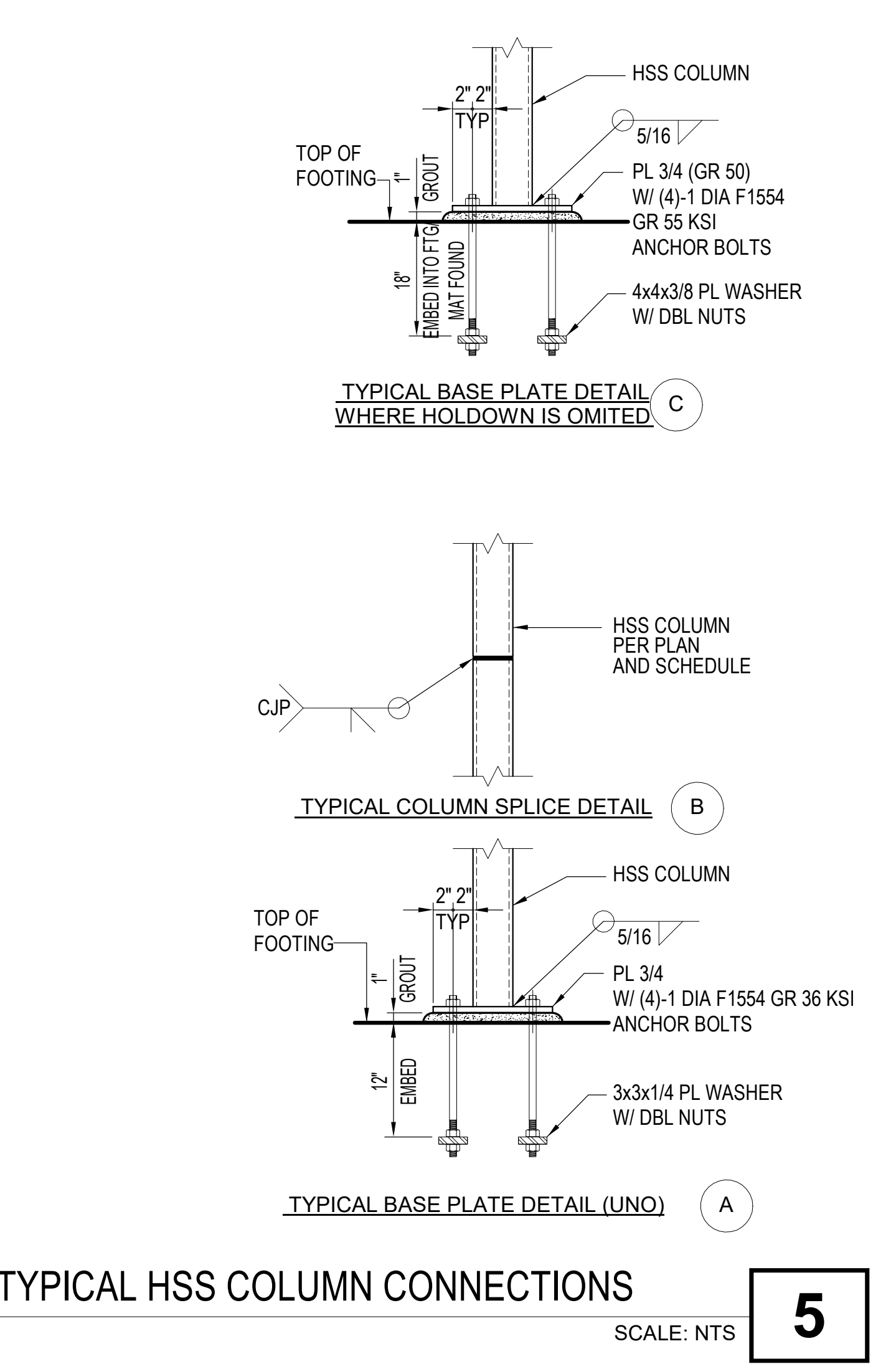
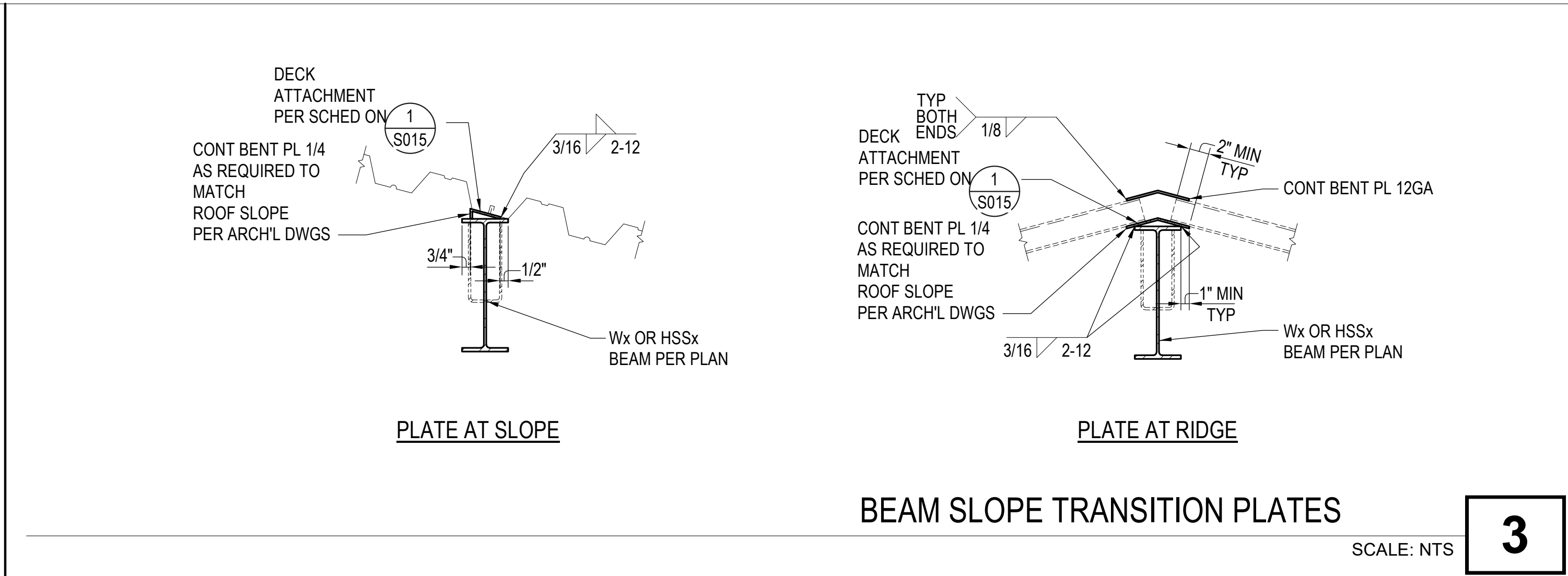
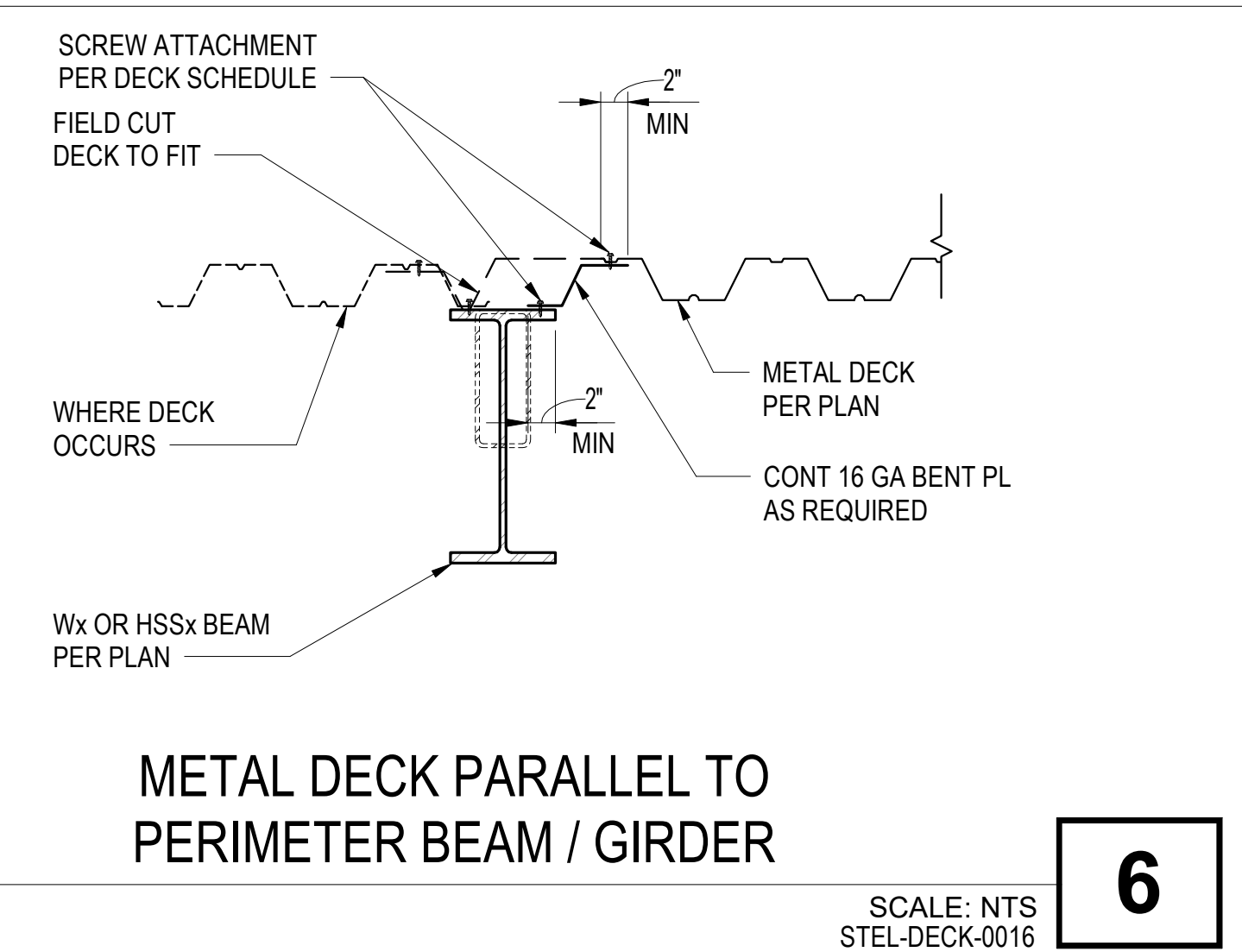
TYPICAL REINFORCING DETAILS
FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S3289
Exp. 12/31/27
STRUCTURAL
STATE OF CALIFORNIA

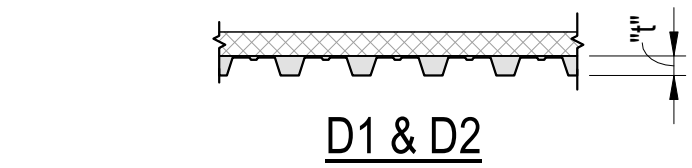
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WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL TAKE PRECEDENCE OVER SCALED DIMENSIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THIS OFFICE MUST BE NOTIFIED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS SHOWN IN THESE DRAWINGS, BEFORE PROCEEDING WITH CONSTRUCTION.
DATE: 05/05/2026

Date	Issue Date
Drawn	
Checked	
Scale	AS NOTED
Job. No.	Project Number

SO
ADDENDUM 5
11
PLAN CHECK SUBMITTAL - October 31 2025



METAL DECK CONSTRUCTION SCHEDULE												
MARK	METAL DECKING		TOTAL SLAB "t"	SLAB DESCRIPTION	METAL DECKING WELDING			MAXIMUM UNSHORED SPAN (CLEAR SPAN BETWEEN SUPPORTS)			HOURLY FIRE RATING	REMARKS
	TYPE	GAUGE			PERPENDICULAR TO SUPPORT	PARALLEL TO SUPPORT	SEAMS	SINGLE	DOUBLE	TRIPLE		
D1A	VERCO HSB-36 DECK	18	1 1/2"	AT TRASH ENCLOSURE 2/S270	(7)#12-24 SIMPSON X-SCREW OR APPROVED EQUAL	(2)#12-24SIMPSON X-SCREW OR APPROVED EQUAL @12" OC	#12-24 SIMPSON X-SCREW OR APPROVED EQUAL @12" OC	8'-0"	8'-0"	8'-0"	NOT RATED	GALVANIZED COATING CONFORMING TO ASTM 525 CLASS G90
D2A	VERCO HSN3-32-NS DECK	18	3"	AT CANOPY 2/S260	(7)#12-24 SIMPSON X-SCREW OR APPROVED EQUAL	(2)#12-24SIMPSON X-SCREW OR APPROVED EQUAL @12" OC	#12-24 SIMPSON X-SCREW OR APPROVED EQUAL @8" OC	10'-0"	10'-0"	10'-0"	NOT RATED	GALVANIZED COATING CONFORMING TO ASTM 525 CLASS G90



NOTES:
1. PROVIDE VERCO METAL DECKING AND ACCESSORIES IN CONFORMANCE WITH APMO ER-2018.

1
TYPICAL METAL DECK CONSTRUCTION SCHEDULE
SCALE: NTS
STEL-DECK-0000 (REV-1)

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2
TEL 310 392 3995

SO
saiful-bouquet
structural engineers

725 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

TYPICAL STEEL FRAMING
AND DECKING DETAILS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S3289
Exp. 12/31/27
STRUCTURAL
STATE OF CALIFORNIA

5

Date Issue Date

Drawn

Checked

Scale AS NOTED

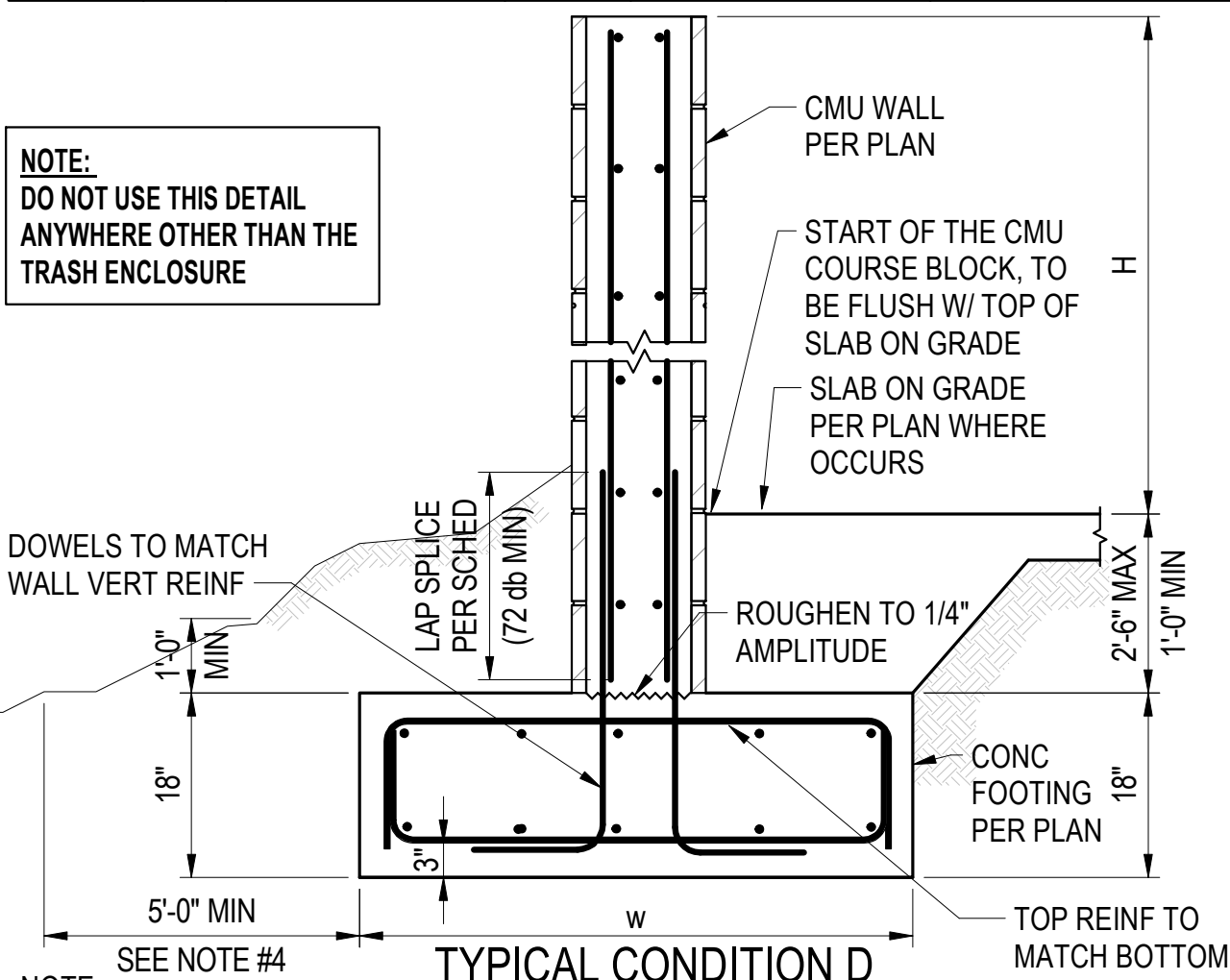
Job No. Project Number

APPENDIX

S015

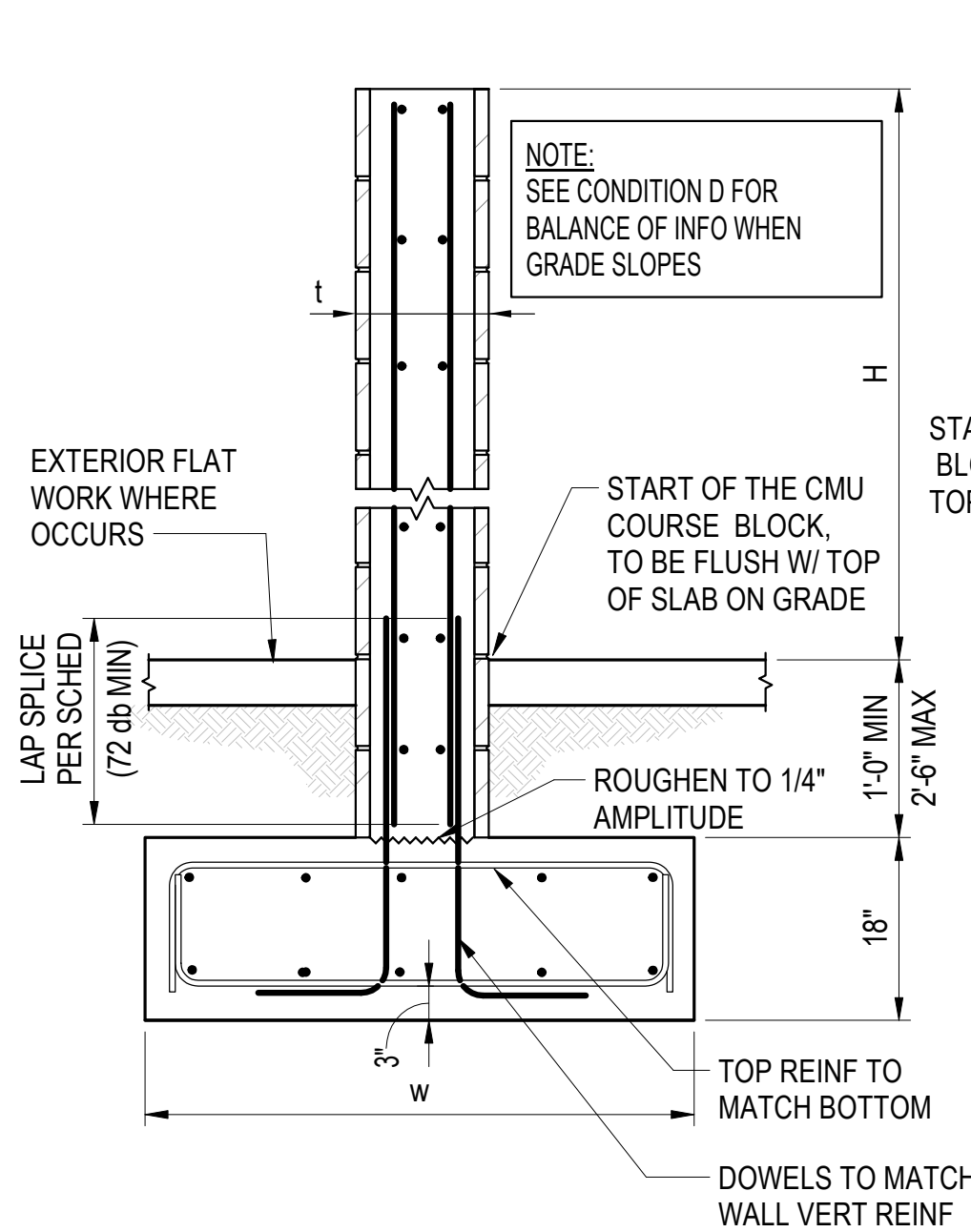
PLAN CHECK SUBMITTAL - October 31 2025

SCHEDULE FOR CONDITION A, B, AND C					
MAX H	WALL		FOOTING		REMARKS
	t	REINFORCING	w	REINFORCING	
4'-6"	8"	#4(V)@16" #4(H)@16"	3'-6"	(3)-#4(T&B) LONG #6(T&B)@12" SHORT	SEE CONDITION A f _m = 1500 PSI
4'-6"	8"	#4(V)@16" #4(H)@16"	3'-6"	(4)-#4(T&B) LONG #6(T&B)@12" SHORT	SEE CONDITION B f _m = 1500 PSI
7'-0"	8"	#5(V)@16" #4(H)@16"	4'-6"	(5)-#5 (T&B) LONG #6(T&B)@12" SHORT	SEE CONDITION A f _m = 2500 PSI
9'-0"	8"	#5(V)@16" EF #4(H)@16" EF	5'-6"	(6)-#5 (T&B) LONG #6(T&B)@12" SHORT	SEE CONDITION D & C f _m = 2500 PSI
12'-0"	12"	#5(V)@8" EF #4(H)@16" EF	8'-0"	(7)-#5 (T&B) LONG #6(T&B)@9" SHORT	SEE CONDITION D & C f _m = 2500 PSI

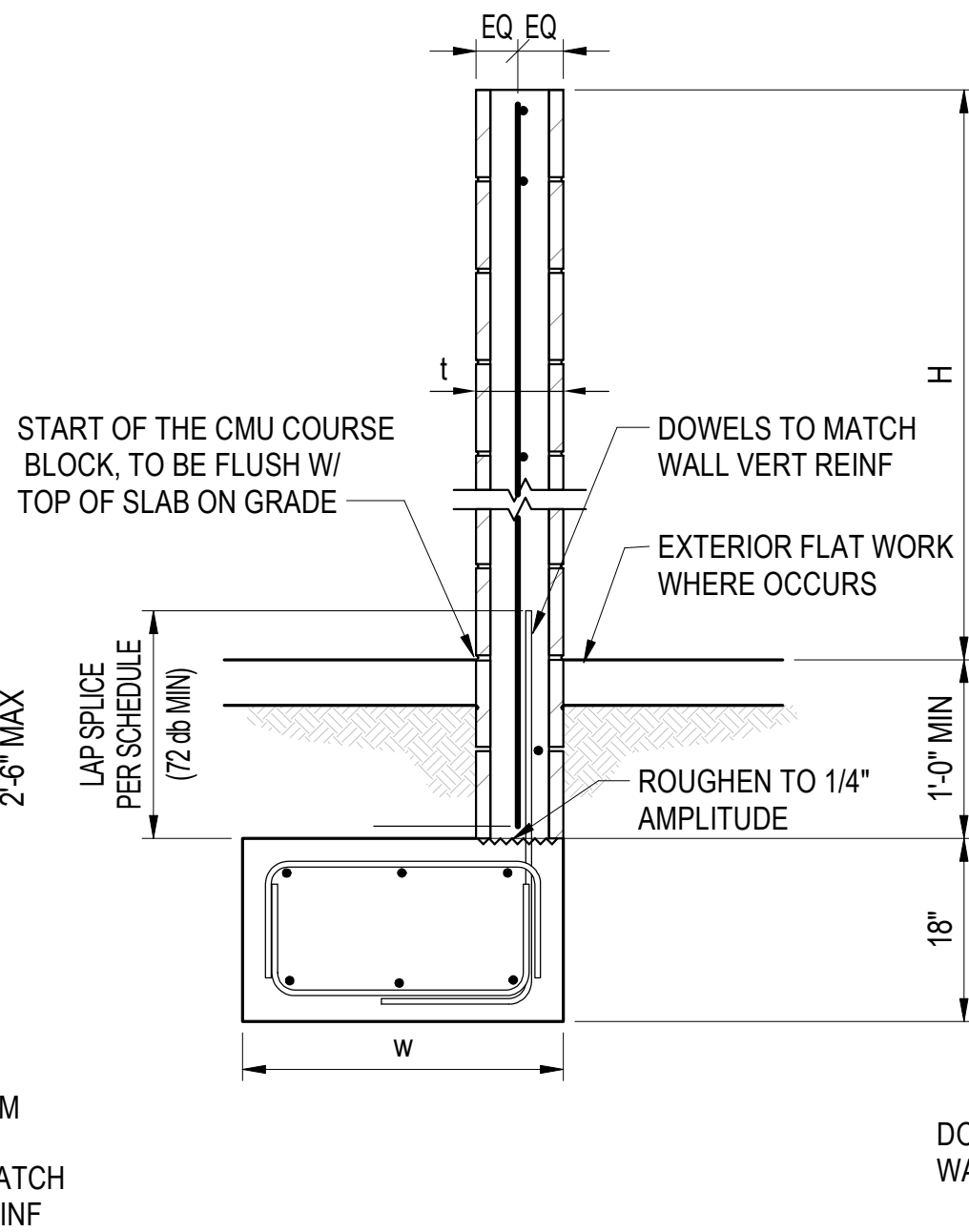


- NOTE:
- WHERE MASONRY WALL FOOTING TIES INTO COLUMN FOOTING, EXTEND MASONRY WALL FOOTING REINFORCING TO THE FAR END OF THE COLUMN FOOTING AND HOOK REINFORCING ENDS.
 - WHERE MASONRY WALL IS RETAINING SOIL ON ONE SIDE, THE MASONRY WALL SHALL HAVE DRAINAGE PER THE GEOTECHNICAL RECOMMENDATIONS AND CIVIL DRAWINGS TO ENSURE A DRAINAGE BACKFILL CONDITION (E.G. NO HYDROSTATIC PRESSURE ON THE MASONRY WALL).
 - WHERE WALL "H" IS BETWEEN VALUES INDICATED, USE DESIGN VALUES FOR HIGHER LISTED "H".
 - LOWER THE FOOTING AS REQUIRED TO ACHIEVE MINIMUM DIMENSIONS INDICATED AND SHOWN

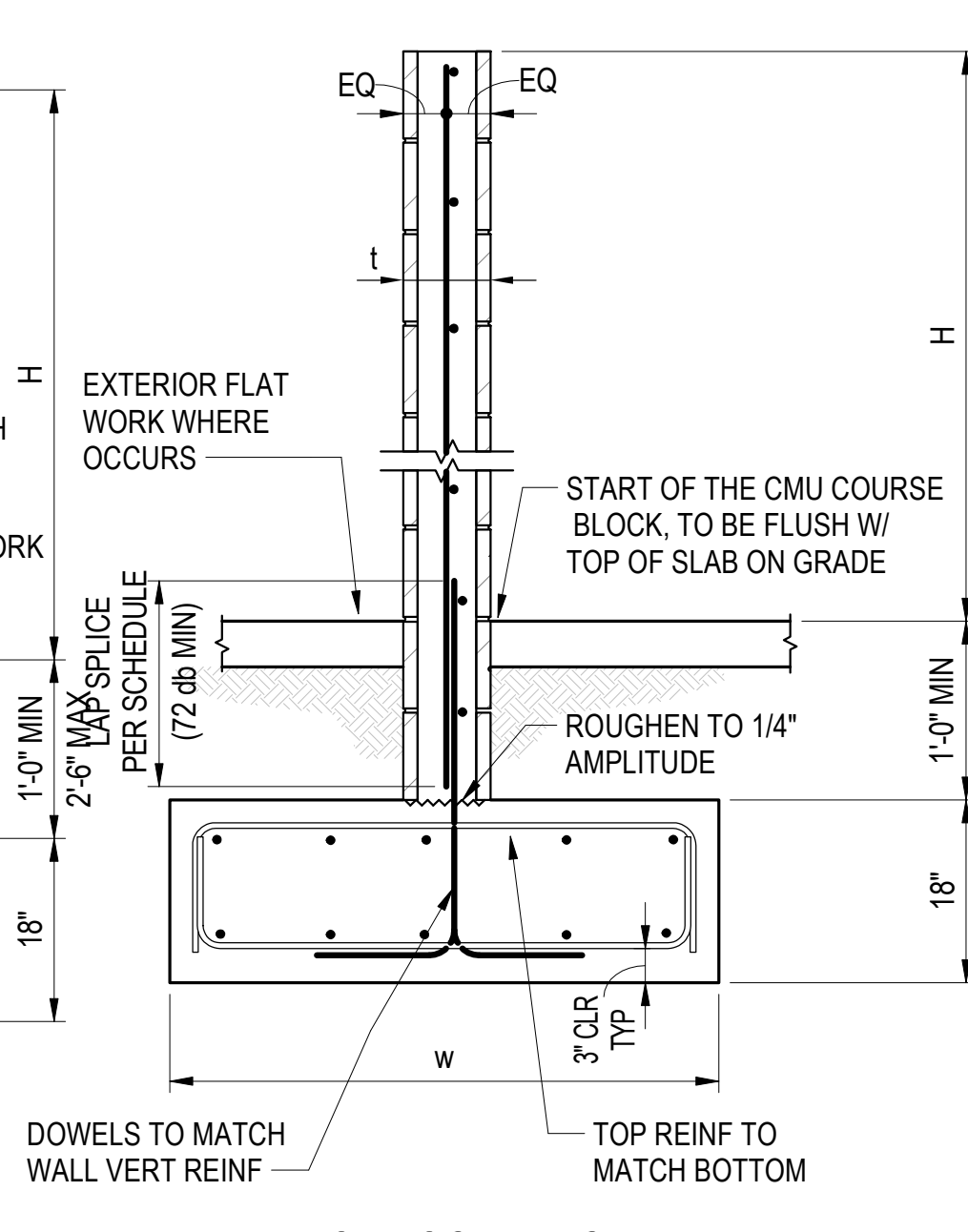
TYPICAL CONDITION D



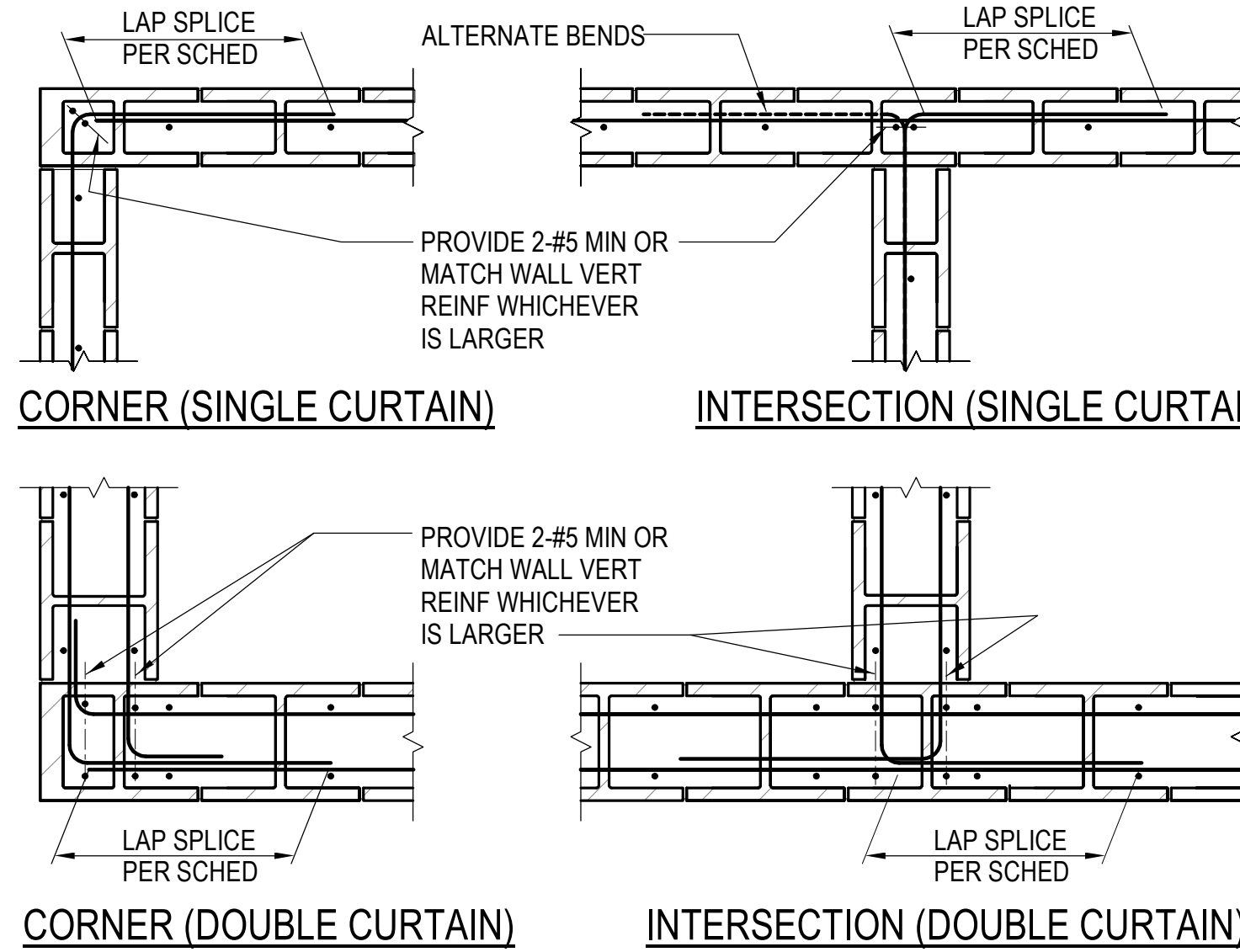
TYPICAL CONDITION C



TYPICAL CONDITION B
AT PROPERTY LINE



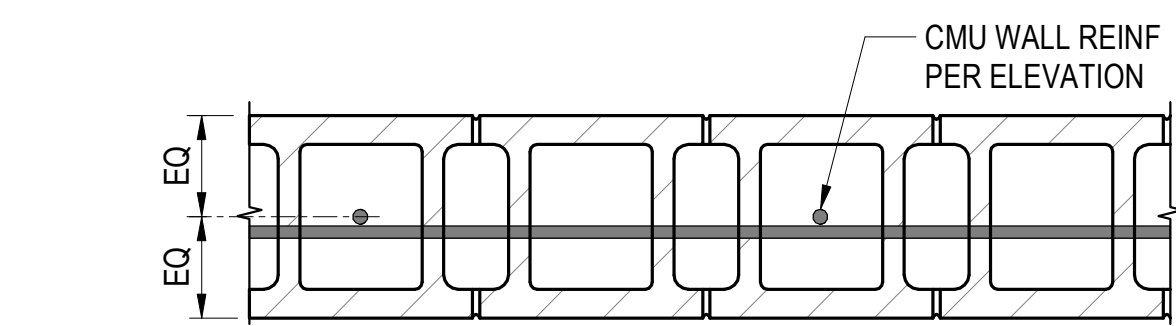
TYPICAL CONDITION A



TYPICAL MASONRY WALL AT INTERSECTIONS DETAIL

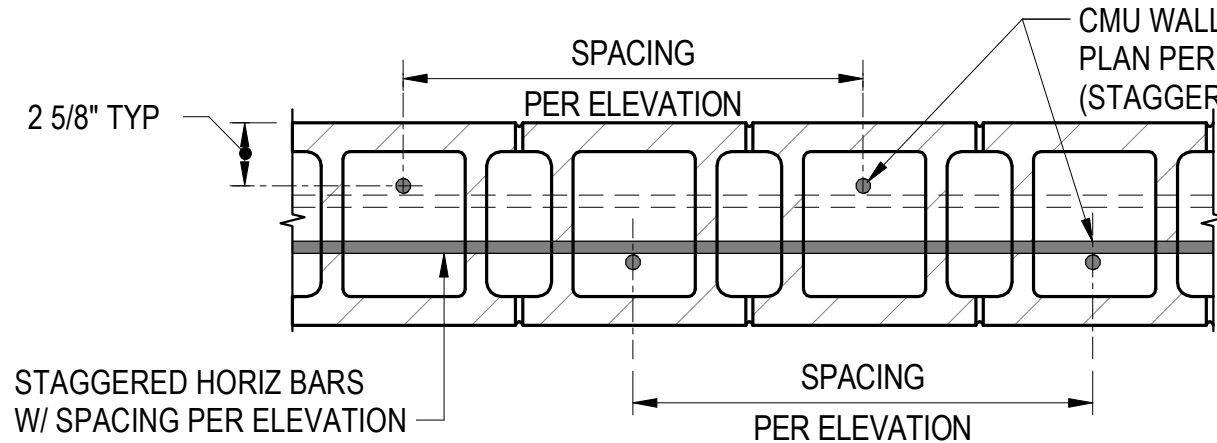
SCALE: NTS
REV 01/06/21

3



8" CMU WALL - REBAR CENTERED

NOTE:
CLOSE END BLOCK SHOWN FOR THE DETAILS, USE THESE REINFORCING SHOWN FOR OPEN END BLOCKS WHERE OCCURS.



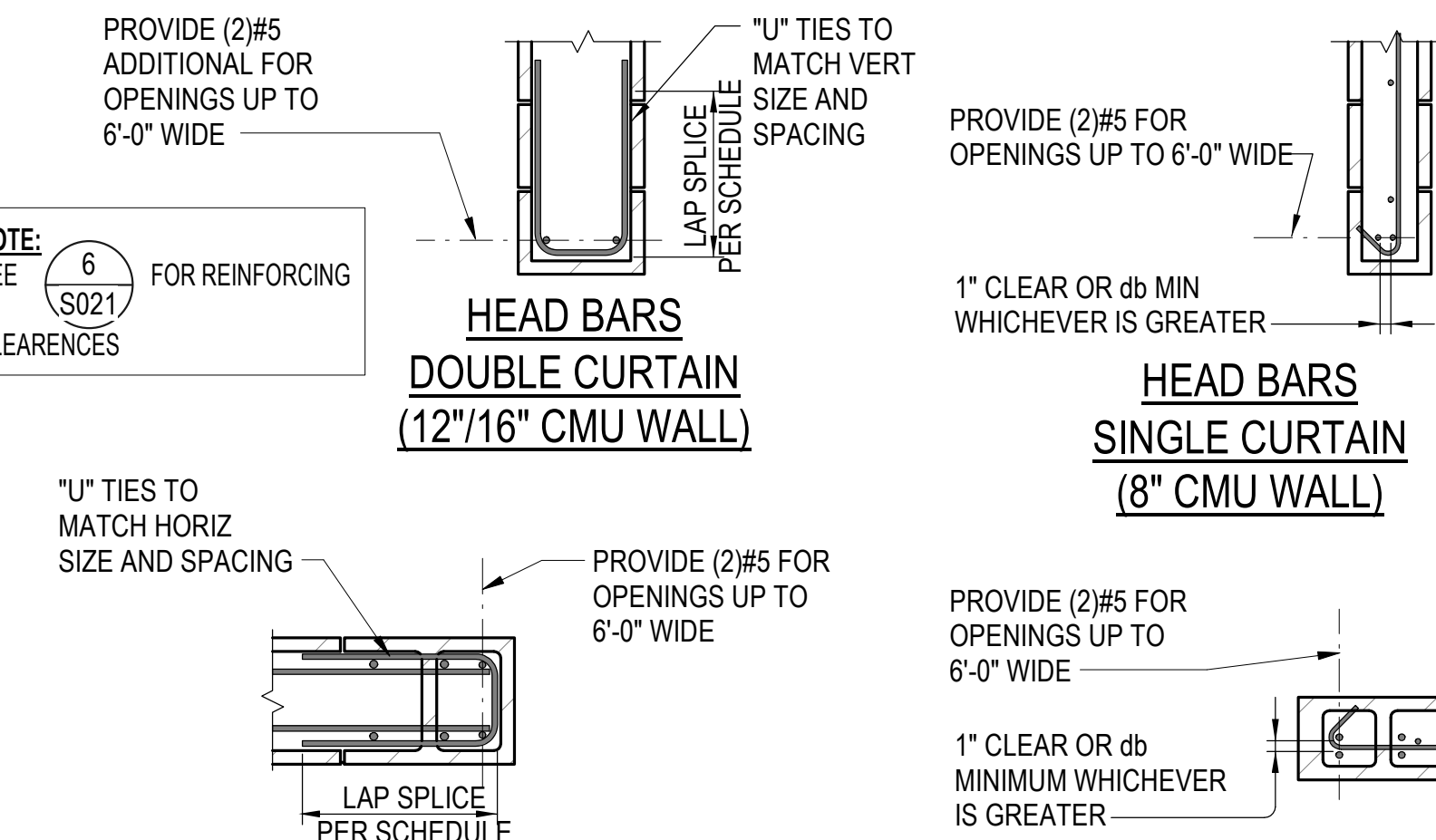
8" CMU WALL - REBAR EACH FACE STAGGERED

NOTE:
1. REFER TO S021 FOR BALANCE OF INFO.

TYPICAL CMU WALL REINFORCING PLACEMENT DETAIL

SCALE: NTS

6

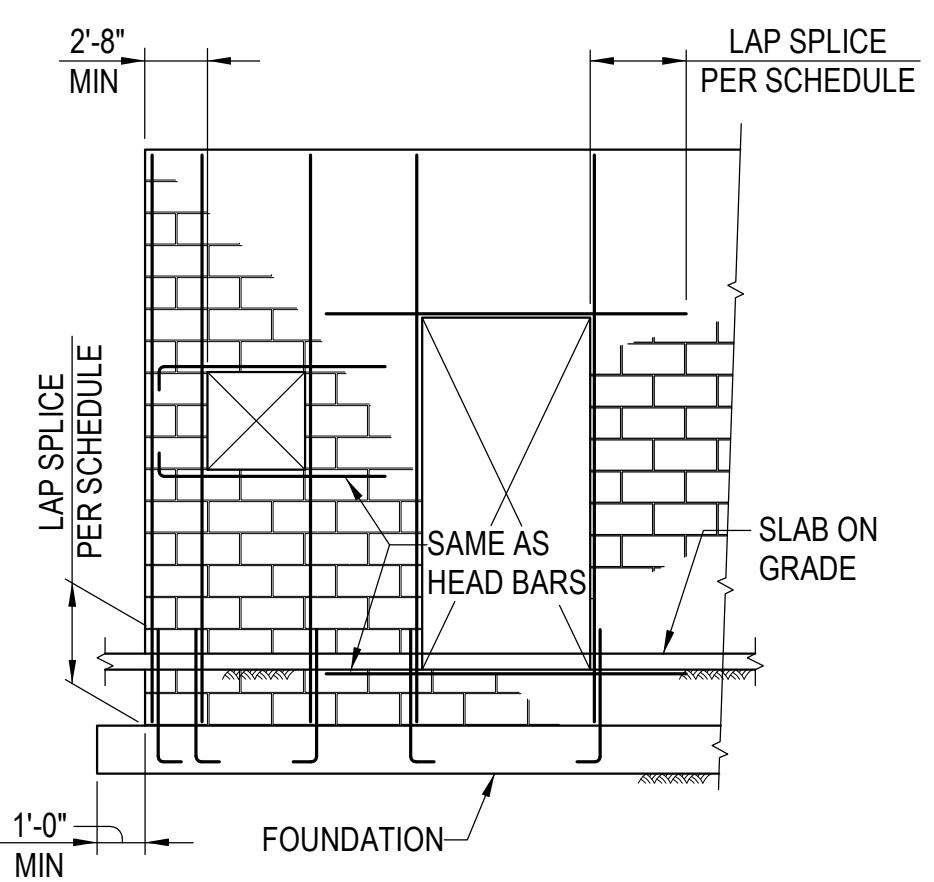


HEAD BARS
DOUBLE CURTAIN
(12"/16" CMU WALL)

HEAD BARS
SINGLE CURTAIN
(8" CMU WALL)

END OF WALL OR JAMB BARS
DOUBLE CURTAIN
(12"/16" CMU WALL)

END OF WALL OR JAMB BARS
SINGLE CURTAIN
(8" CMU WALL)



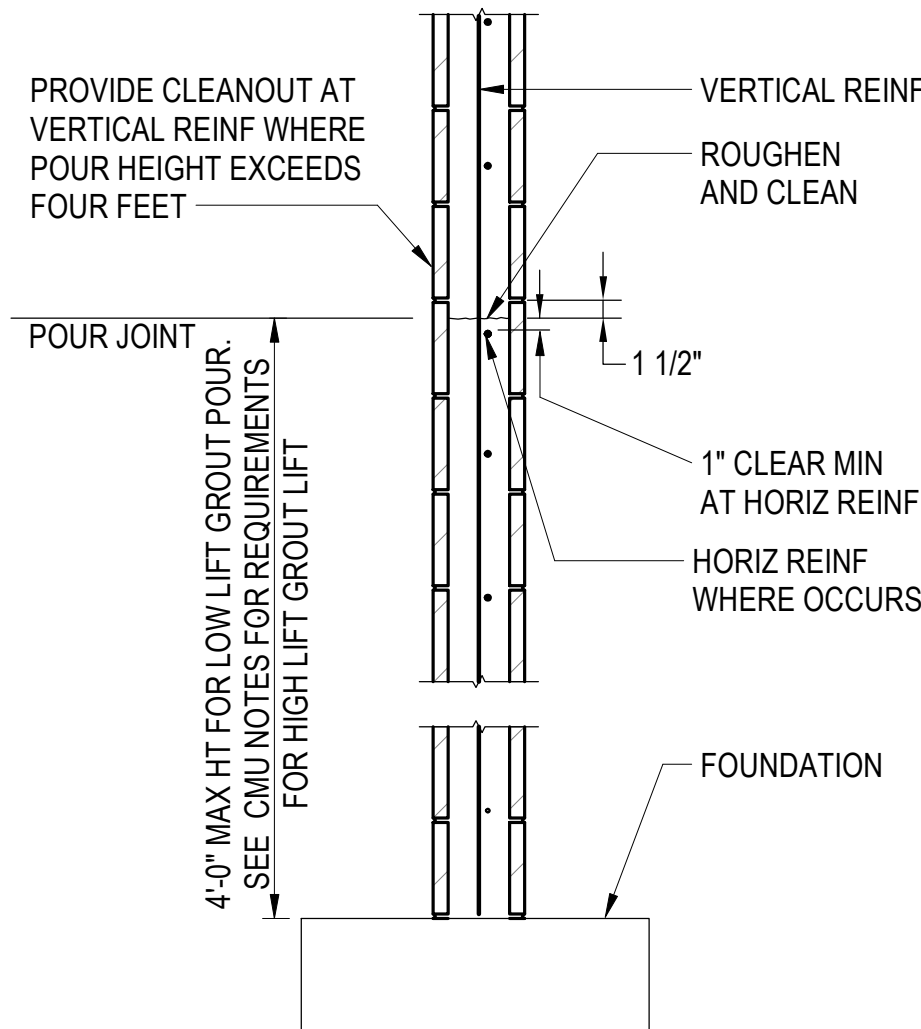
TYPICAL ELEVATION

NOTE:
USE OPEN END BLOCK TYPICAL

TYPICAL MASONRY WALL AT OPENINGS AND WALL ENDS DETAIL

SCALE: NTS

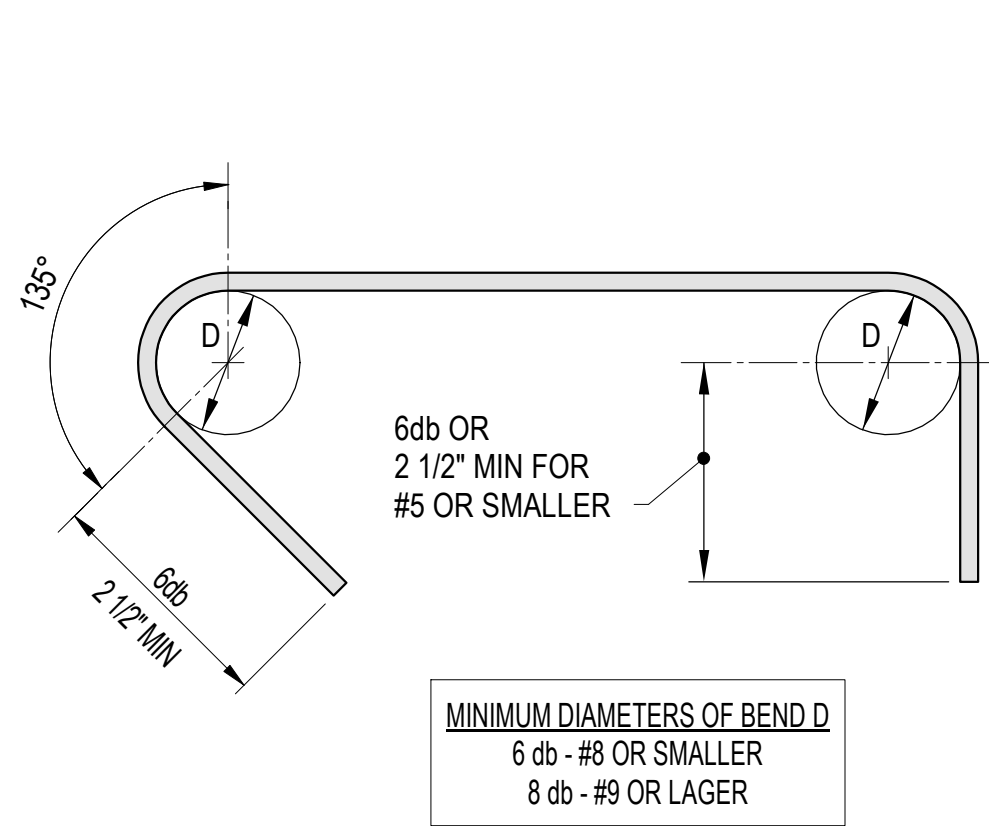
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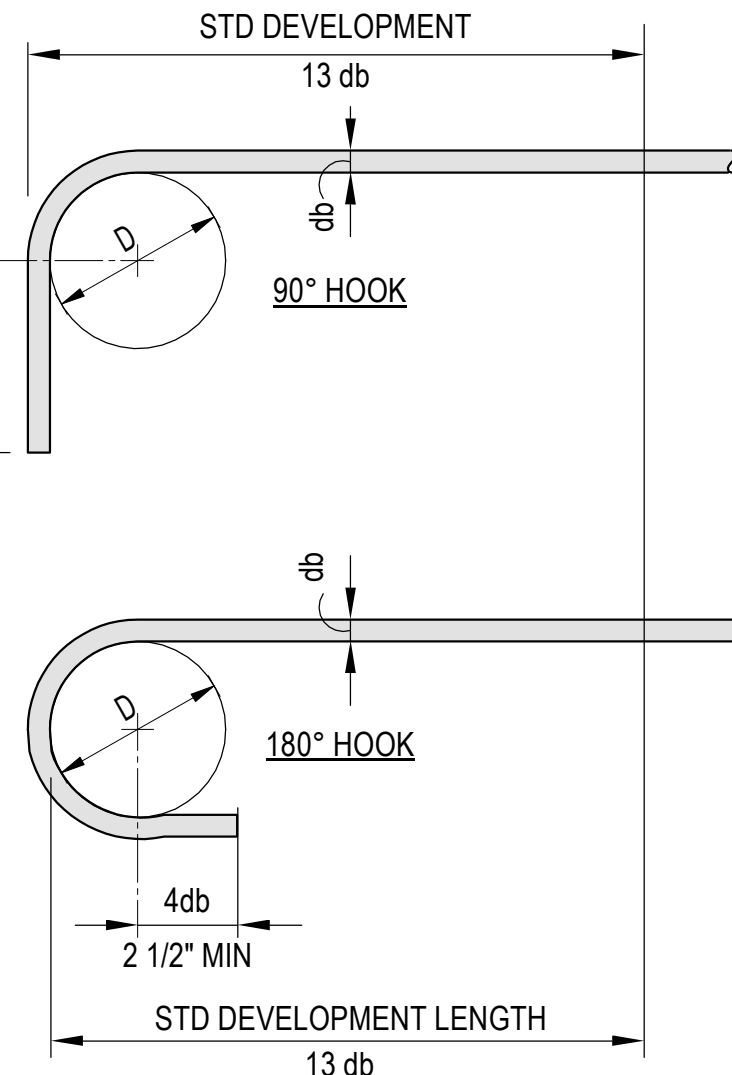
TYPICAL HORIZONTAL CONSTRUCTION
JOINT IN MASONRY WALL DETAIL

SCALE: NTS

4



STANDARD HOOKS FOR TIES AND STIRRUPS



TYPICAL STANDARD HOOK BEND AND
HOOKED DEVELOPMENT LENGTH DETAILS IN MASONRY DETAIL

SCALE: NTS

2

STRAIGHT DEVELOPMENT / LAP SPLICE LENGTH SCHEDULE (IN INCHES)										
MIN MASONRY COMPRESSIVE STRENGTH (f _m =2500 PSI)	MINIMUM (CC, CS) IN INCHES									
	1.5	2	2.5	3	3.5	4	4.5	5	5.5	6
BAR SIZE / BAR DIAMETER (db)										
#4	0.500	30	24	24	24	24	24	24	24	24
#5	0.625	45	35	30	30	30	30	30	30	30
#6	0.750	54	54	51	43	37	36	36	36	36
#7	0.875	63	63	63	58	50	44	42	42	42
#8	1.000	72	72	72	72	72	66	59	53	48
#9	1.128	82	82	82	82	82	82	74	67	56

- NOTES:
- CC INDICATES CLEAR COVER.
 - CS INDICATES BAR CLEAR SPACING.
 - db INDICATES BAR DIAMETER.
 - INDICATES LENGTHS GOVERNED BY 9db.
 - ALL LENGTHS INDICATED ARE IN INCHES.
 - 72 db MAX, 48db MIN.
 - IF ACTUAL CC (OR CS) FALLS BETWEEN THAT SHOWN IN SCHEDULE, DEVELOPMENT SPLICE LENGTH TO BE BASED ON SMALLER CC (OR CS)

TYPICAL REINF STRAIGHT DEVELOPMENT LENGTH/LAP SPLICE LENGTH SCHEDULE FOR MASONRY

SCALE: NTS

1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

SO
saiful-bouquet
structural engineers

726 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

TYPICAL CMU DETAILS

FIRE STATION 46

MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA



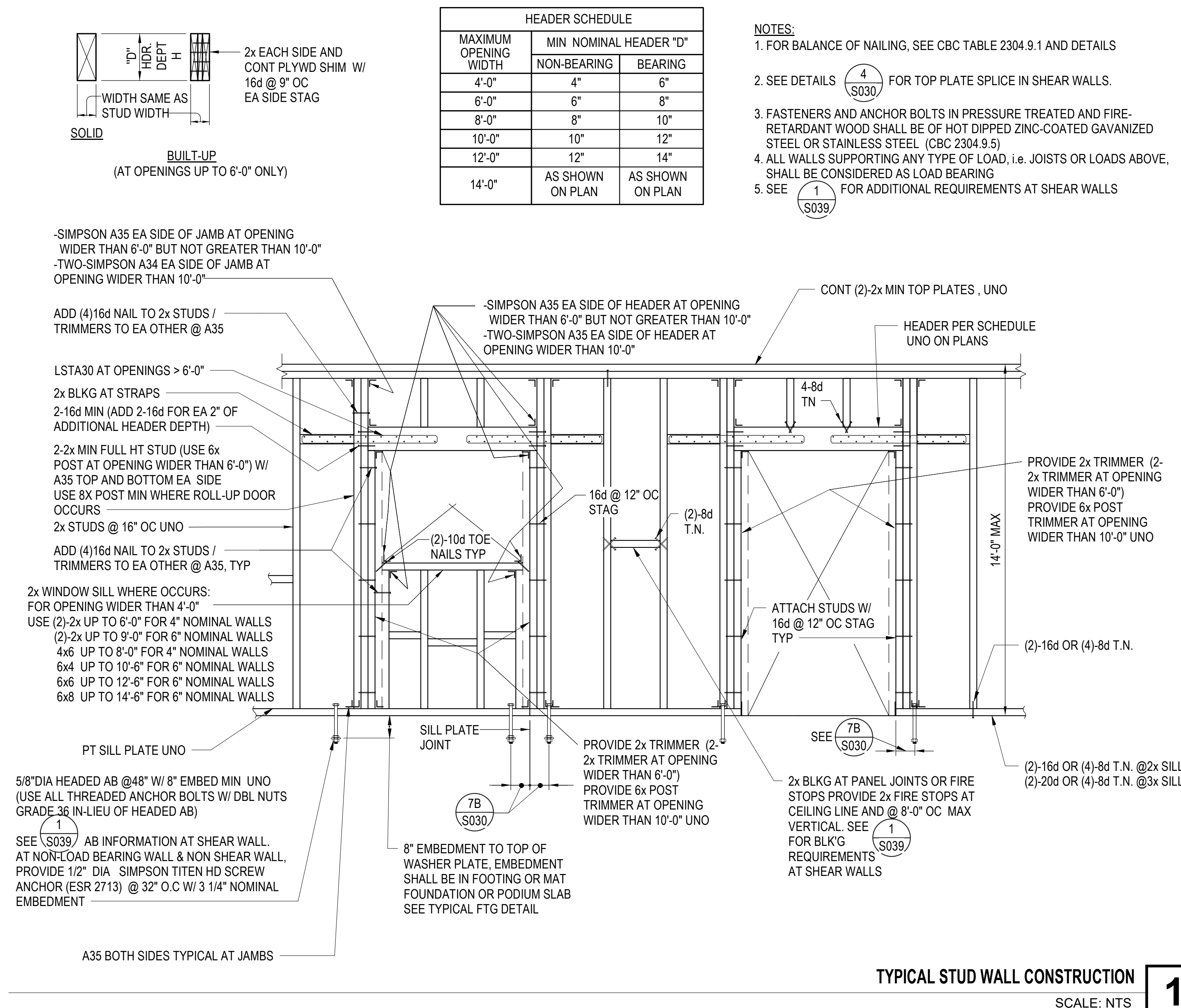
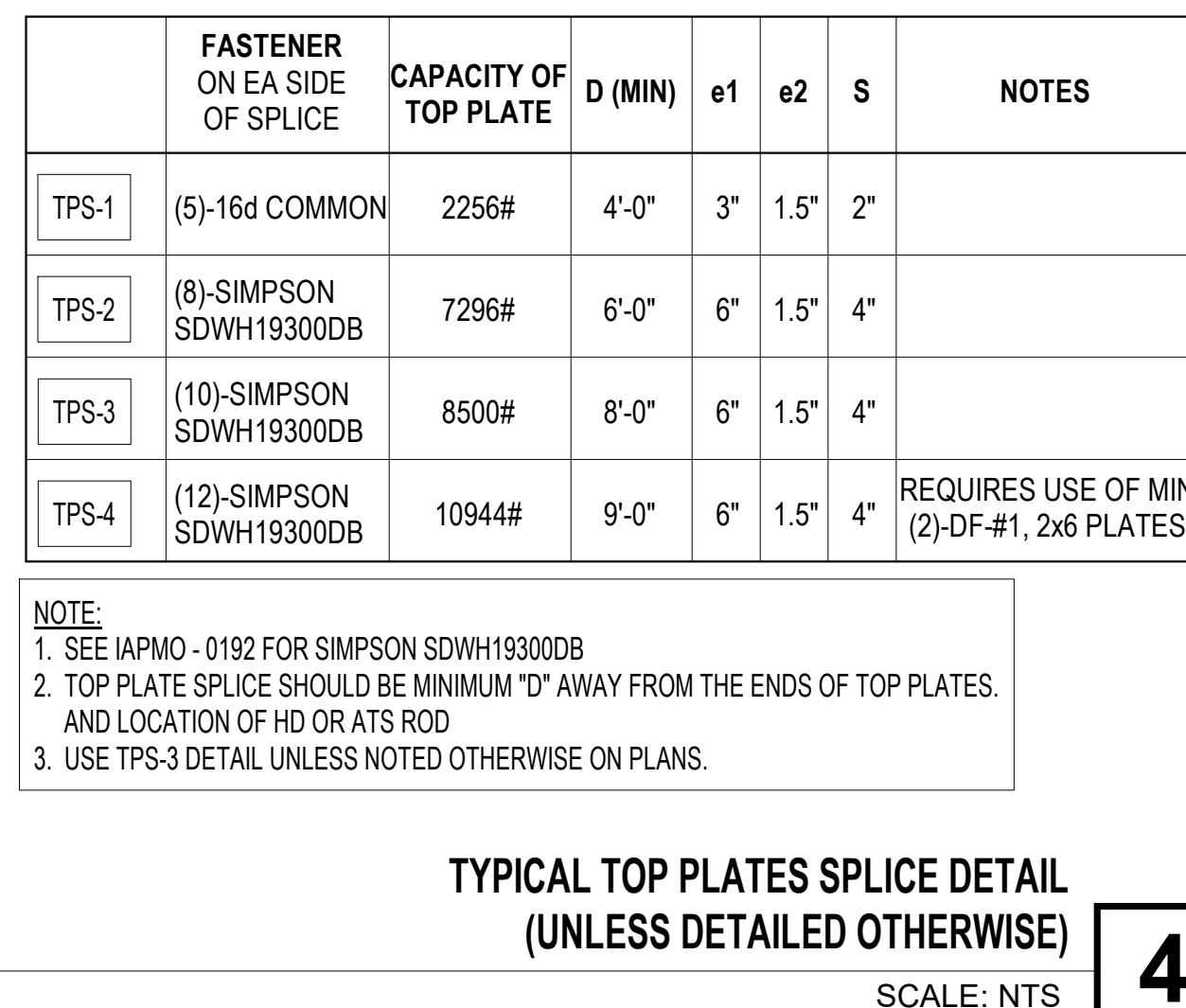
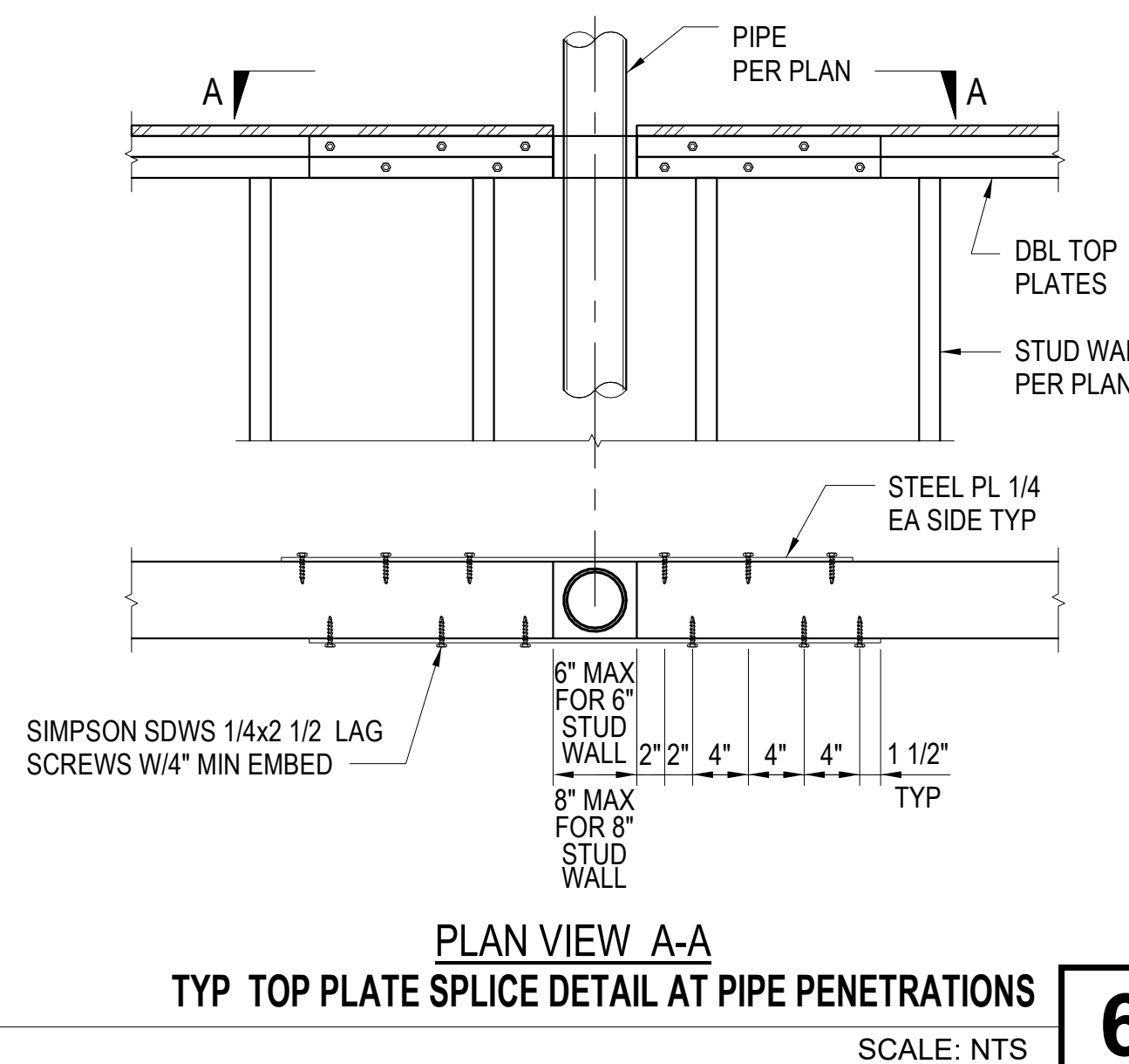
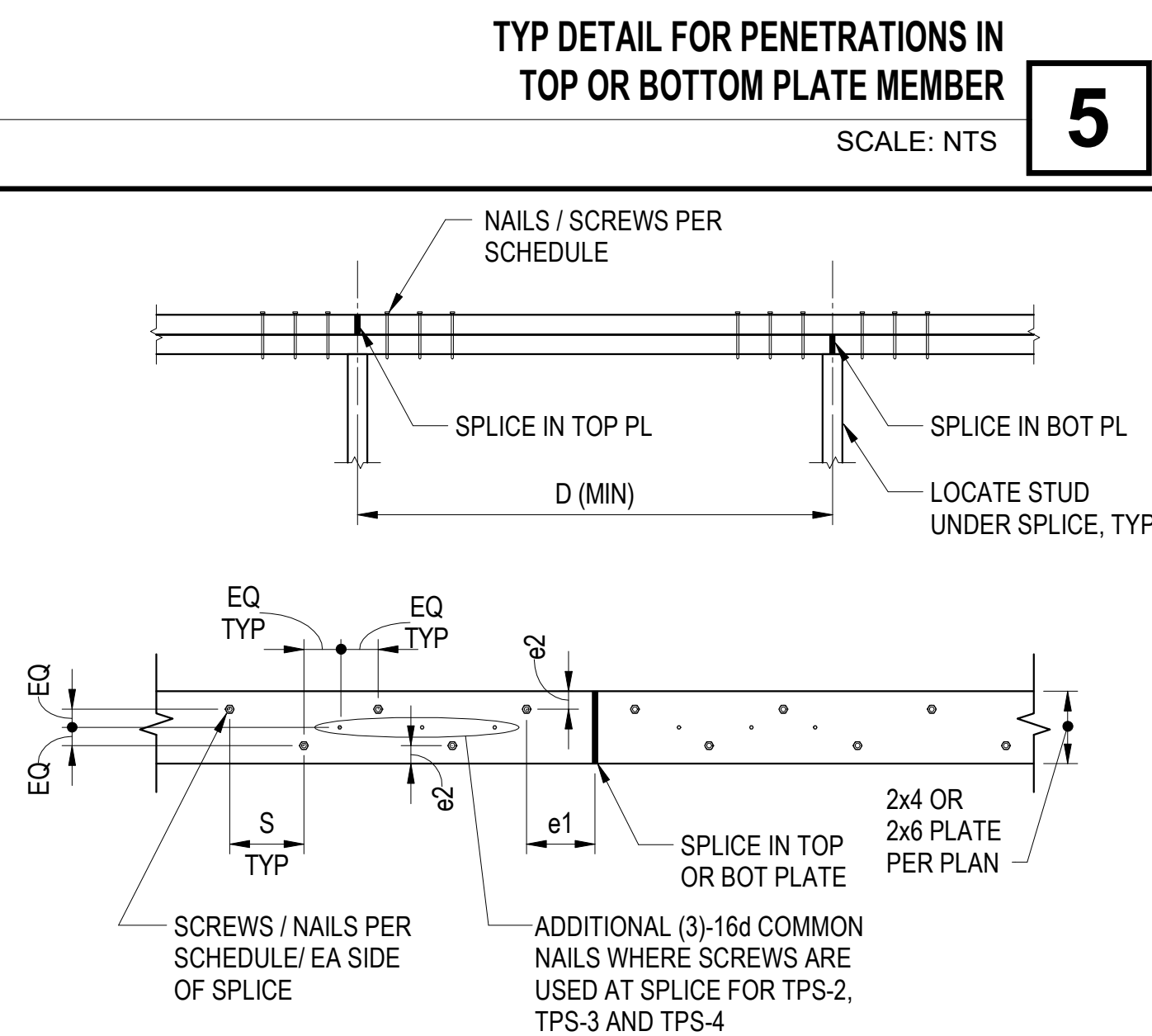
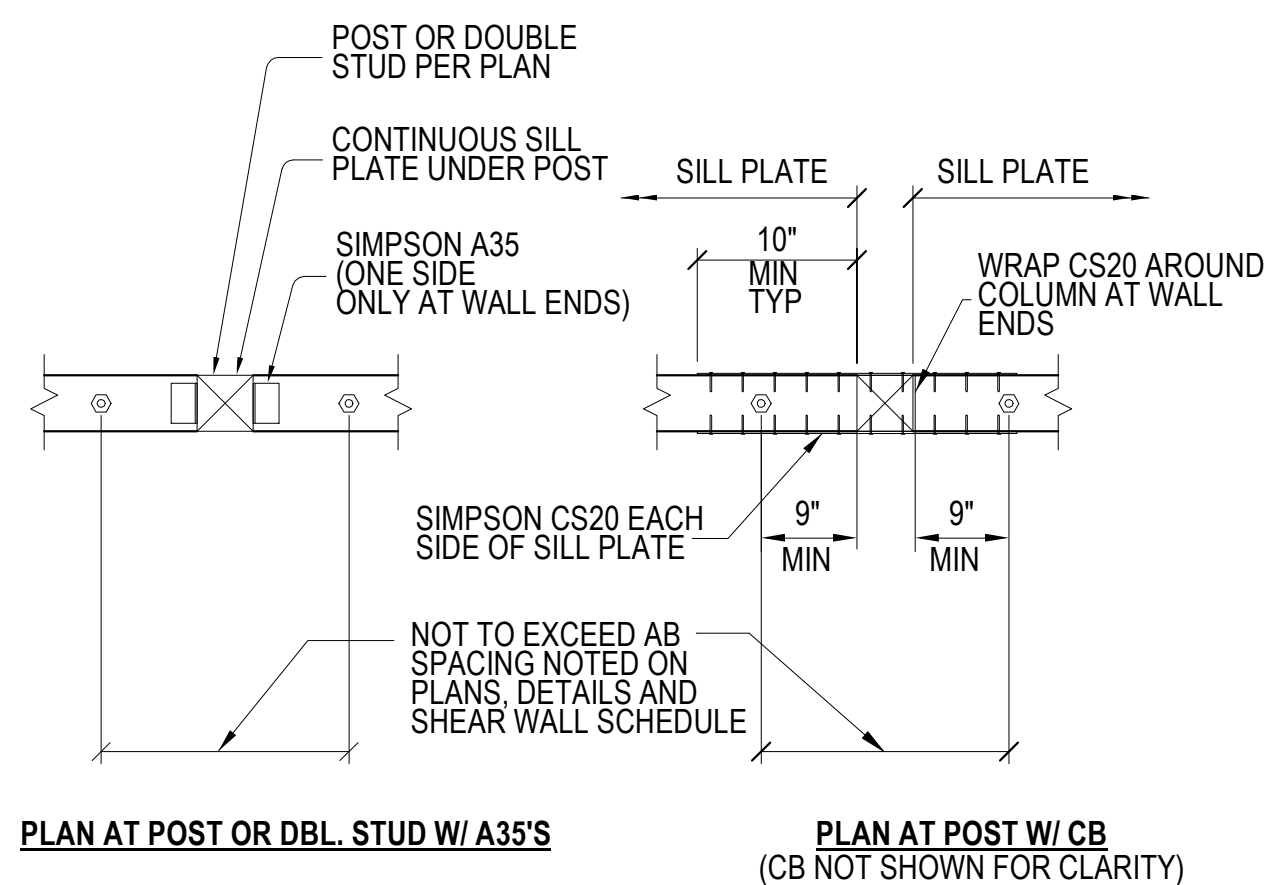
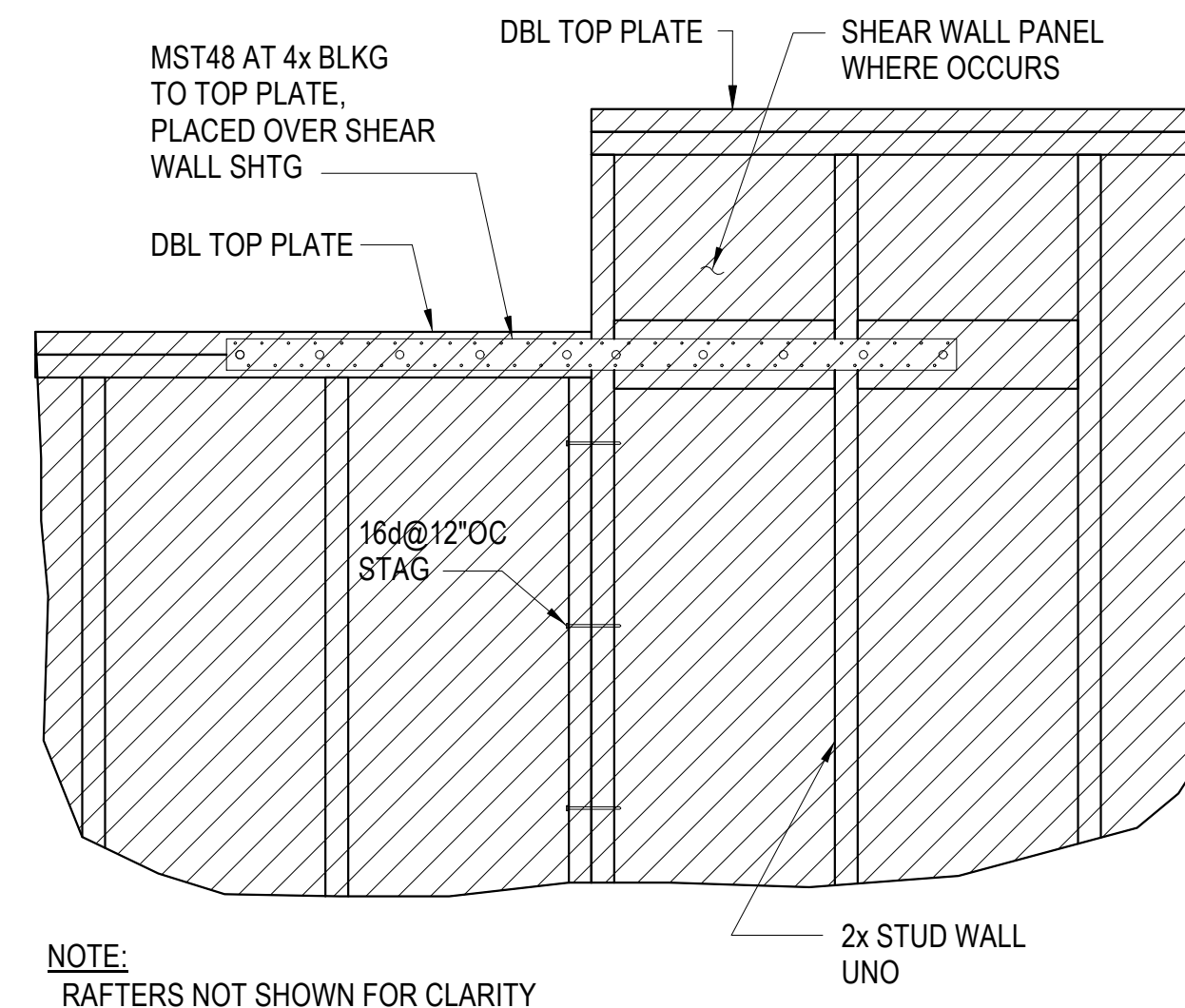
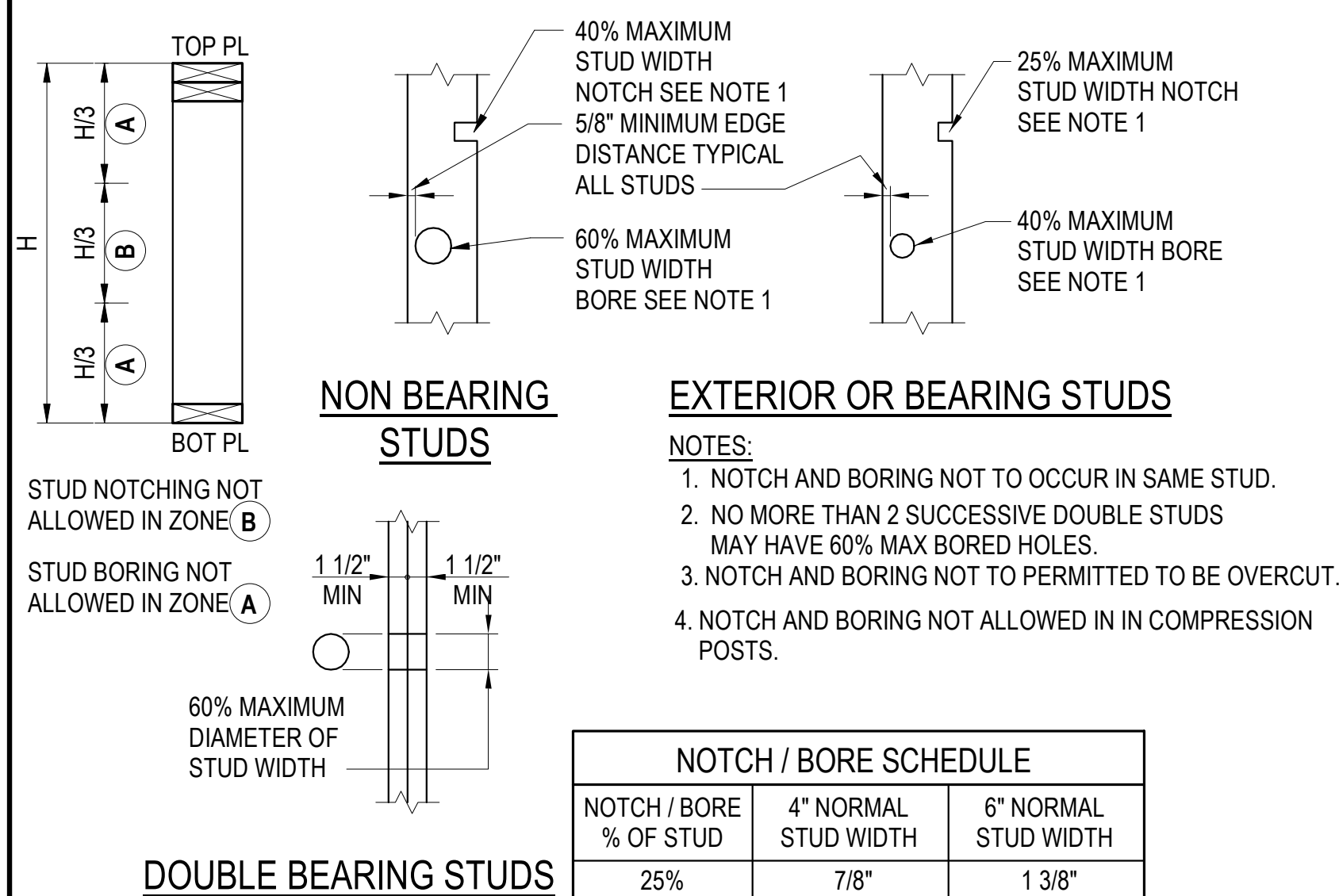
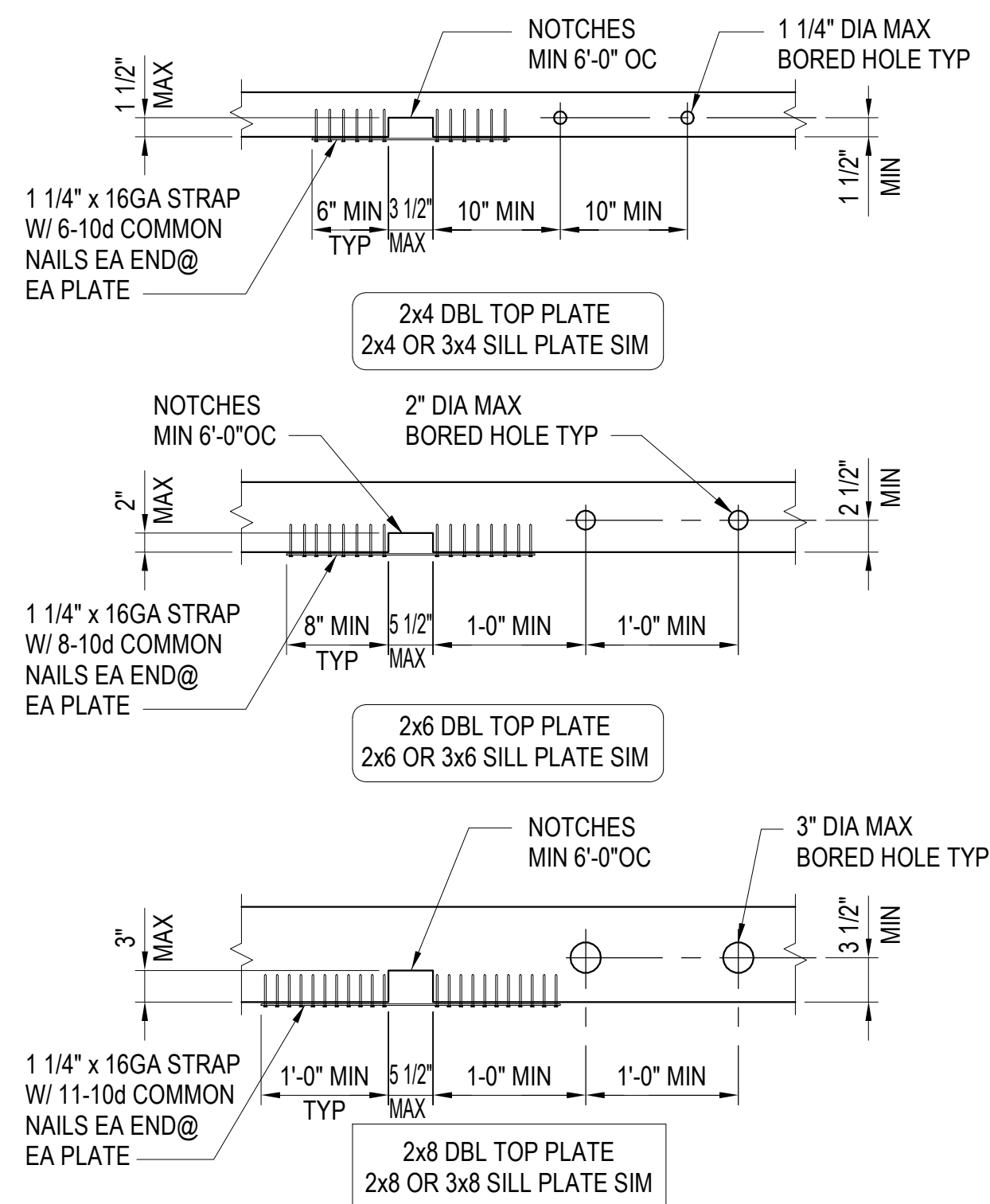
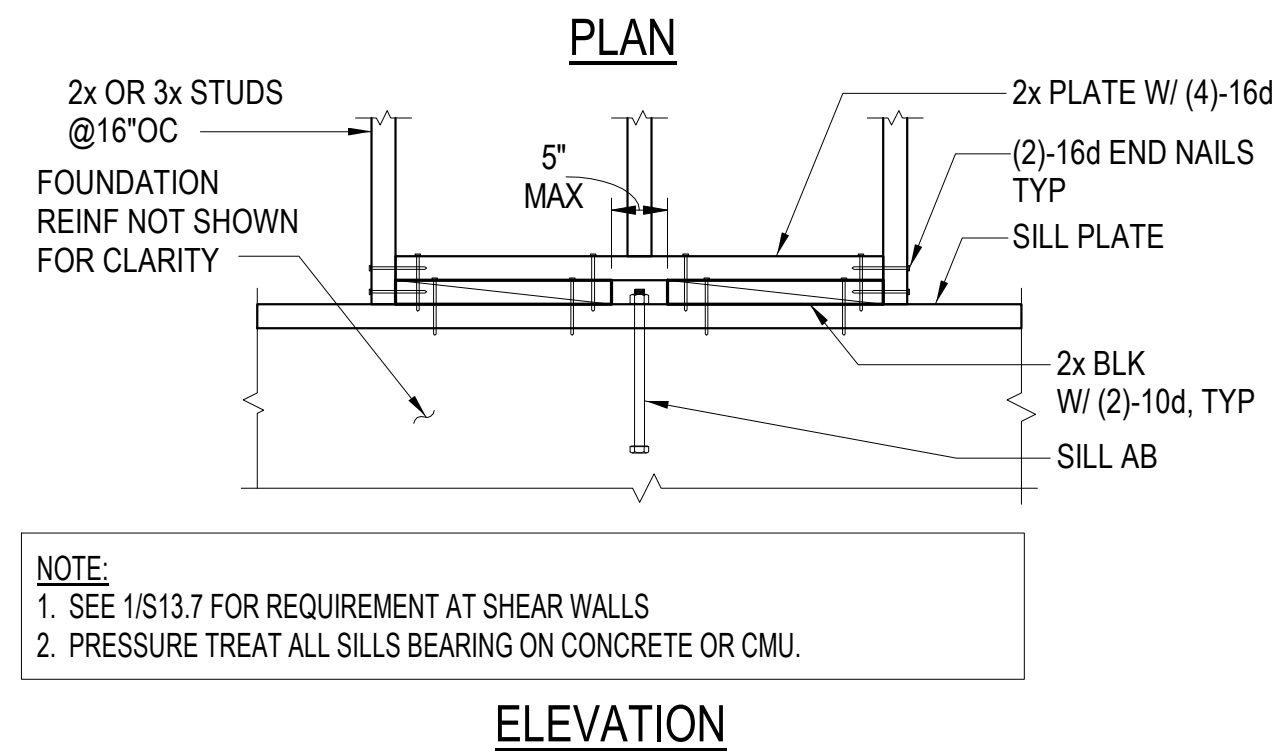
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DATE
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JOB NO.

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AS NOTED
PROJECT NUMBER

SO21

APPENDIX 5



WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995



saiful-bouquet

7725 S. Figueroa St.,
37th Floor
Los Angeles, CA 90017
www.safulbouquet.com
(213) 315-2277
Project #25534

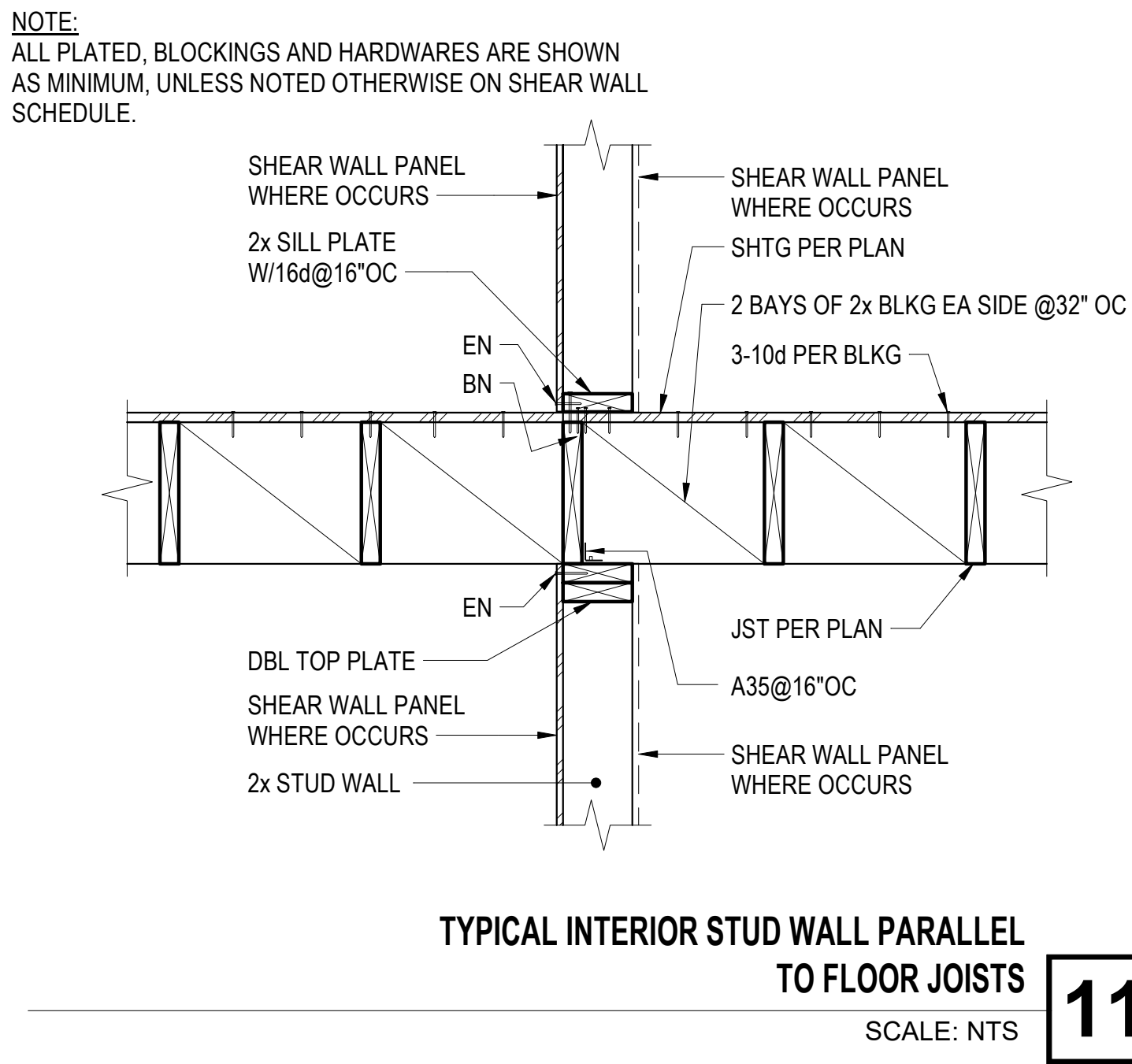
TYPICAL WOOD DETAILS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

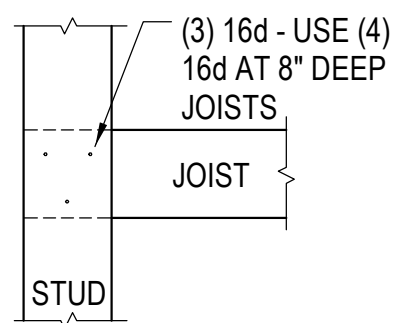
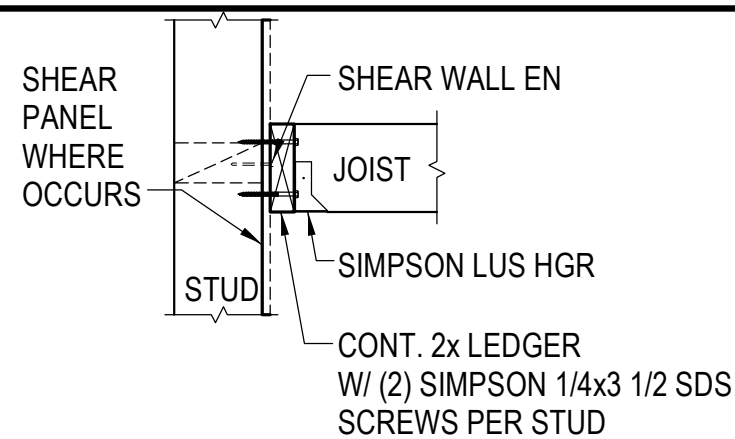
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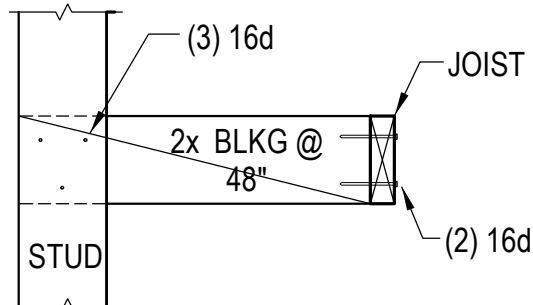
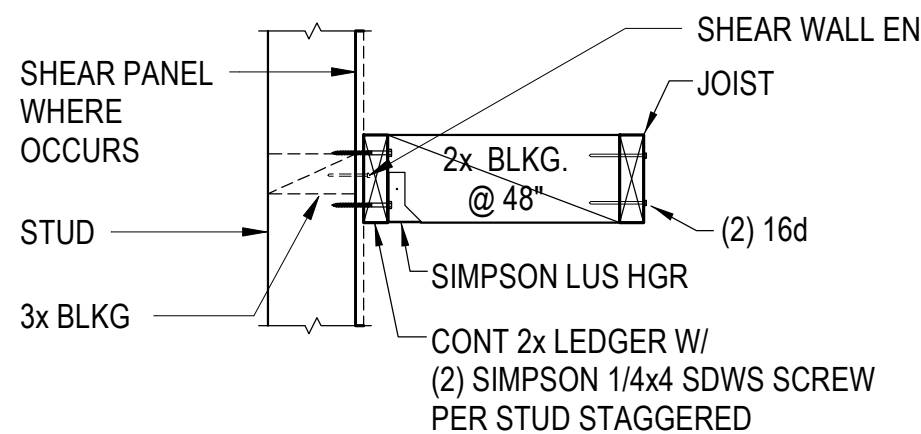
PLAN CHECK SUBMITTAL - October 31 2025



11



PERPENDICULAR CONDITION WHERE JOISTS CAN LAP STUDS

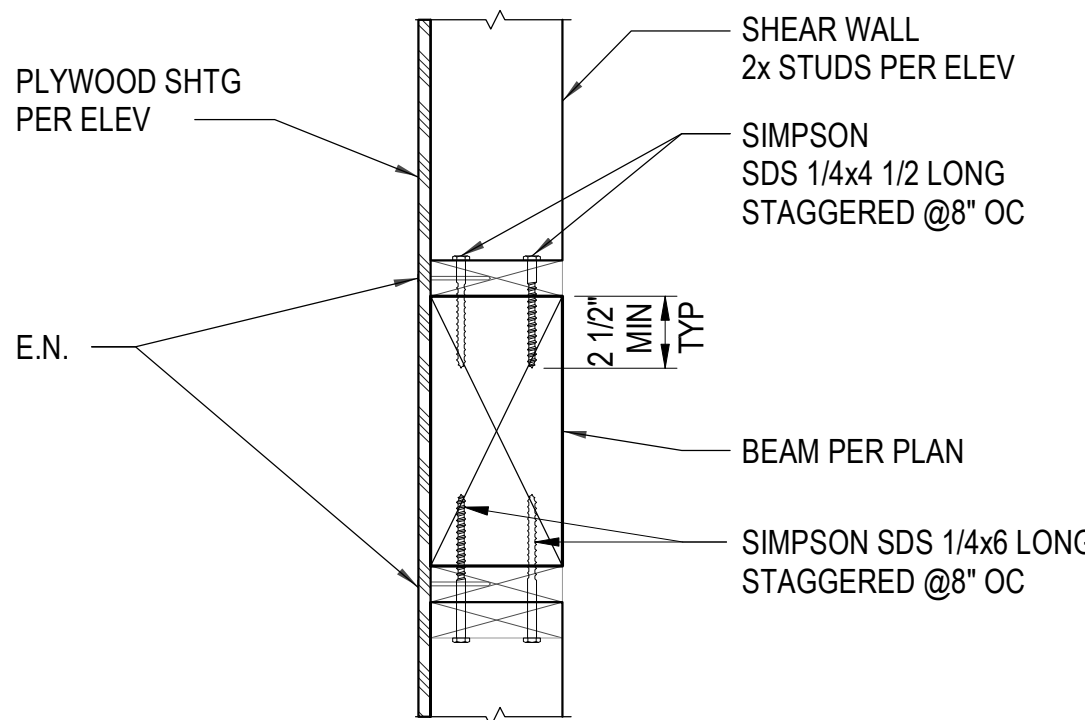


PARALLEL CONDITION WHERE BLOCKING CAN LAP STUDS

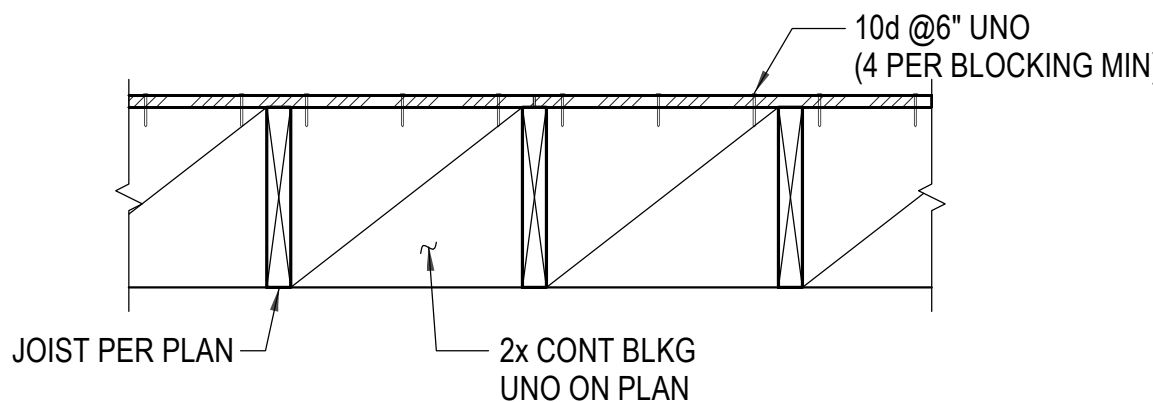
TYP CEILING JOIST TO STUD WALL CONNECTION
SCALE: NTS

10

SCHEDULE		
JOIST	SPACING	MAX SPAN
2x4	12"	12'-0"
	16"	11'-0"
	24"	9'-0"
2x6	12"	19'-0"
	16"	17'-0"
	24"	15'-0"
2x8	12"	25'-0"
	16"	23'-0"
	24"	20'-0"

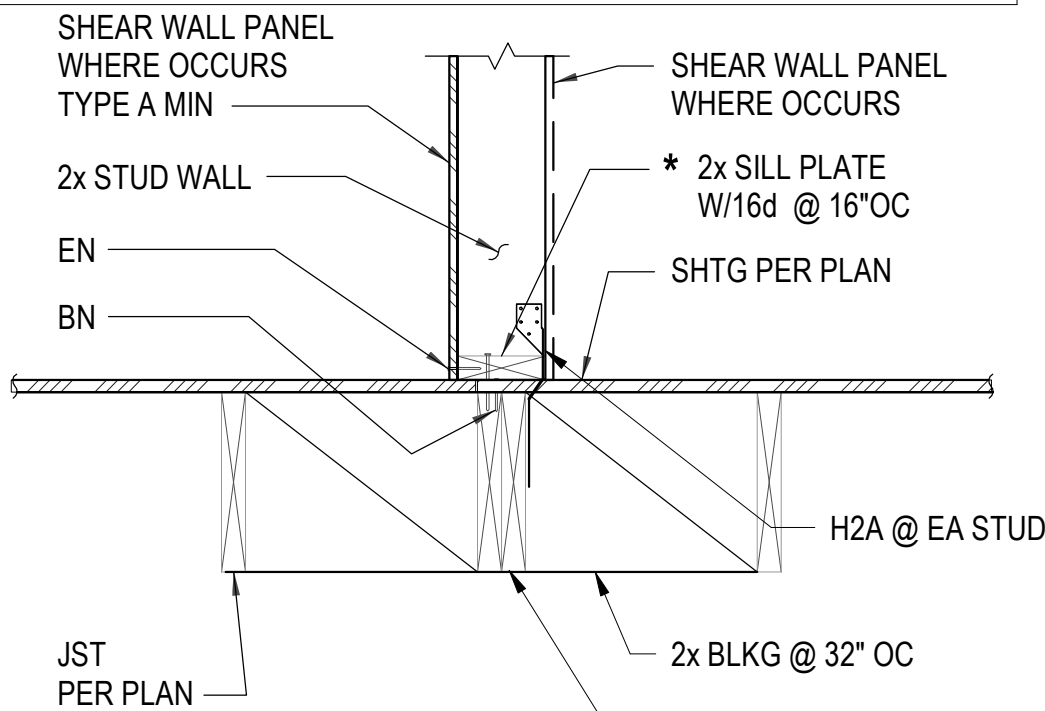


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8

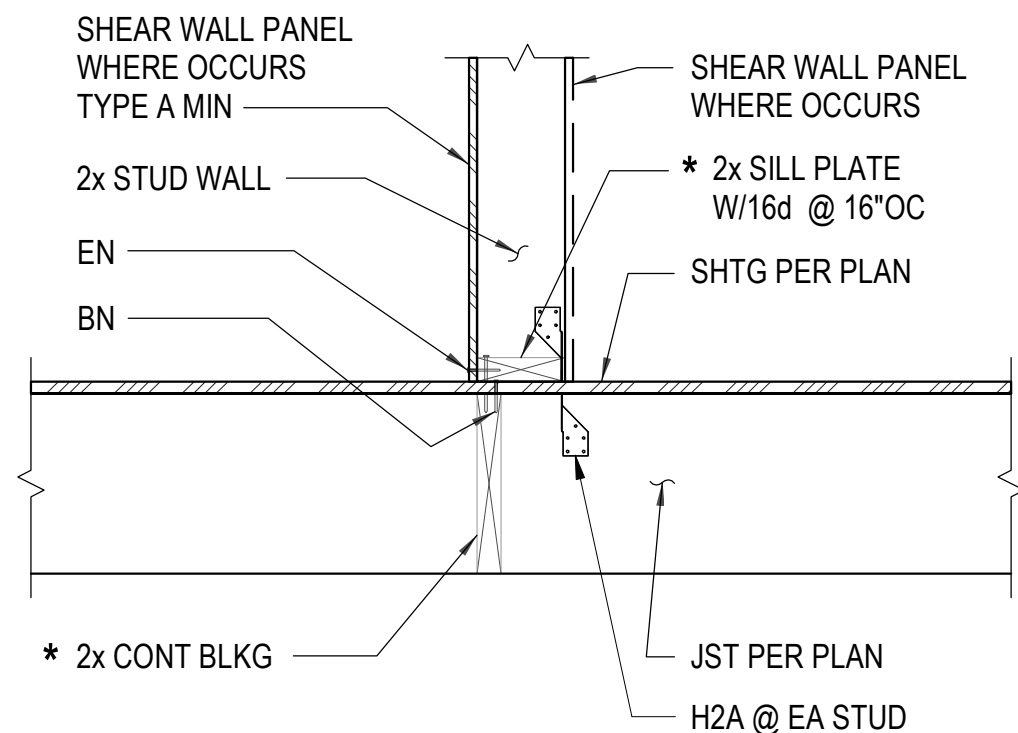
NOTE: ALL PLATES, BLOCKING, JOISTS, NAILING AND HARDWARES ARE SHOWN AS A MINIMUM UNLESS NOTED OTHERWISE ON SHEAR WALL SCHEDULE AND DIAPHRAGM REQUIREMENTS. REFER TO SHEAR WALL SCHEDULE PLUS DIAPHRAGM DETAILS AND CALLOUTS TO DETERMINE IF LARGER MEMBER SIZES, MORE RESTRICTIVE NAILING OR CONNECTOR SIZE AND SPACING IS REQUIRED.



7

* INDICATES CHECK W/ S.W. OR DIAPHRAGM SCHEDULE & DETAILS FOR MORE RESTRICTIVE REQUIREMENTS. ALSO SEE TYP. SHEAR WALL ATTACHMENT WHERE SILL PLATE SCREWS ARE USED FOR ADDITIONAL REQUIREMENT.

NOTE:
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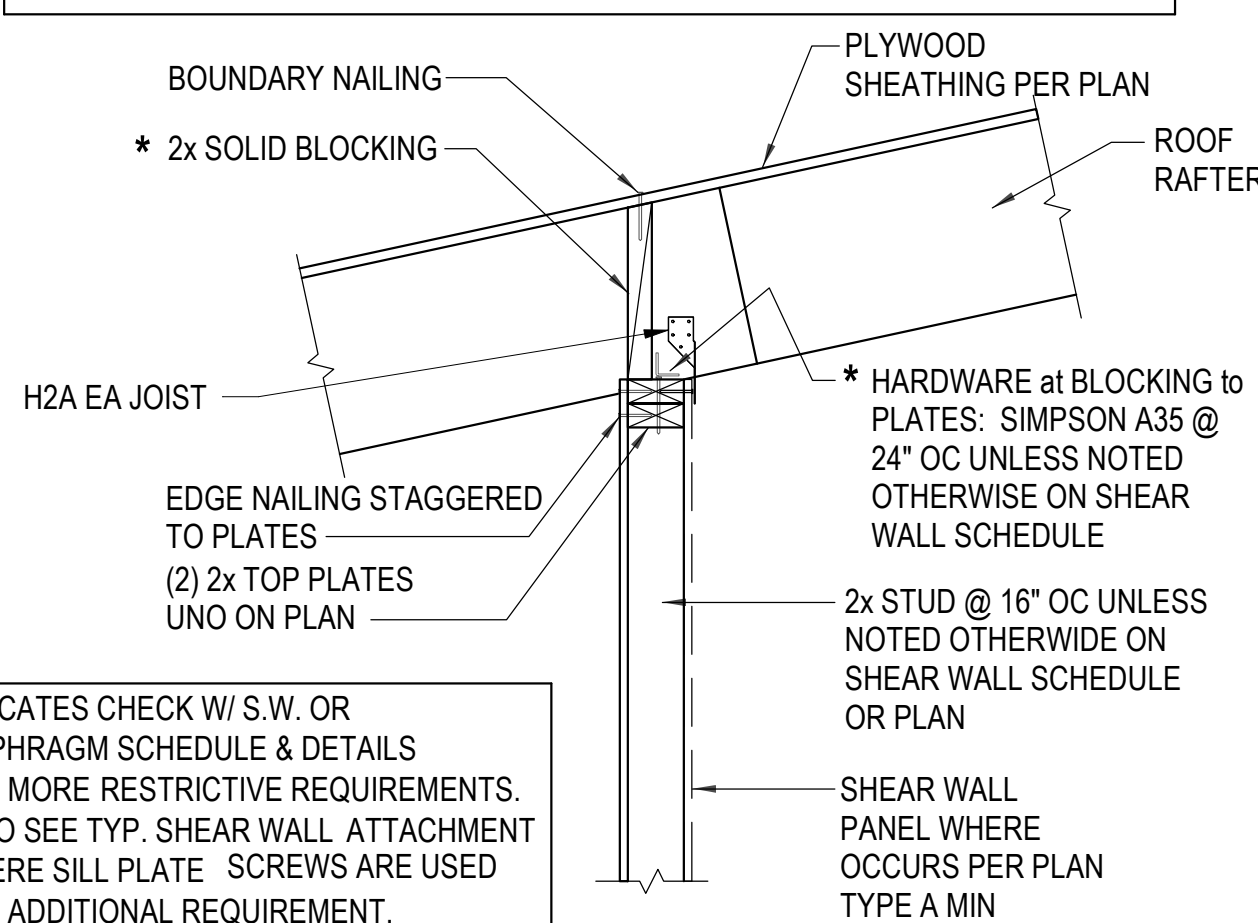


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TYPICAL CRIPPLE STUD PERPENDICULAR TO JOISTS DETAIL

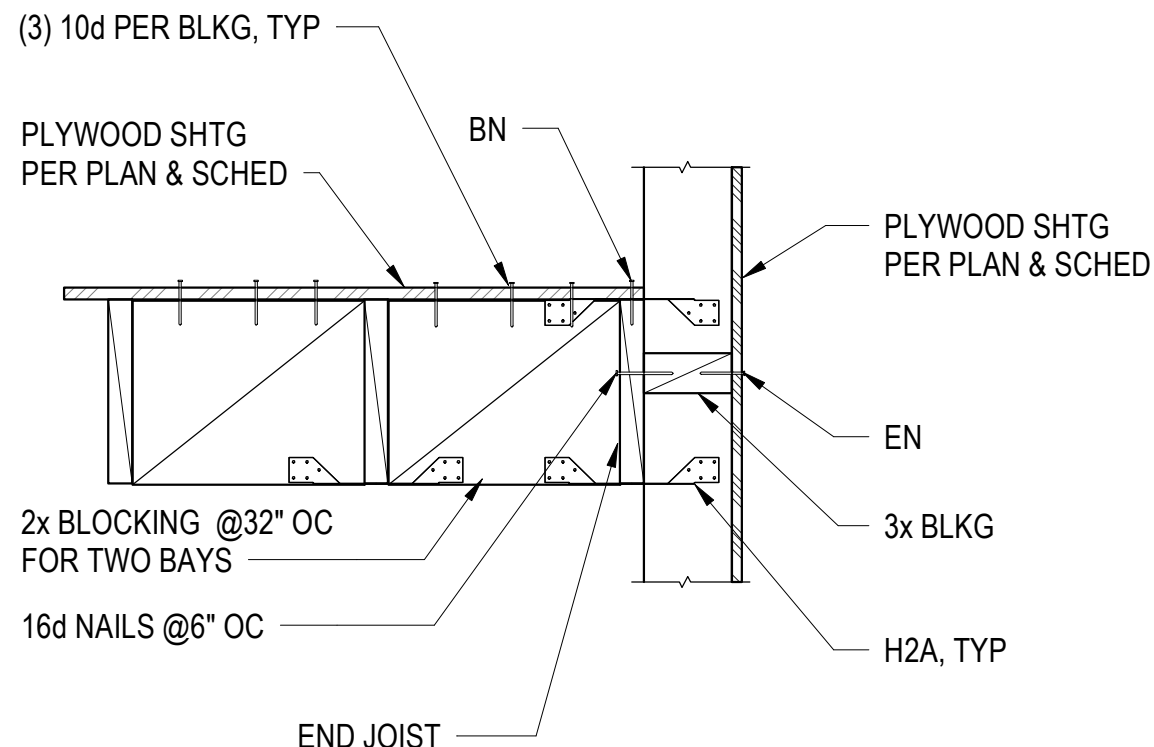
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NOTE:
ALL PLATES, BLOCKING, JOISTS, NAILING AND HARDWARES ARE SHOWN AS A MINIMUM UNLESS NOTED OTHERWISE ON SHEAR WALL SCHEDULE AND DIAPHRAGM REQUIREMENTS. REFER TO SHEAR WALL SCHEDULE PLUS DIAPHRAGM DETAILS AND CALLOUTS TO DETERMINE IF LARGER MEMBER SIZES, MORE RESTRICTIVE NAILING OR CONNECTOR SIZE AND SPACING IS REQUIRED.



TYPICAL STUD WALL PERPENDICULAR TO ROOF RAFTERS

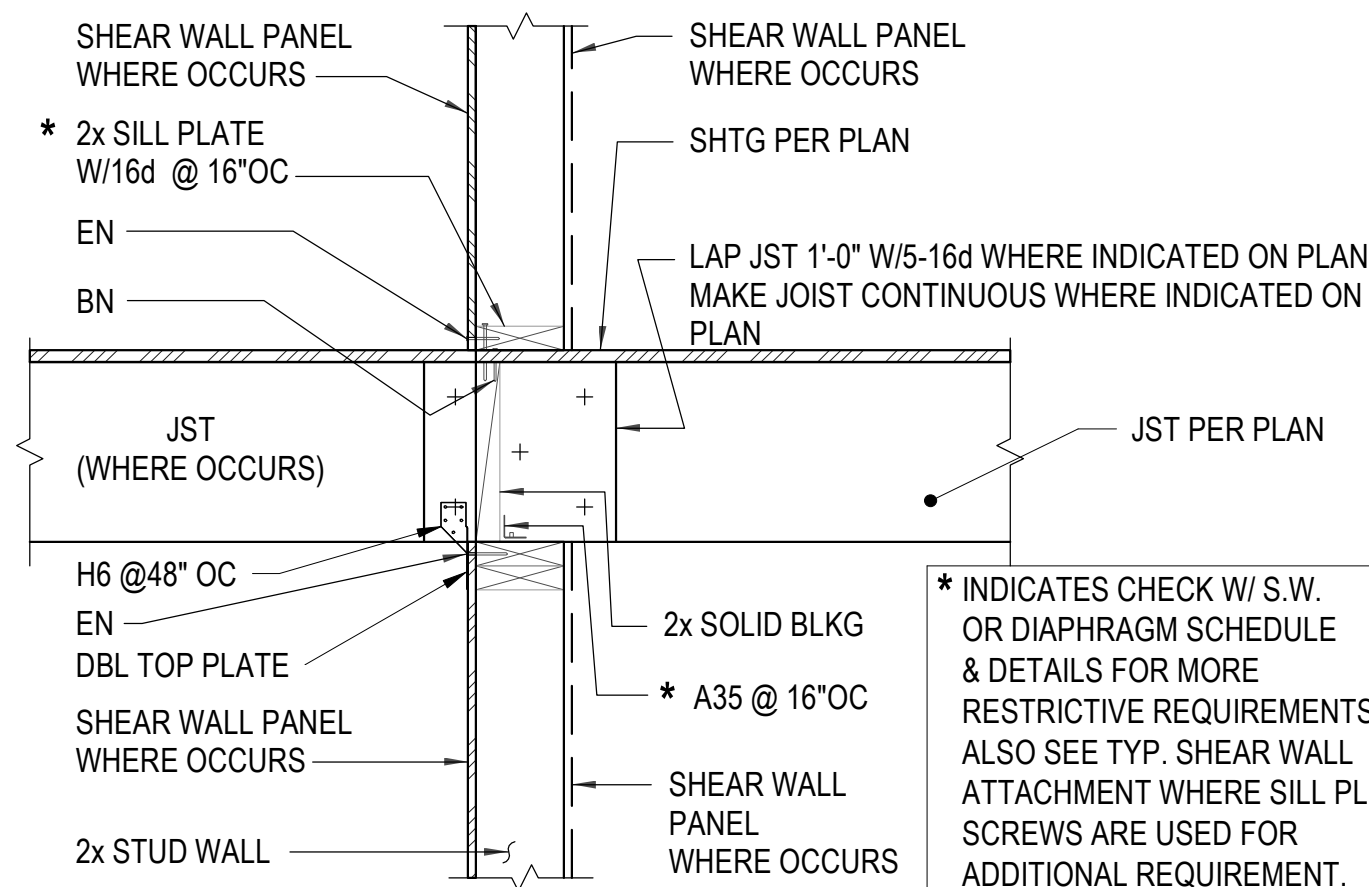
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4

TYPICAL LEDGER TO WALL CONNECTION DETAIL
SCALE: NTS

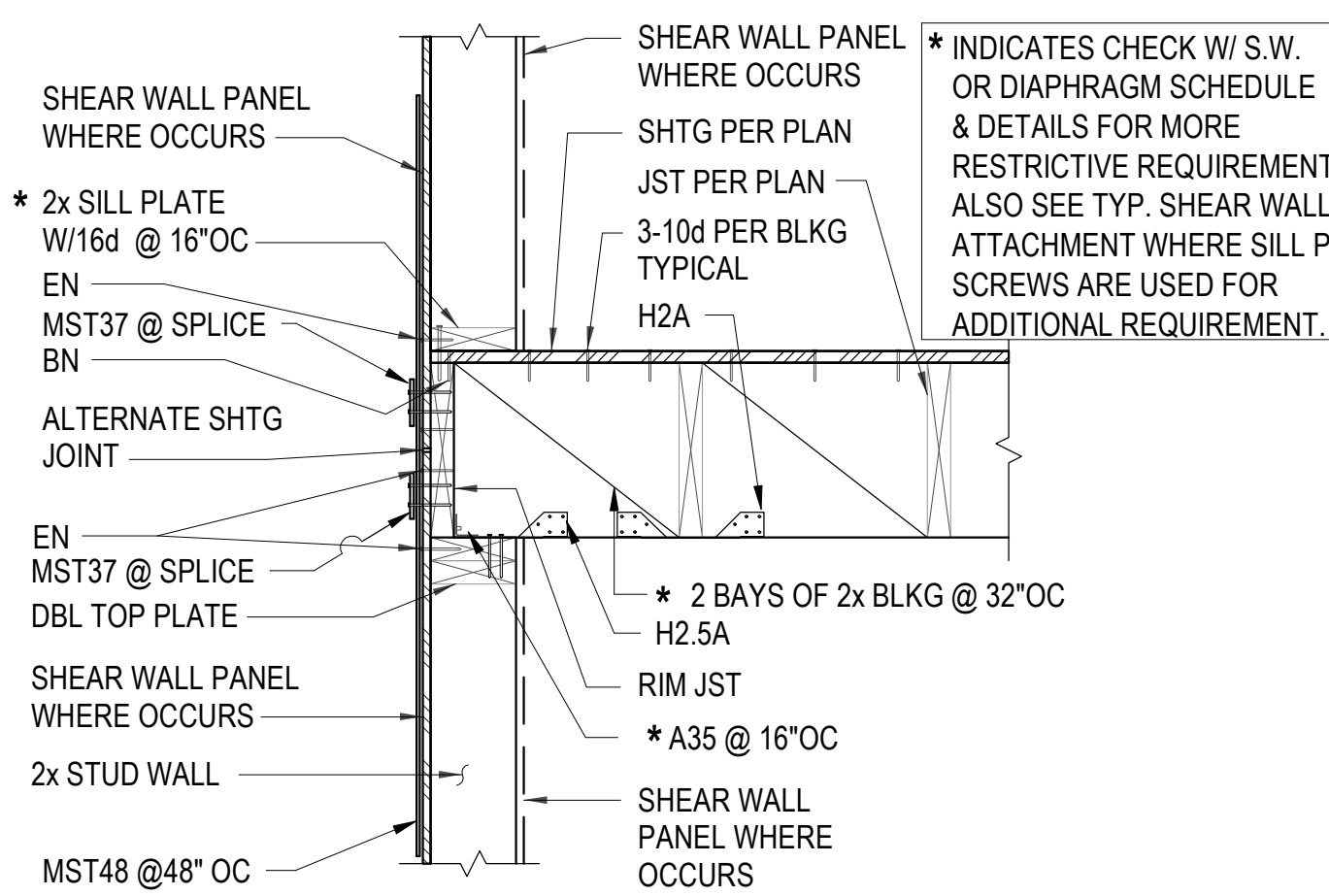
NOTE:
ALL PLATES, BLOCKING, JOISTS, NAILING AND HARDWARES ARE SHOWN AS A MINIMUM UNLESS NOTED OTHERWISE ON SHEAR WALL SCHEDULE AND DIAPHRAGM REQUIREMENTS. REFER TO SHEAR WALL SCHEDULE PLUS DIAPHRAGM DETAILS AND CALLOUTS TO DETERMINE IF LARGER MEMBER SIZES, MORE RESTRICTIVE NAILING OR CONNECTOR SIZE AND SPACING IS REQUIRED.



3

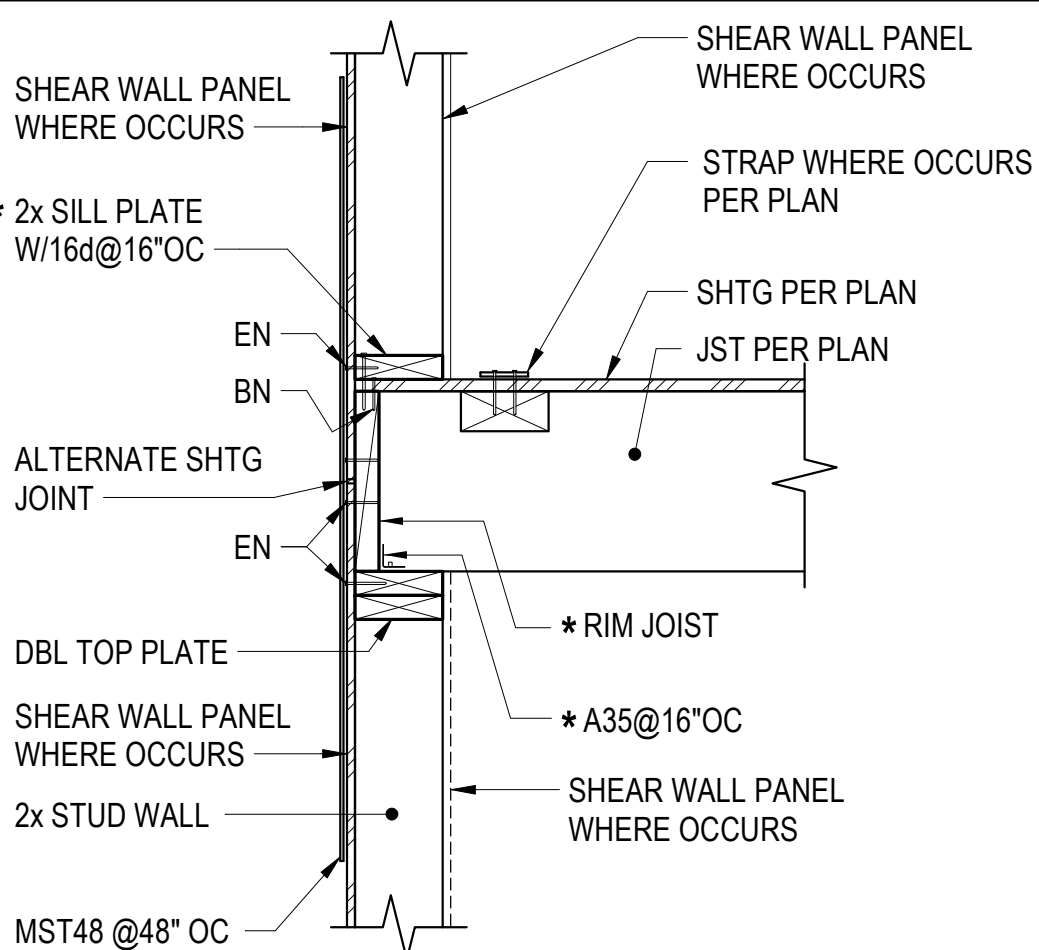
TYPICAL INTERIOR WALL TO PERP JOISTS DETAIL

NOTE: ALL PLATES, BLOCKING, JOISTS, NAILING AND HARDWARES ARE SHOWN AS A MINIMUM UNLESS NOTED OTHERWISE ON SHEAR WALL SCHEDULE AND DIAPHRAGM REQUIREMENTS. REFER TO SHEAR WALL SCHEDULE PLUS DIAPHRAGM DETAILS AND CALLOUTS TO DETERMINE IF LARGER MEMBER SIZES, MORE RESTRICTIVE NAILING OR CONNECTOR SIZE AND SPACING IS REQUIRED.



2

TYPICAL EXTERIOR STUD WALL PARALLEL TO FLOOR JOISTS
SCALE: NTS



1

TYPICAL EXTERIOR STUD WALL PERPENDICULAR TO JOISTS DETAIL
SCALE: NTS

NOTE
ALL PLATES, BLOCKING, JOIST, NAILING AND HARDWARE ARE SHOWN AS A MINIMUM UNLESS NOTED OTHERWISE ON SHEAR WALL SCHEDULE AND DIAPHRAGM REQUIREMENTS. REFER TO SHEAR WALL SCHEDULE PLUS DIAPHRAGM DETAILS AND CALLOUTS TO DETERMINE IF LARGER MEMBER SIZES, MORE RESTRICTIVE NAILING OR CONNECTOR SIZE AND SPACING IS REQUIRED.

* INDICATES CHECK W/ SW OR DIAPHRAGM SCHEDULE & DETAILS FOR MORE RESTRICTIVE REQUIREMENTS. ALSO SEE TYP SHEAR WALL ATTACHMENT WHERE SILL PLATE SCREWS ARE USED FOR ADDITIONAL REQUIREMENTS

WILLIAM LOYD JONES ARCHITECT
9415 culver boulevard
culver city, california 90230
www.willlloydjones.com
TEL 310 392 3995

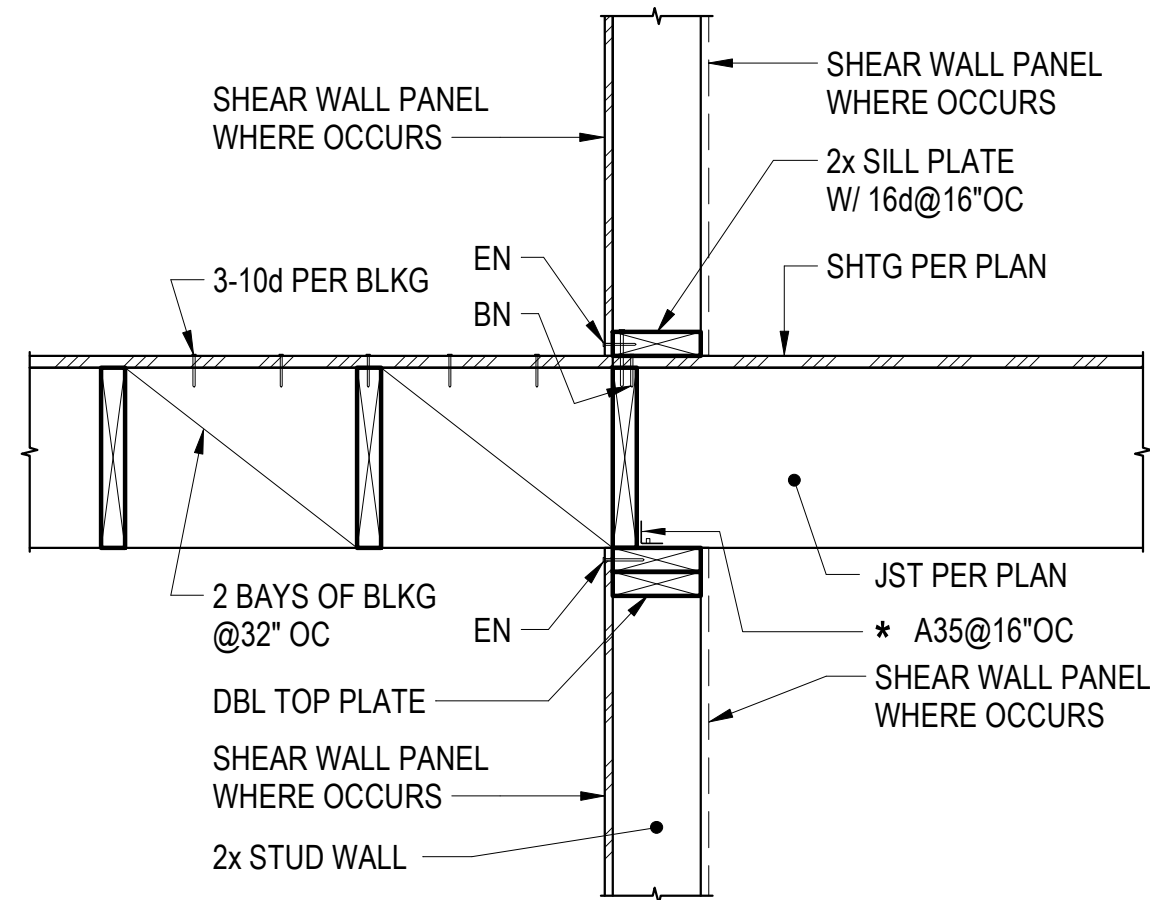
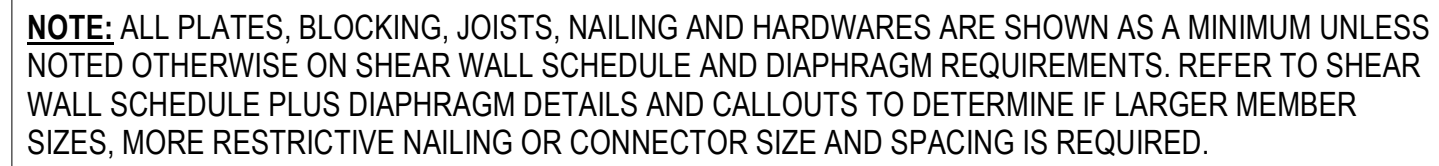
SO
structural engineers
725 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
(213) 315-2277
Project #25534

SECTIONS AND TYPICAL DETAILS
FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S3289
Exp. 12/31/27
STRUCTURAL
STATE OF CALIFORNIA

DATE _____ **ISSUE DATE** _____
DRAWN _____
CHECKED _____
SCALE _____ **AS NOTED**
JOB NO. _____ **PROJECT NUMBER** _____

SO
ADDENDUM 5
31

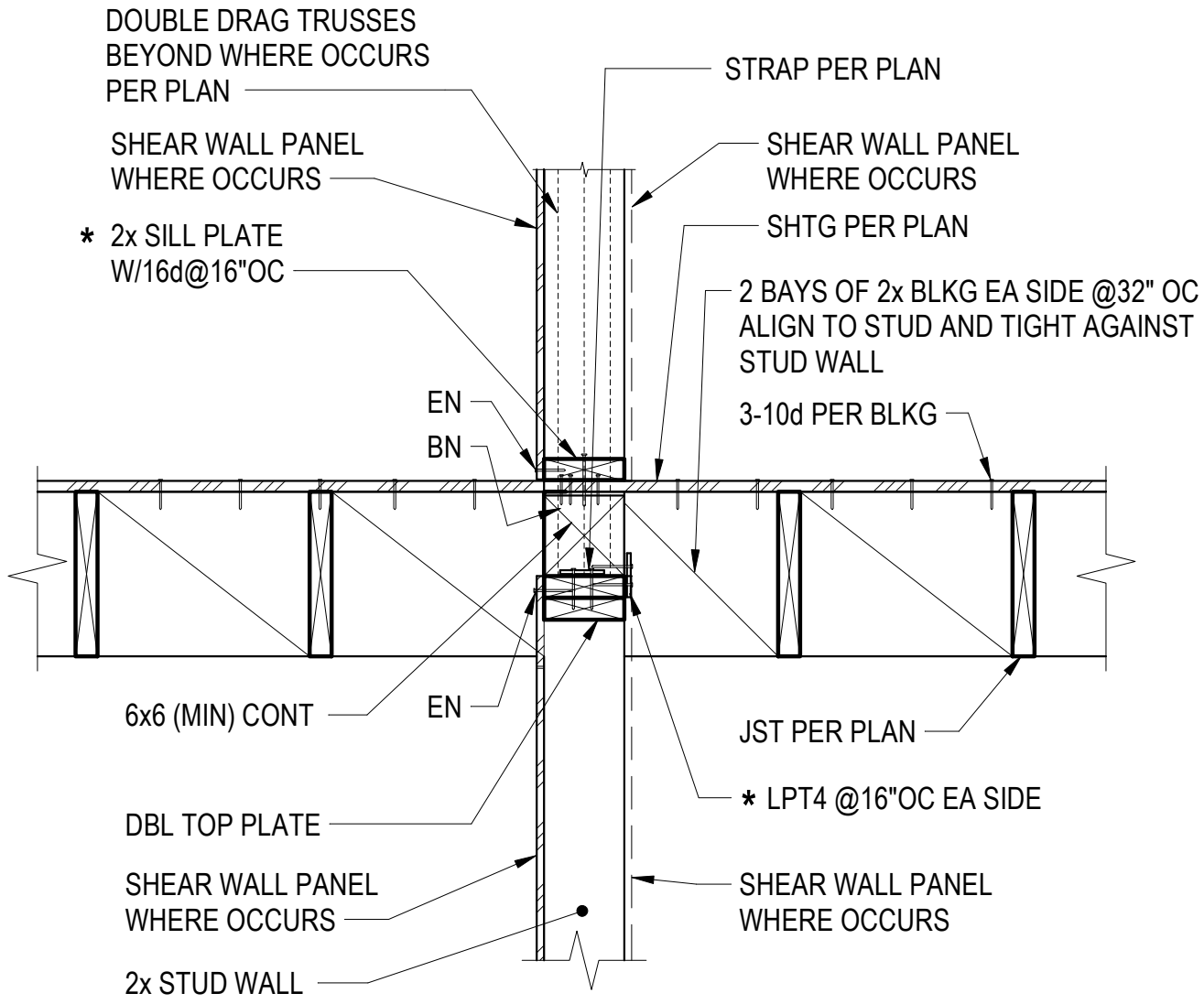


* INDICATES CHECK W/ SW OR DIAPHRAGM SCHEDULE
& DETAILS FOR MORE RESTRICTIVE REQUIREMENTS.
ALSO SEE TYP. SHEAR WALL ATTACHMENT WHERE SILL
PLATE SCREWS ARE USED FOR ADDITIONAL REQUIREMENT.

TYPICAL

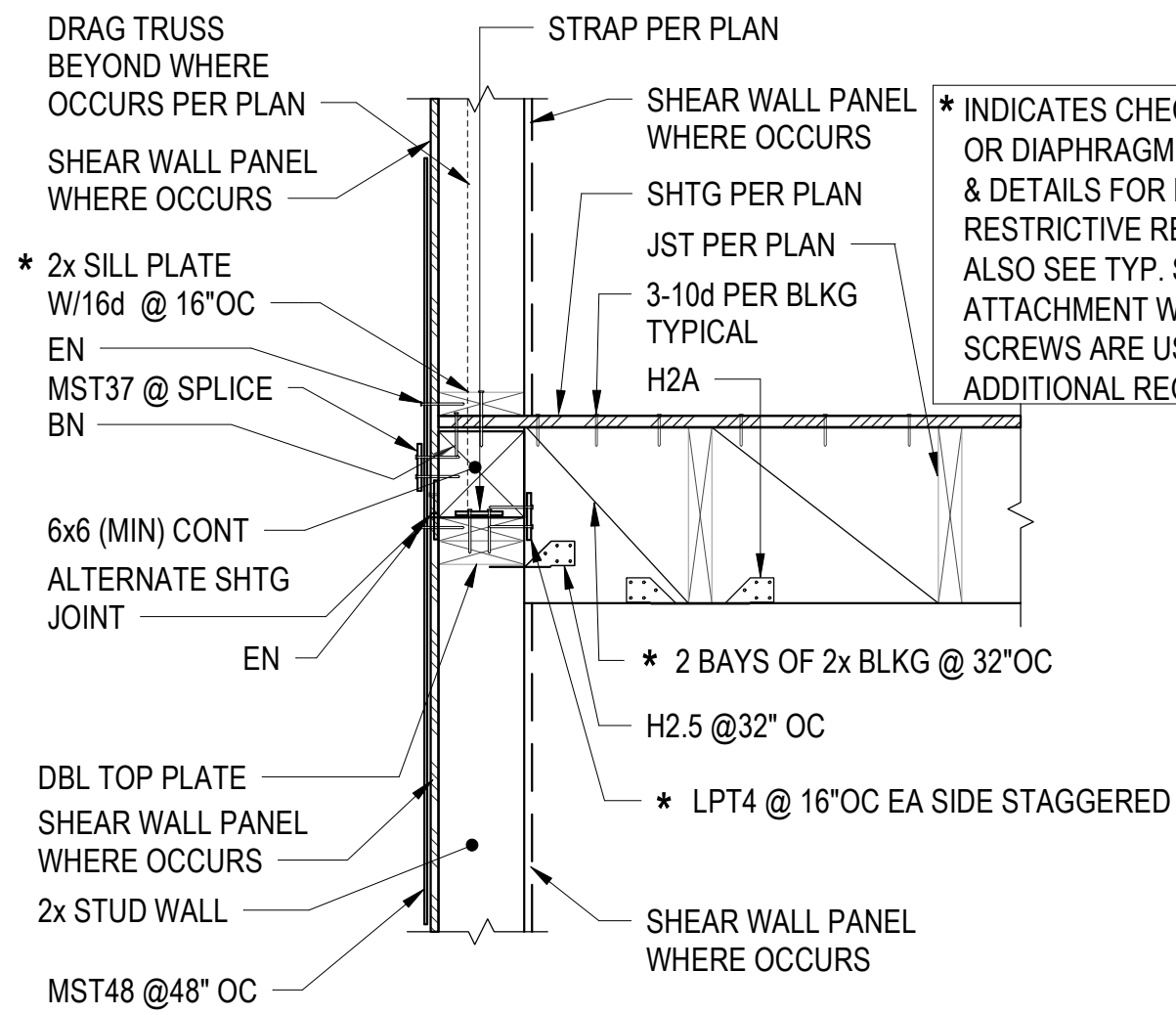
SCALE: NTS

2



INTERIOR CONDITION

NOTE: ALL PLATES, BLOCKING, JOISTS, NAILING AND HARDWARES ARE SHOWN AS A MINIMUM UNLESS NOTED OTHERWISE ON SHEAR WALL SCHEDULE AND DIAPHRAGM REQUIREMENTS. REFER TO SHEAR WALL SCHEDULE PLUS DIAPHRAGM DETAILS AND CALLOUTS TO DETERMINE IF LARGER MEMBER SIZES, MORE RESTRICTIVE NAILING OR CONNECTOR SIZE AND SPACING IS REQUIRED.



* INDICATES CHECK W/ S.W.
OR DIAPHRAGM SCHEDULE
& DETAILS FOR MORE
RESTRICTIVE REQUIREMENTS
ALSO SEE TYP. SHEAR WALL
ATTACHMENT WHERE SILL PL.
SCREWS ARE USED FOR
ADDITIONAL REQUIREMENT.

EXTERIOR CONDITION

**TYPICAL EXTERIOR STUD WALL PARALLEL TO FLOOR JOISTS
WITH WALL TOP PLATES HIGHER THAN BOTTOM OF JOIST**

SCALE: NTS

1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

0 2 3 2

TEL 310 392 3995



saiful-bouquet
structural engineers

725 S. Figueroa St.,
37th Floor
Los Angeles, CA 90017
www.saifubouquet.com
(213) 315-2277
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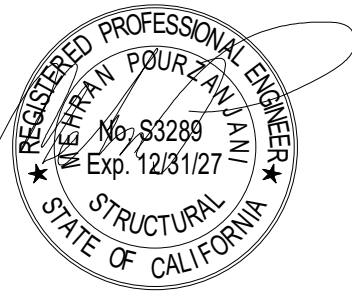
Project #25534

TYPICAL WOOD DETAILS

FIRE STATION 46

MISSION VILLAGE

COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

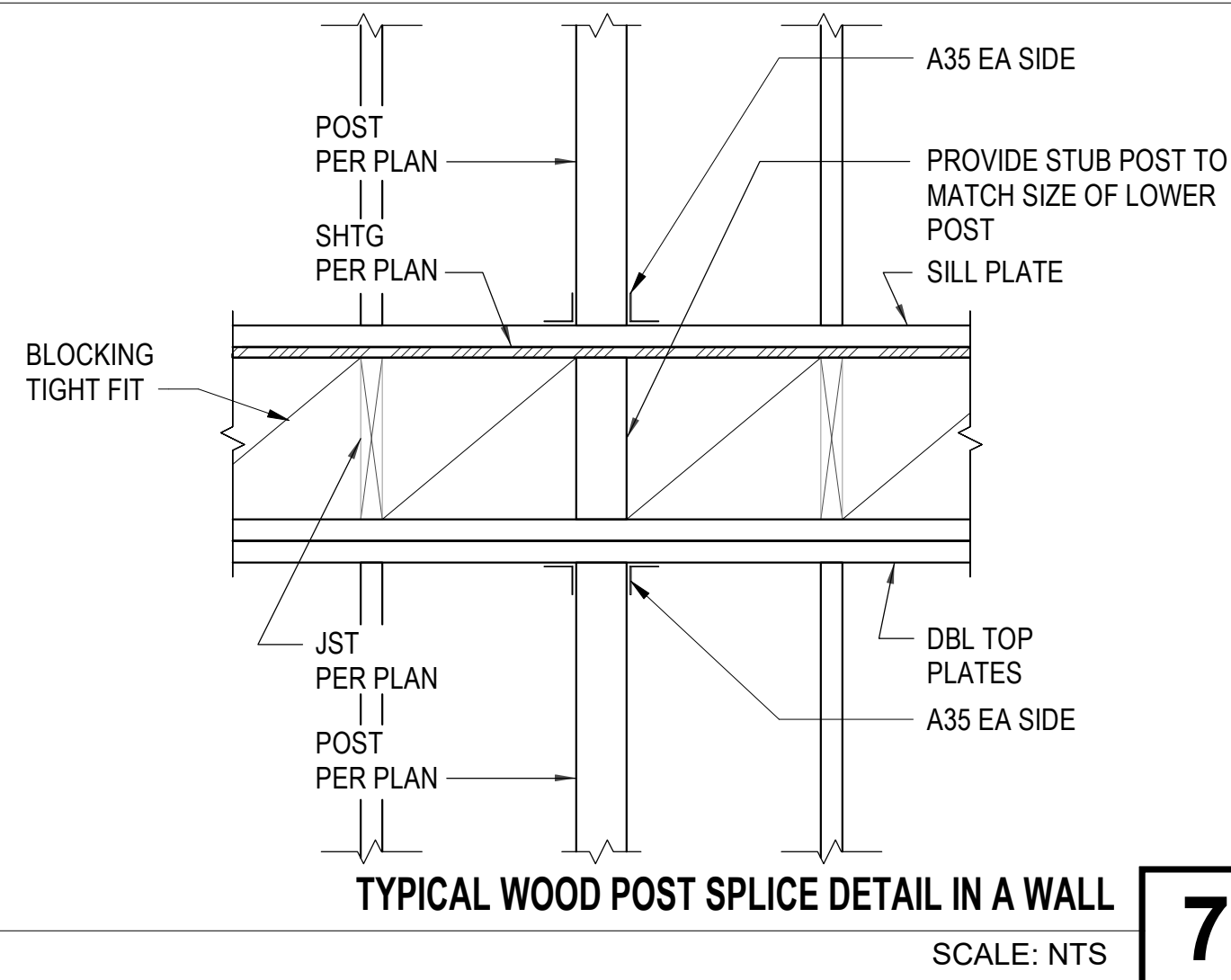


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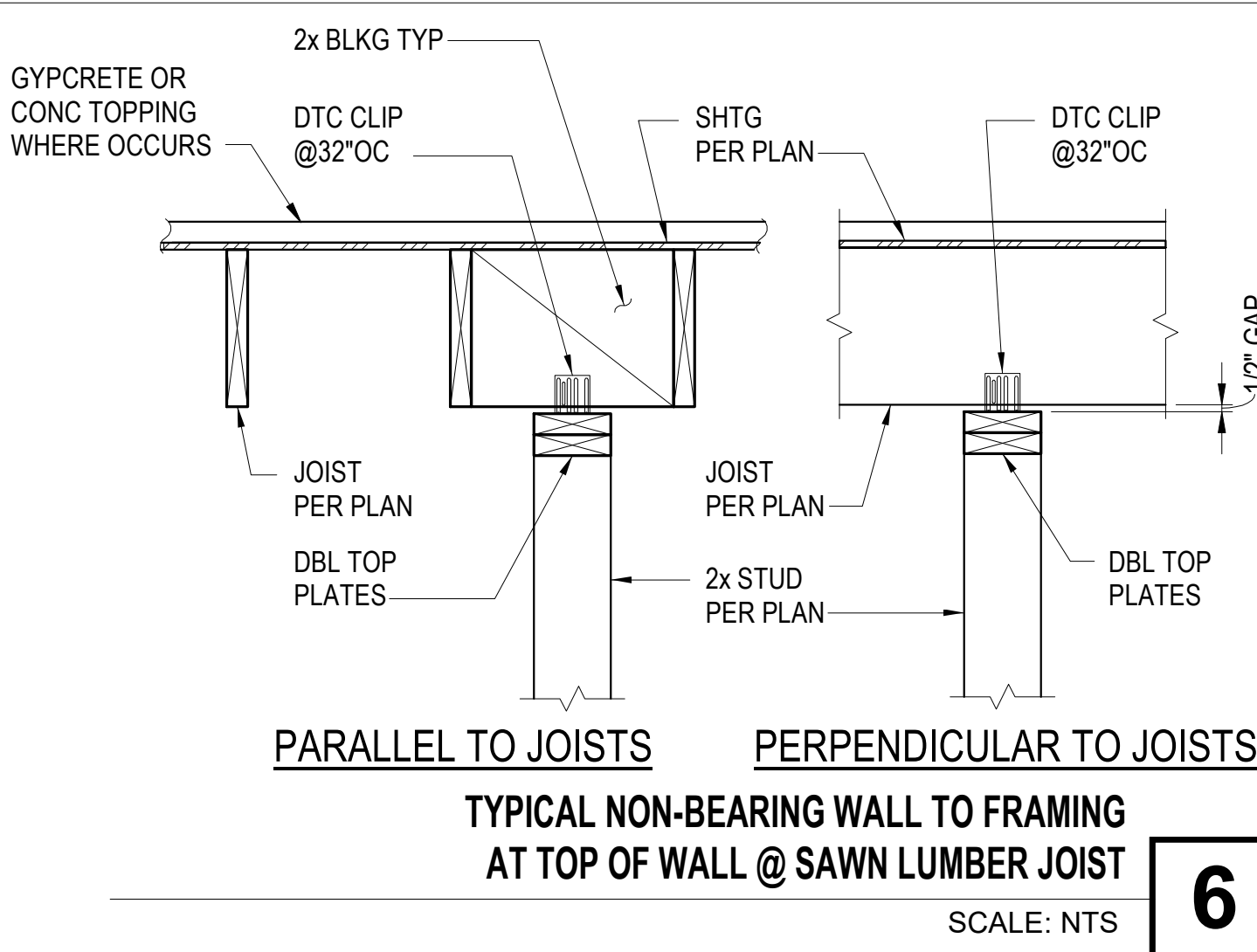
WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALED DIMENSIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS ON THE JOB AND ANY DISCREPANCIES MUST BE NOTIFIED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THESE DRAWINGS. SHOP CUTS SHALL BE SUBMITTED TO THIS OFFICE FOR APPROVAL BEFORE PROCEEDING WITH FABRICATION.

Date	Issue Date
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Scale	AS NOTED
Job. No.	Project Number

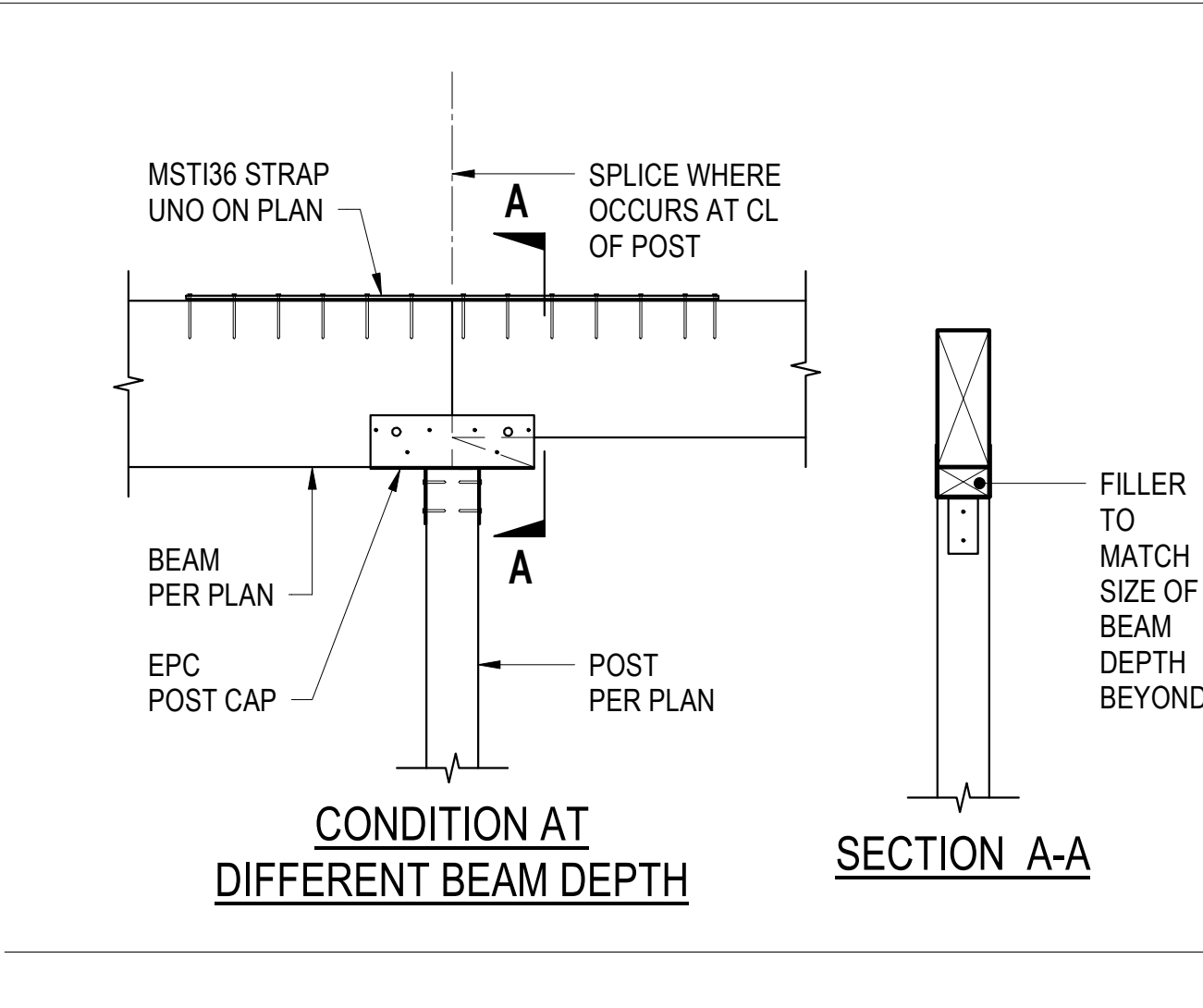
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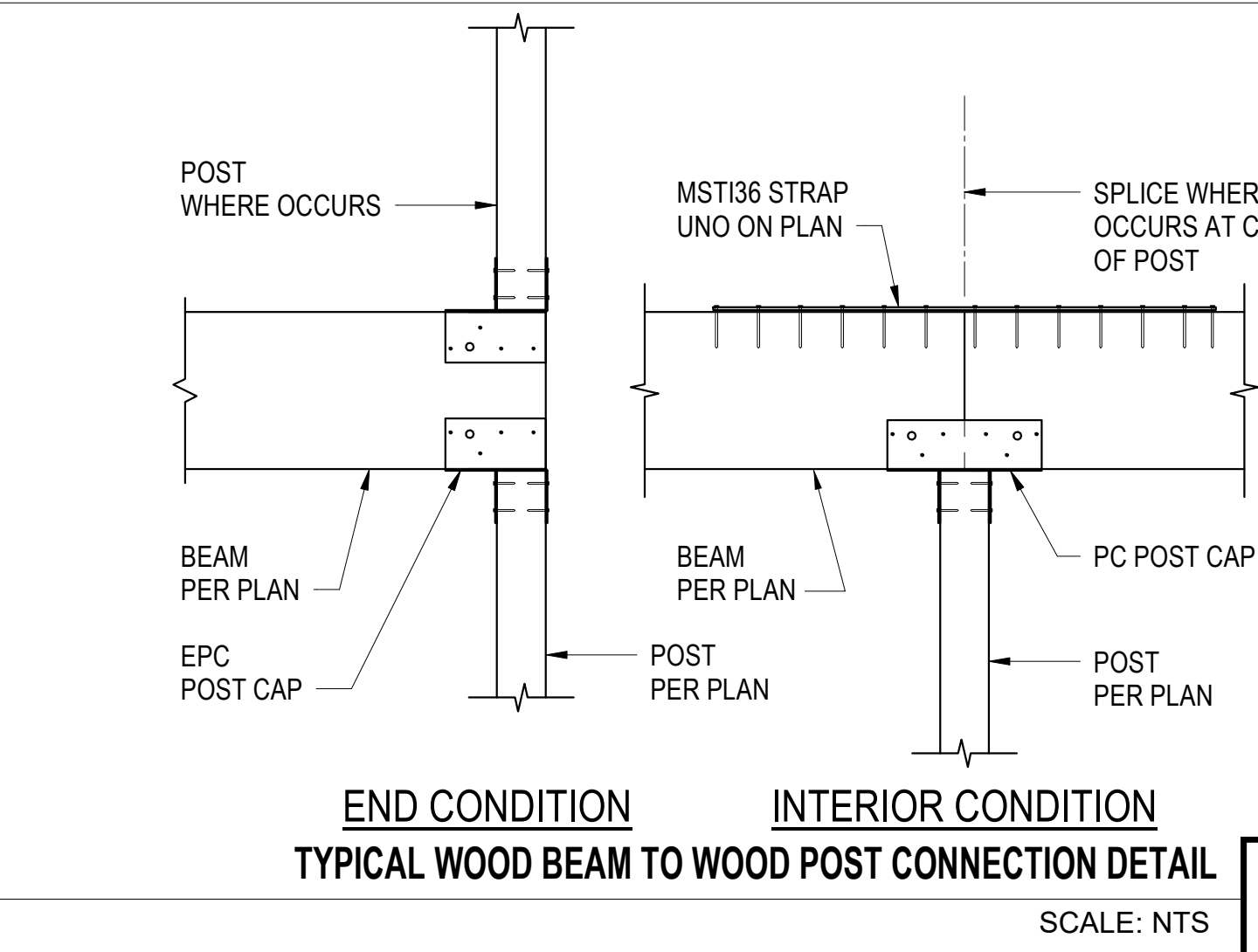
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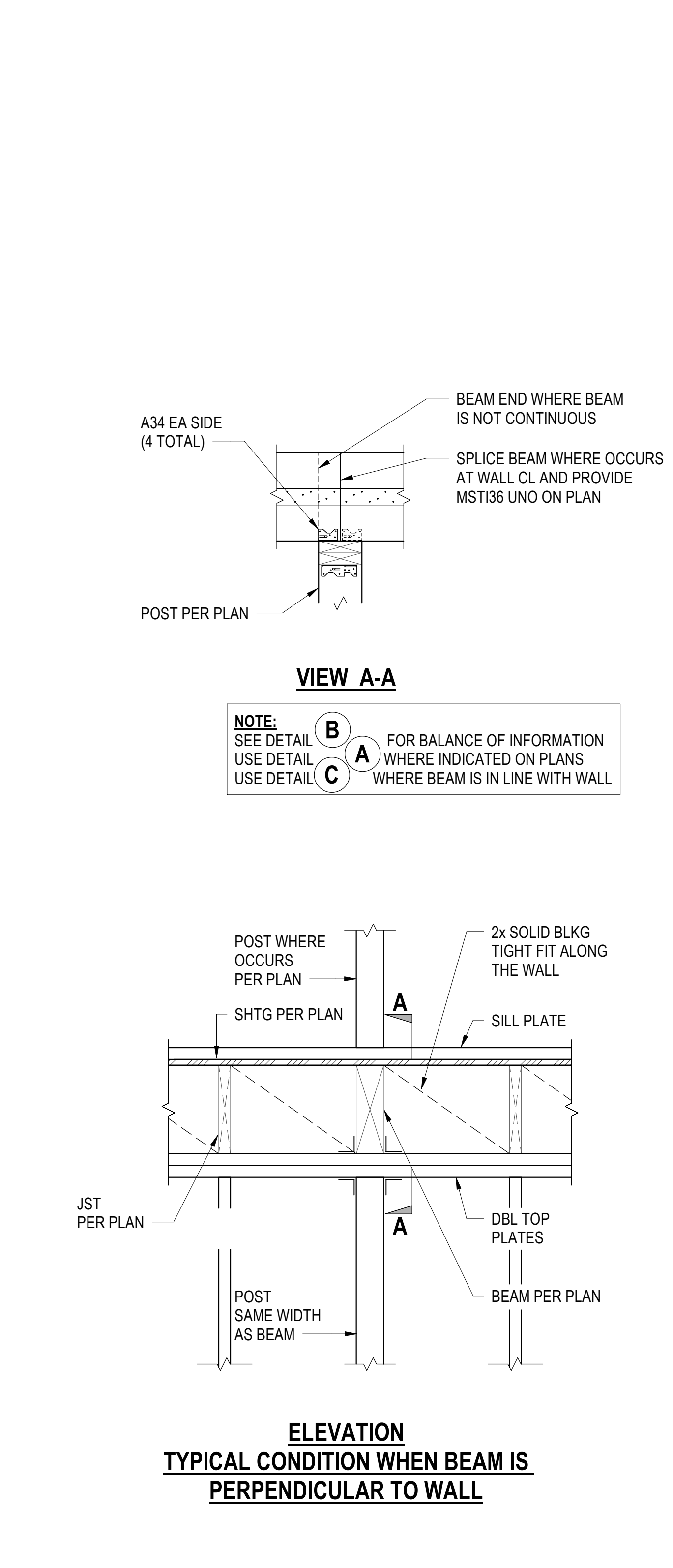
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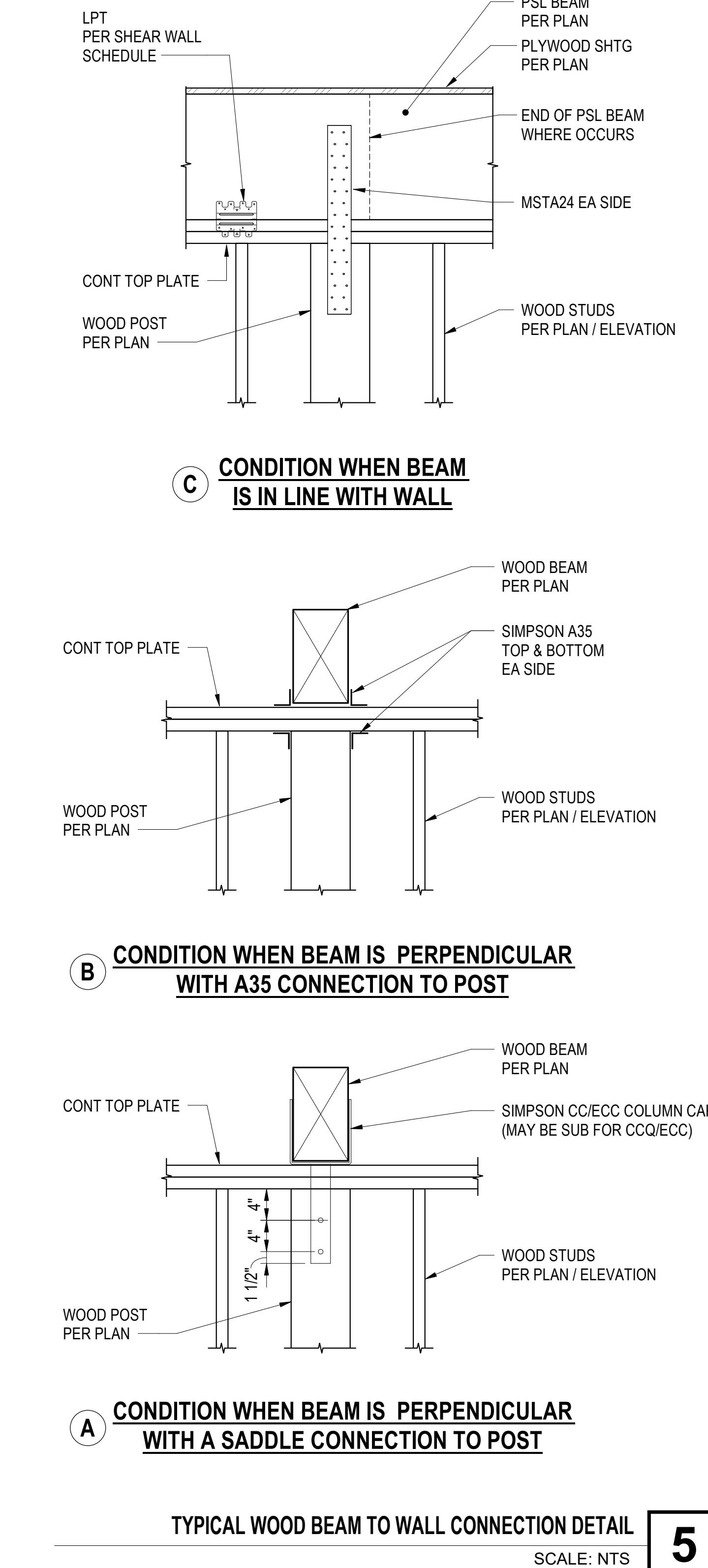
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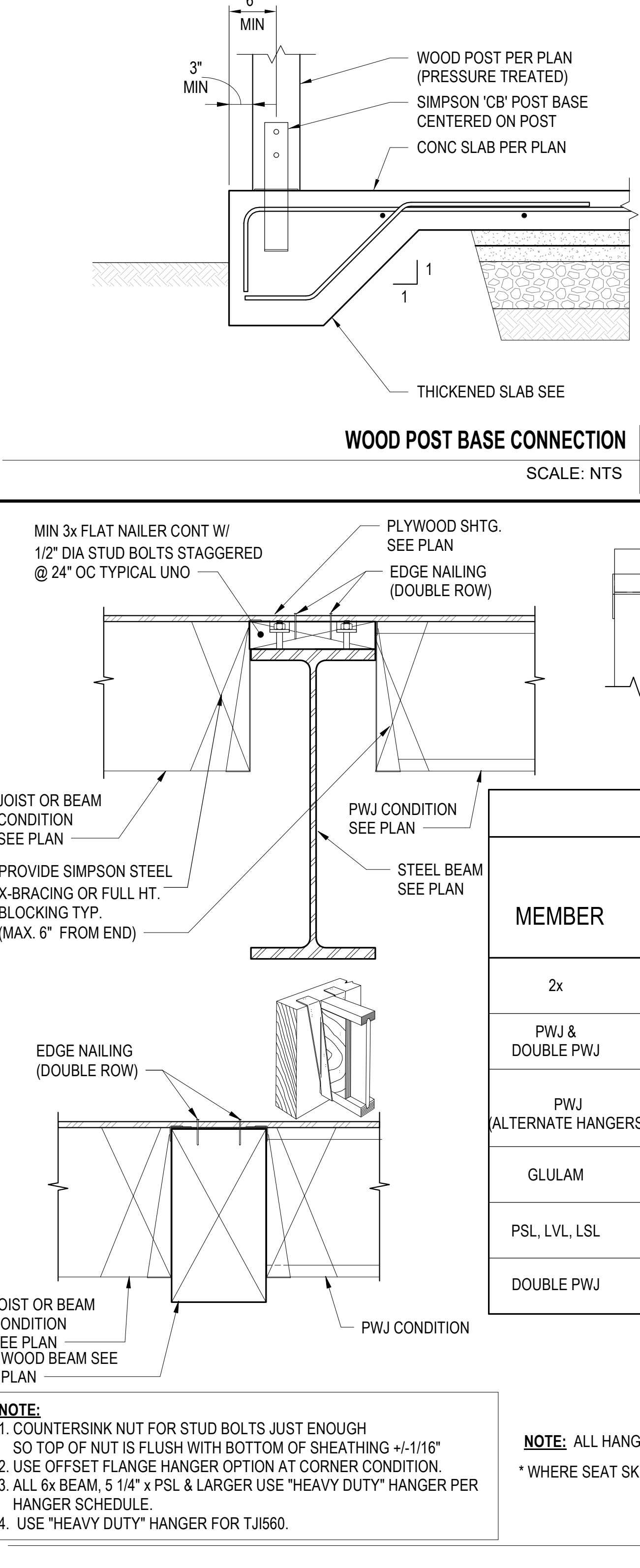
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4



1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232
TEL 310 392 3995

SO
saiful-bouquet
structural engineers

726 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

FIRE STATION 46

MISSION VILLAGE

COUNTY OF LOS ANGELES FIRE DEPARTMENT

VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER

NO. S3289

EXP. 12/31/27

STRUCTURAL

STATE OF CALIFORNIA

DATE

DRAWN

CHECKED

SCALE

JOB NO.

ISSUE DATE

AS NOTED

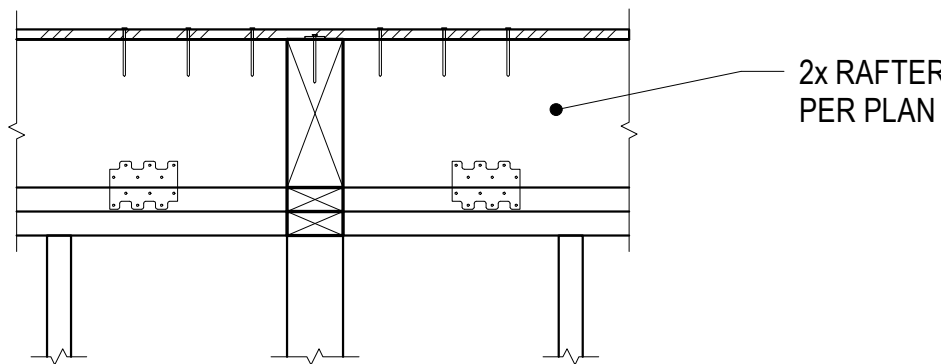
PROJECT NUMBER

APPENDIX M

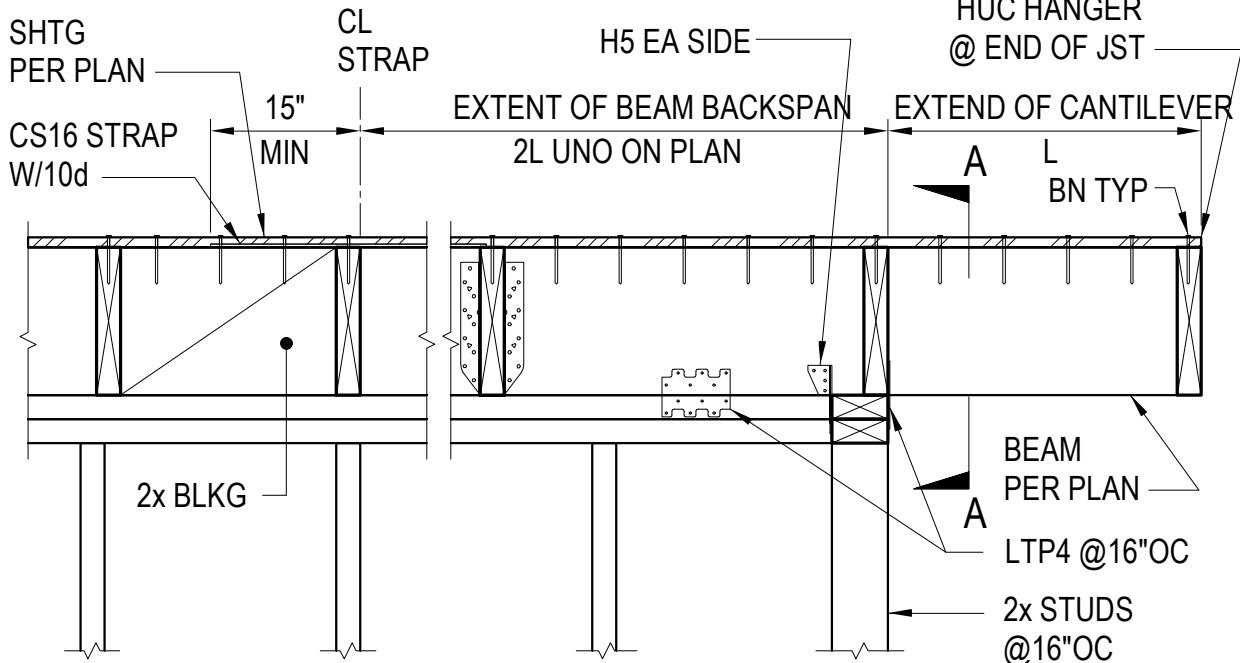
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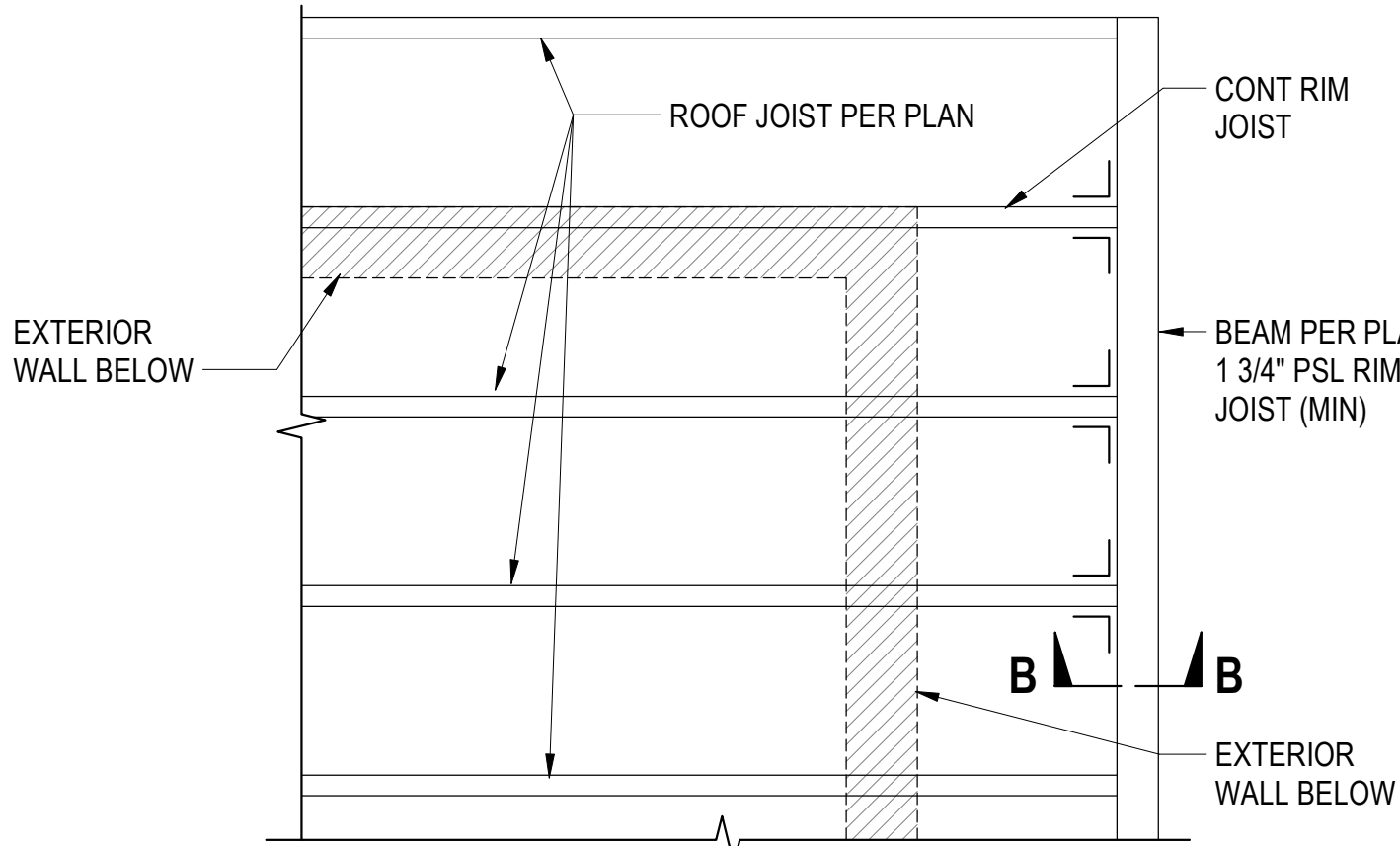


SECTION A-A



TYPICAL CANTILEVER BEAM ON TOP OF WALL
SCALE: NTS

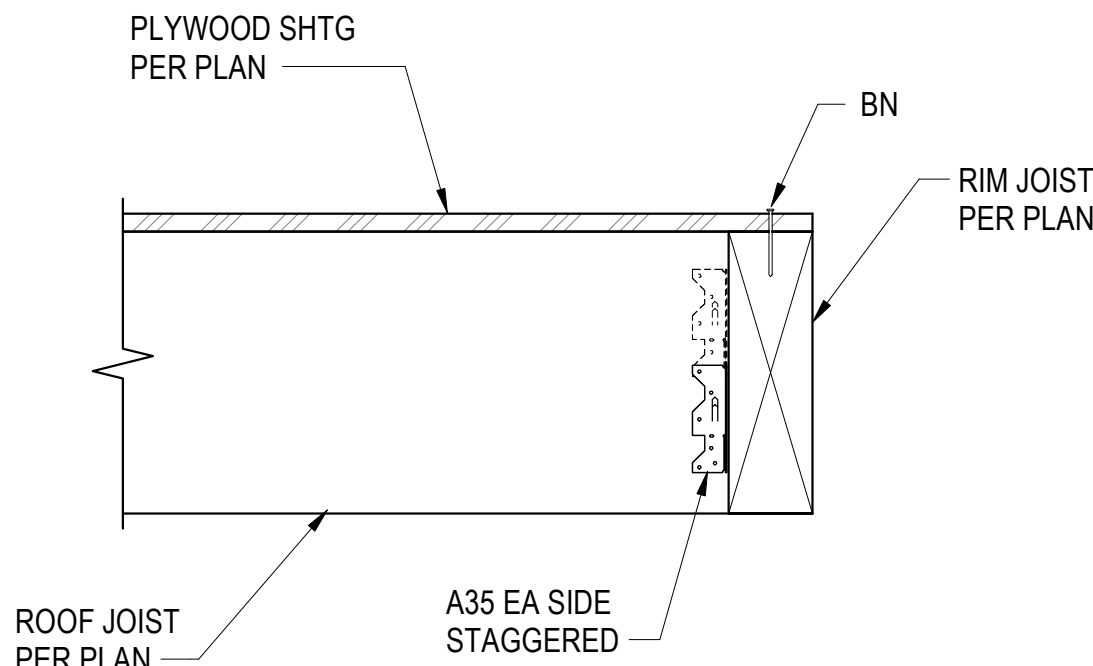
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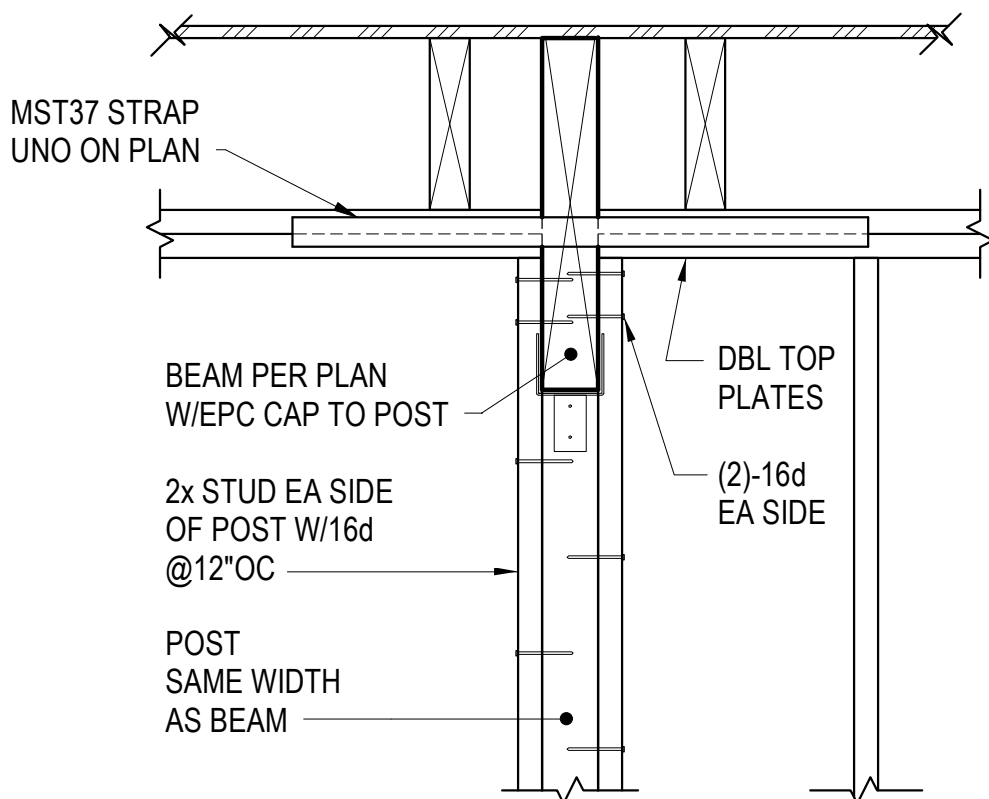
TYPICAL FRAMING AT ROOF EAVE CORNERS DETAIL

SCALE: NTS

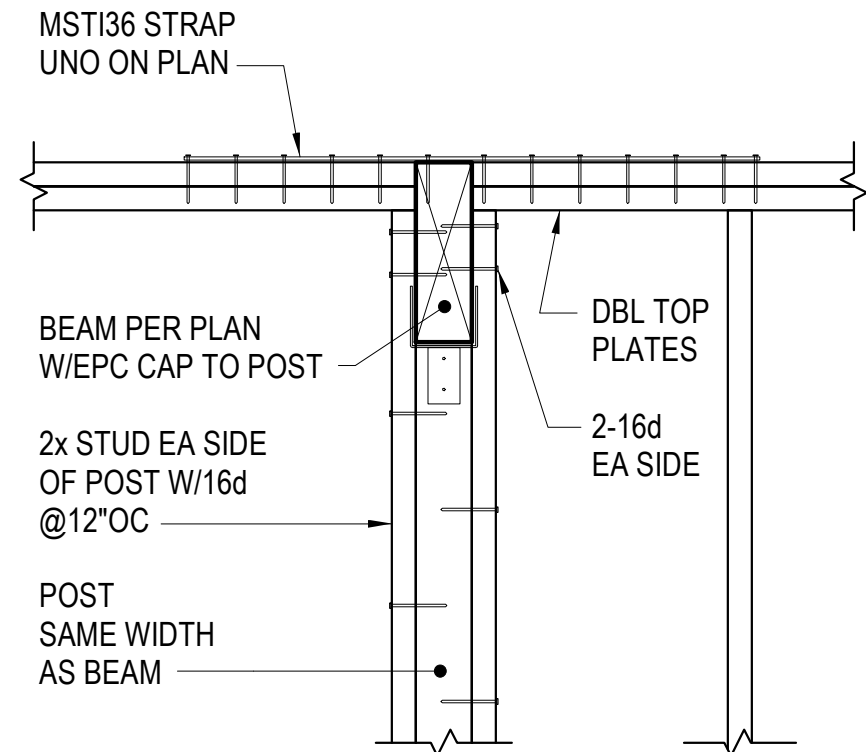
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SECTION B-B



CONDITION C

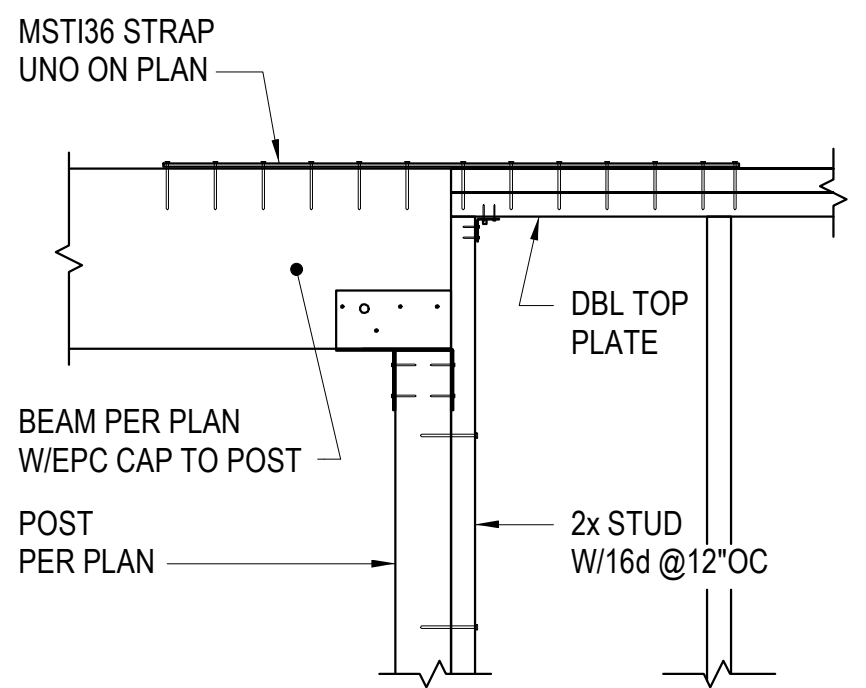


CONDITION A

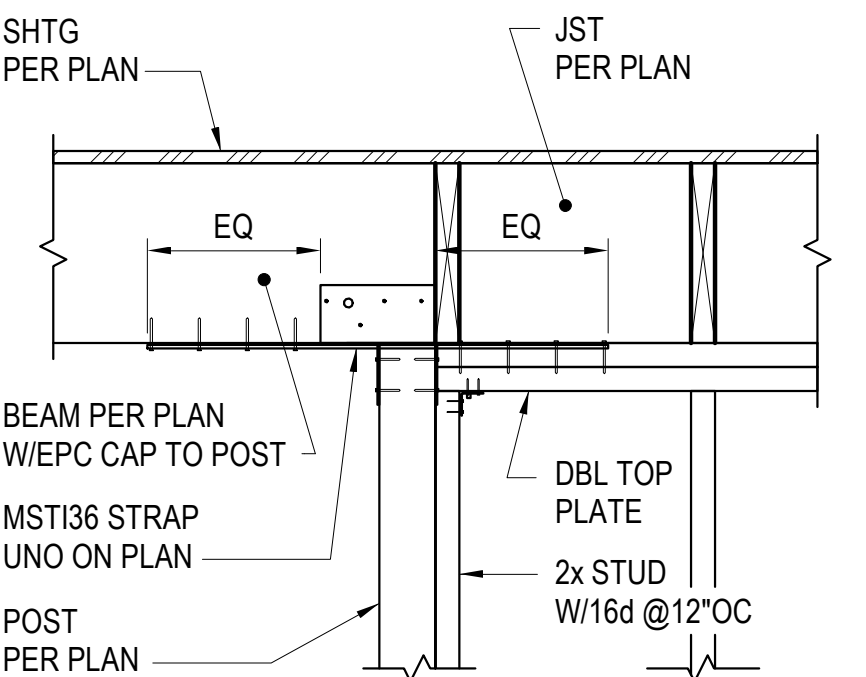
TYPICAL WOOD DROP BEAM PERPENDICULAR TO WALL

SCALE: NTS

2



DROP CONDITION



FLUSH CONDITION

TYPICAL WOOD BEAM PARALLEL TO WALL

SCALE: NTS

1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

SO
saiful-bouquet
structural engineers

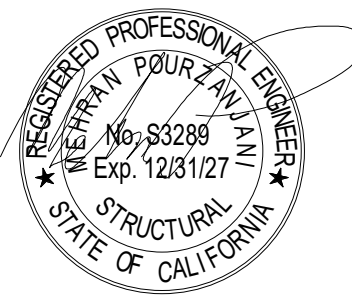
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(213) 315-2277
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TYPICAL WOOD DETAILS

FIRE STATION 46

MISSION VILLAGE

COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA



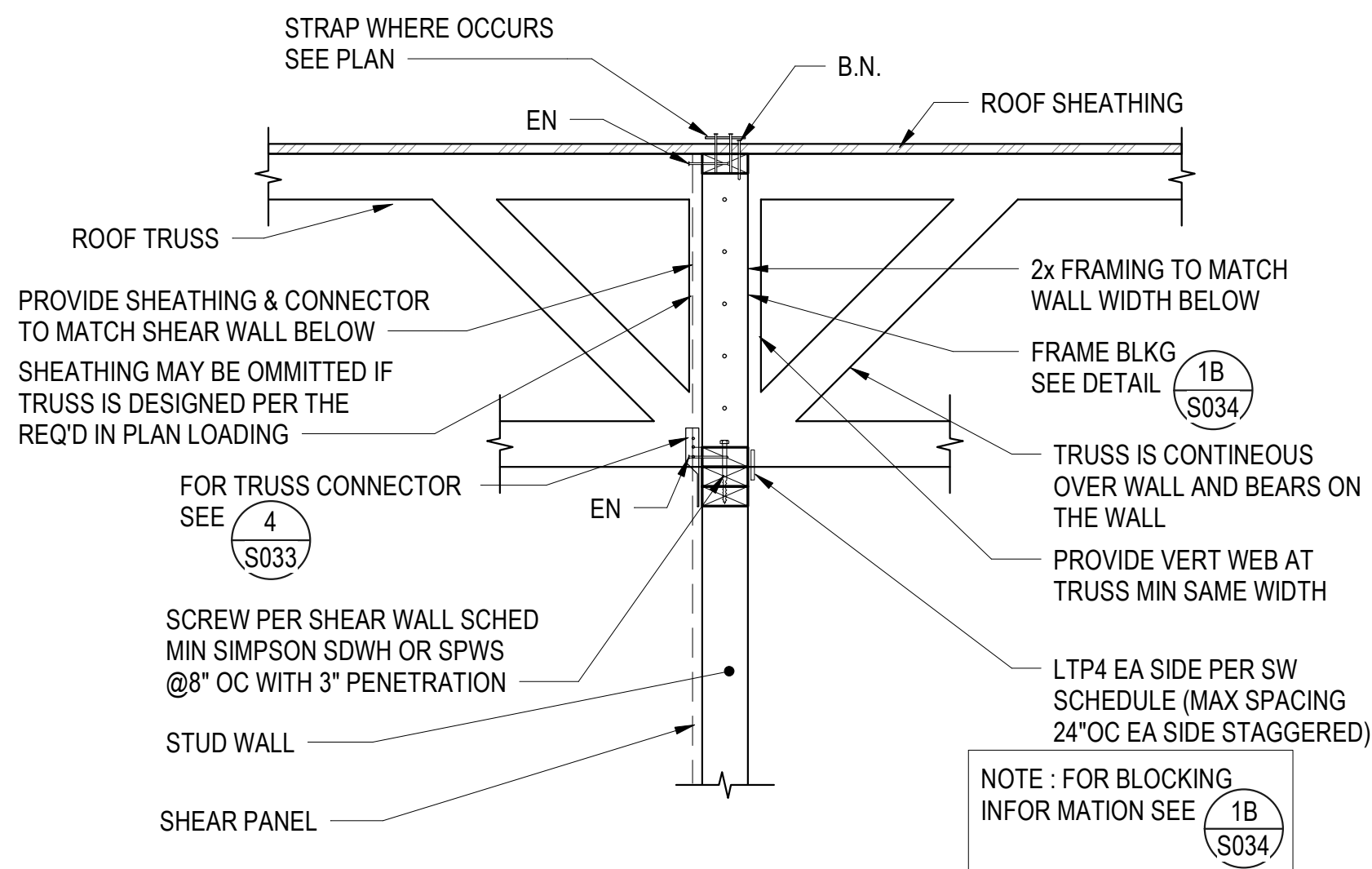
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CHECKED
SCALE
JOB NO.

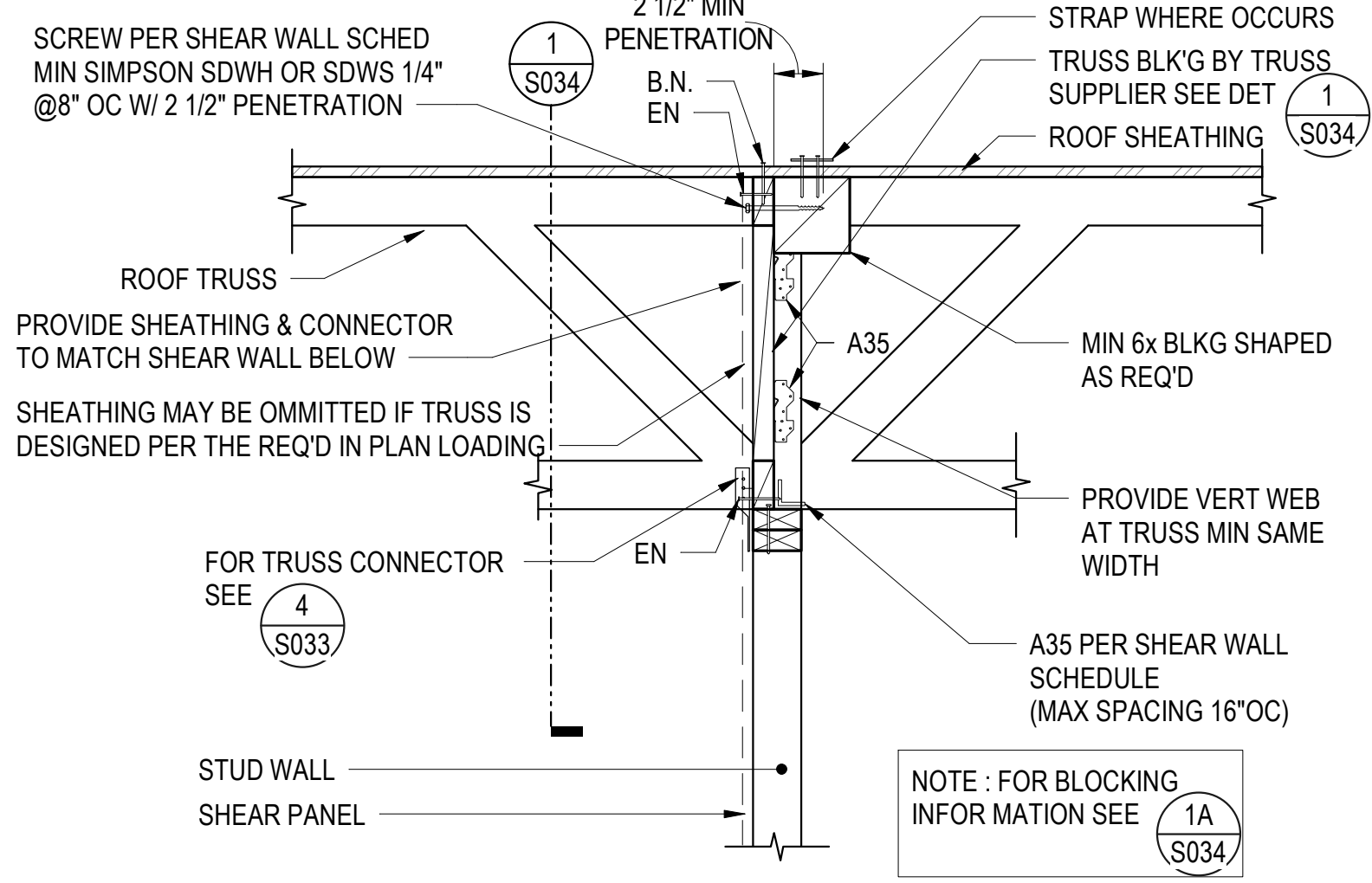
ISSUE DATE
AS NOTED
PROJECT NUMBER

S032B

APPENDIX 5



B OPTION WITH FRAMED BLOCKING

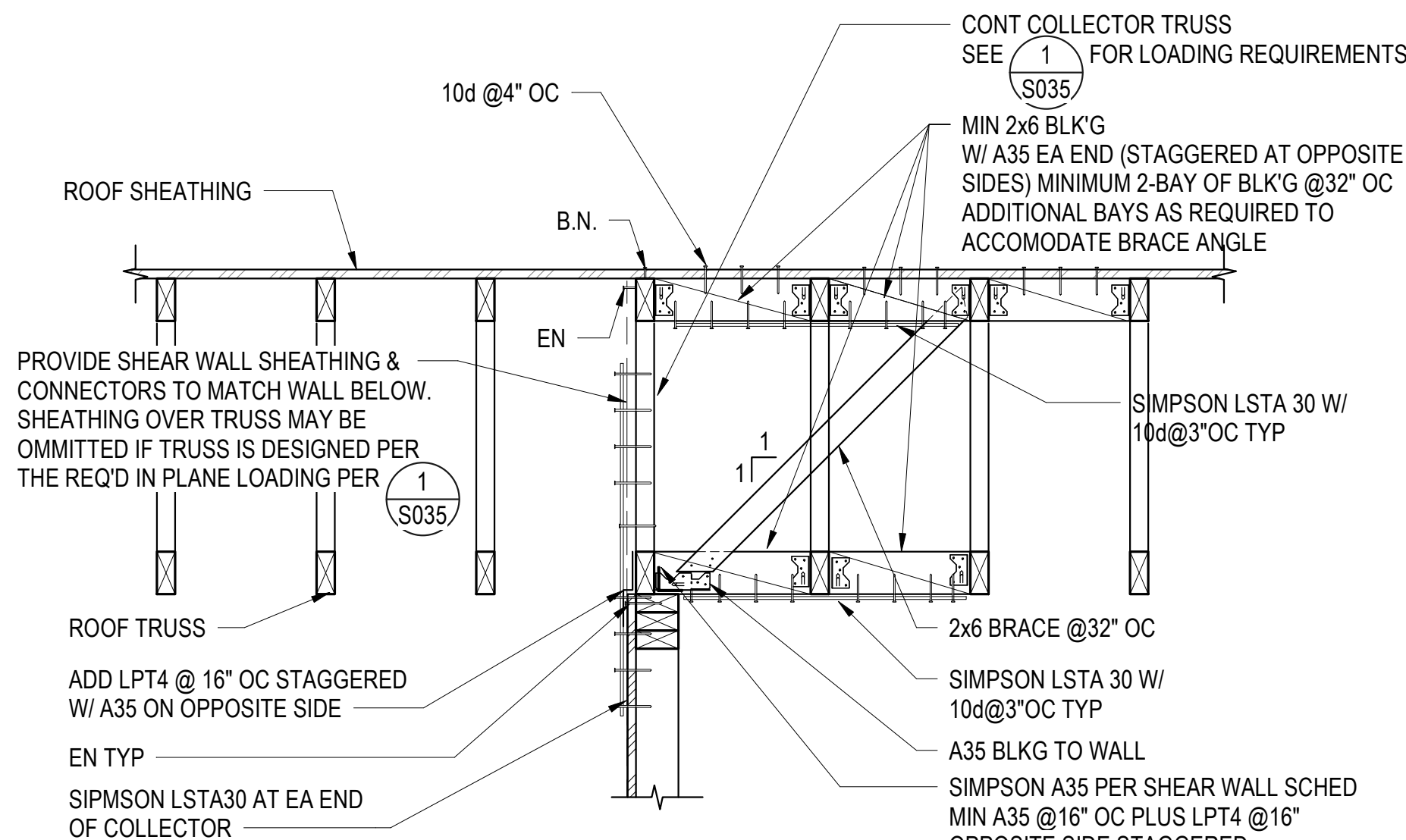


A OPTION WITH TRUSS BLOCKING

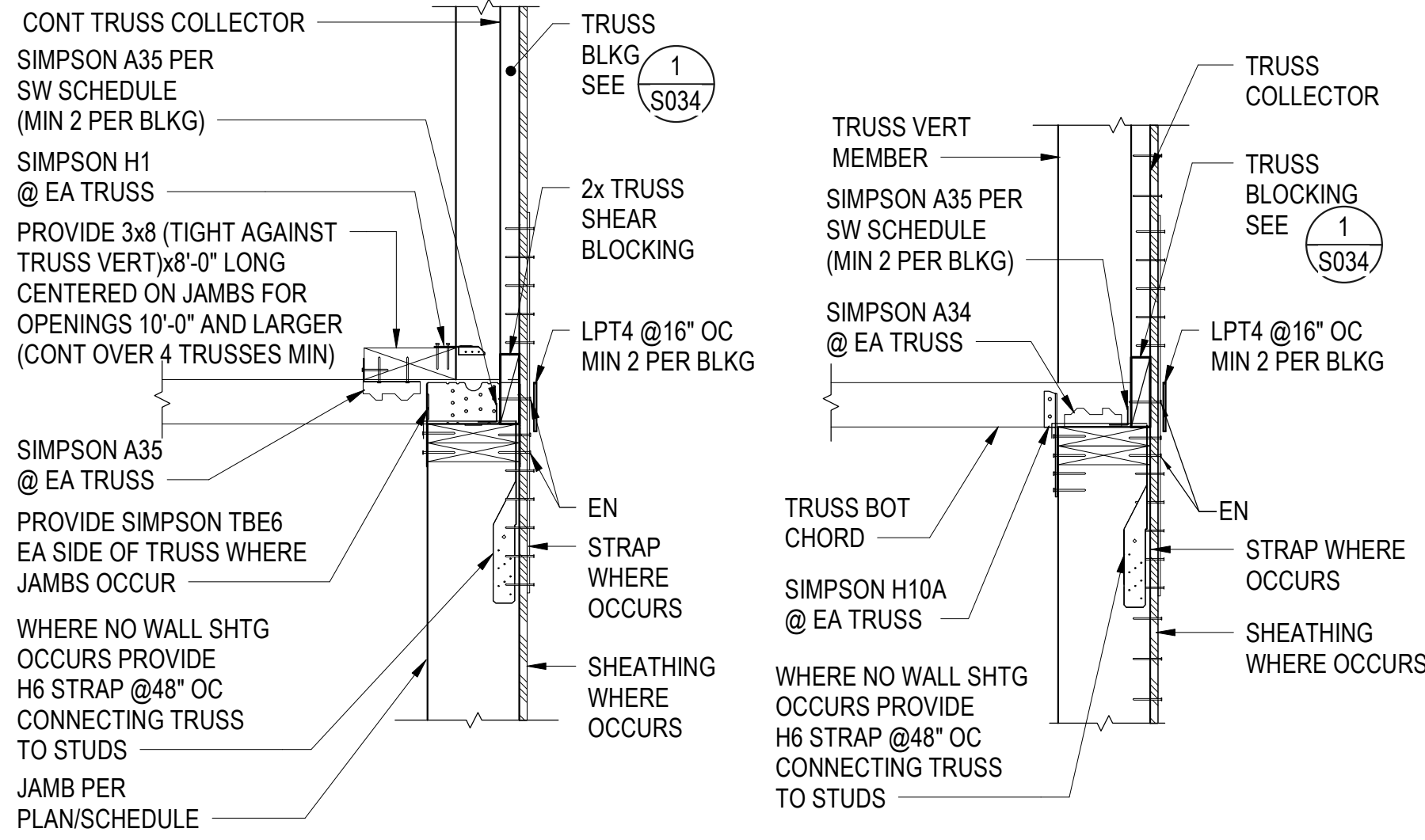
TYPICAL CONTINUOUS ROOF TRUSS TO STUD WALL CONNECTION

SCALE: NTS

5



B CONDITION AT ROOF TRUSS PARALLEL TO WALL



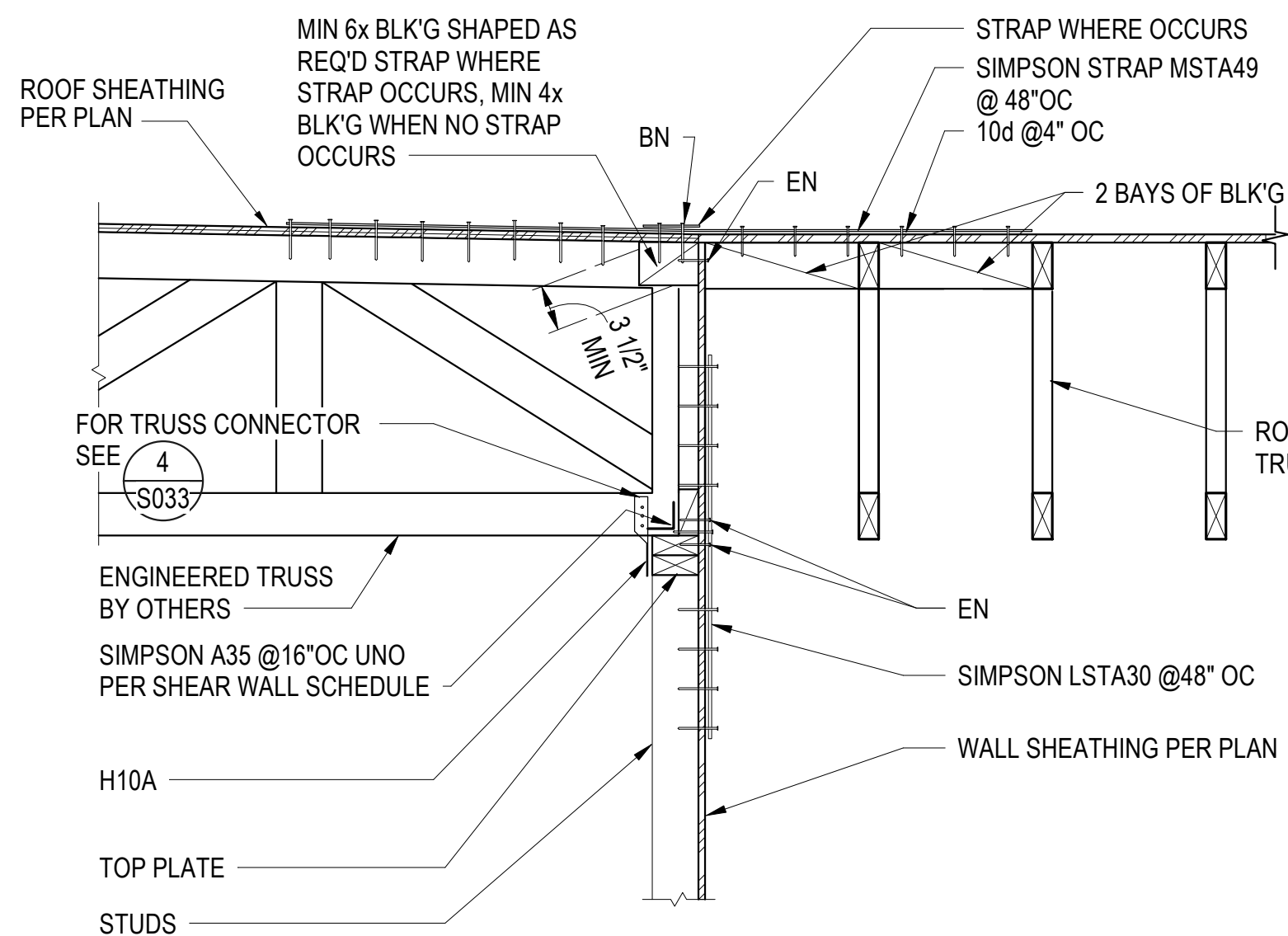
B CONDITION WHERE JAMBS FOR OPENINGS 10'-0" AND LARGER OCCUR

A TYPICAL CONDITION

TYPICAL ROOF TRUSS CONNECTION TO EXTERIOR WALL

SCALE: NTS

4

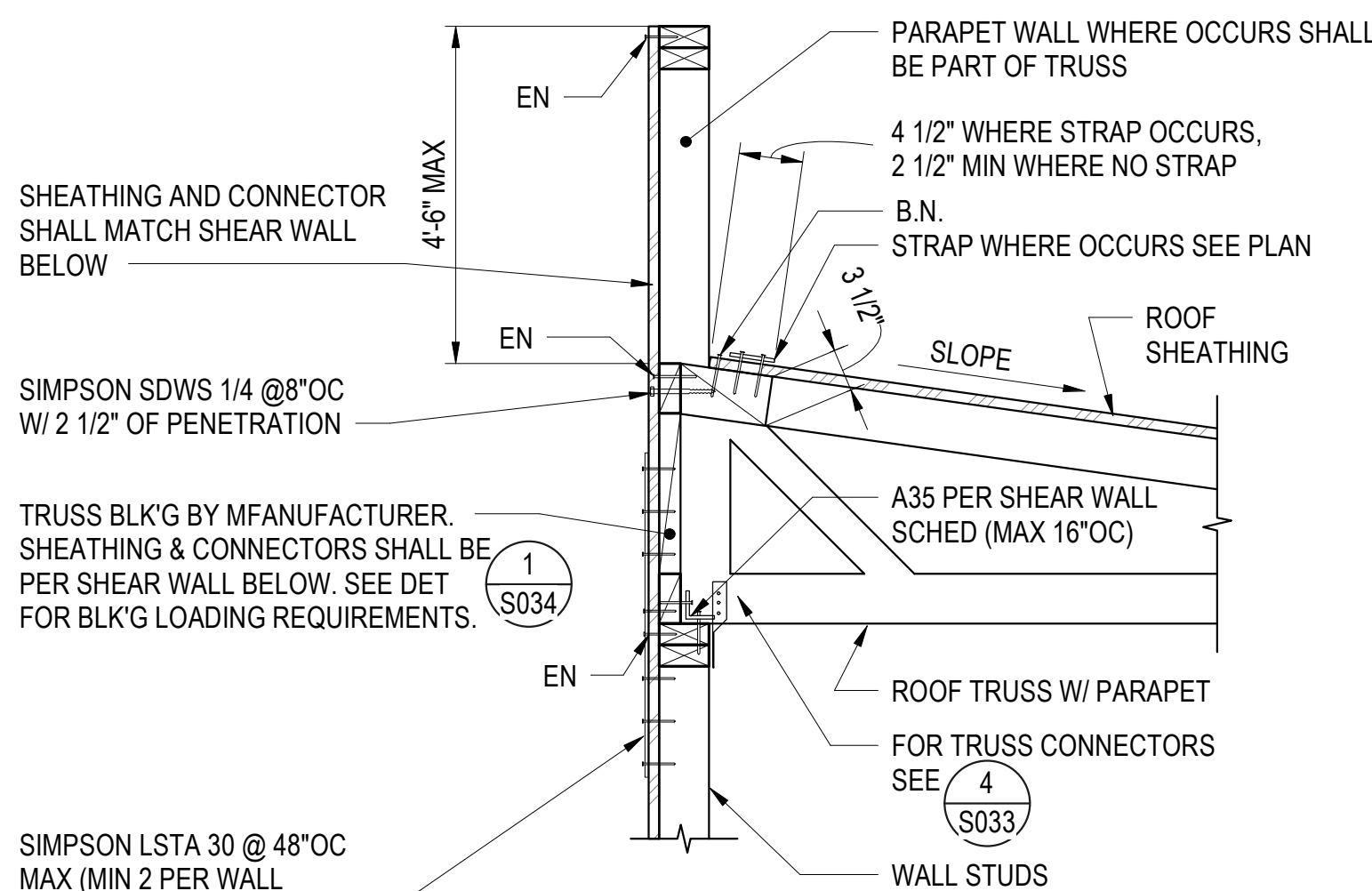


A CONDITION WHERE TRUSSES CHANGE DIRECTION

TYP ROOF TRUSS TO INTERIOR WALL CONNECTION

SCALE: NTS

6

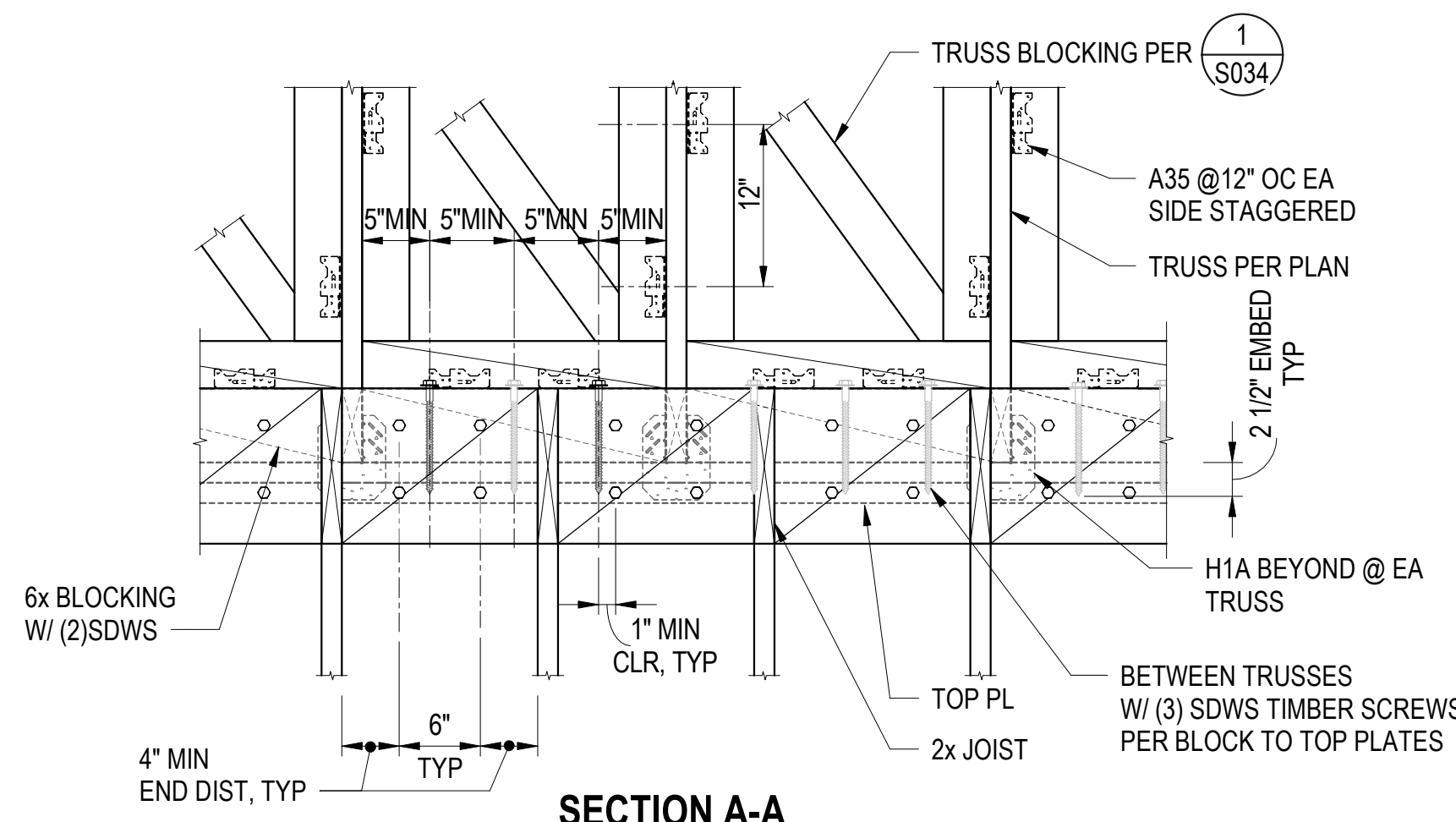


A TYPICAL CONDITION

TYPICAL ROOF TRUSS PERPENDICULAR TO NON-RATED EXTERIOR WALL (W/ PARAPET HEIGHT 4'-6" MAX)

SCALE: NTS

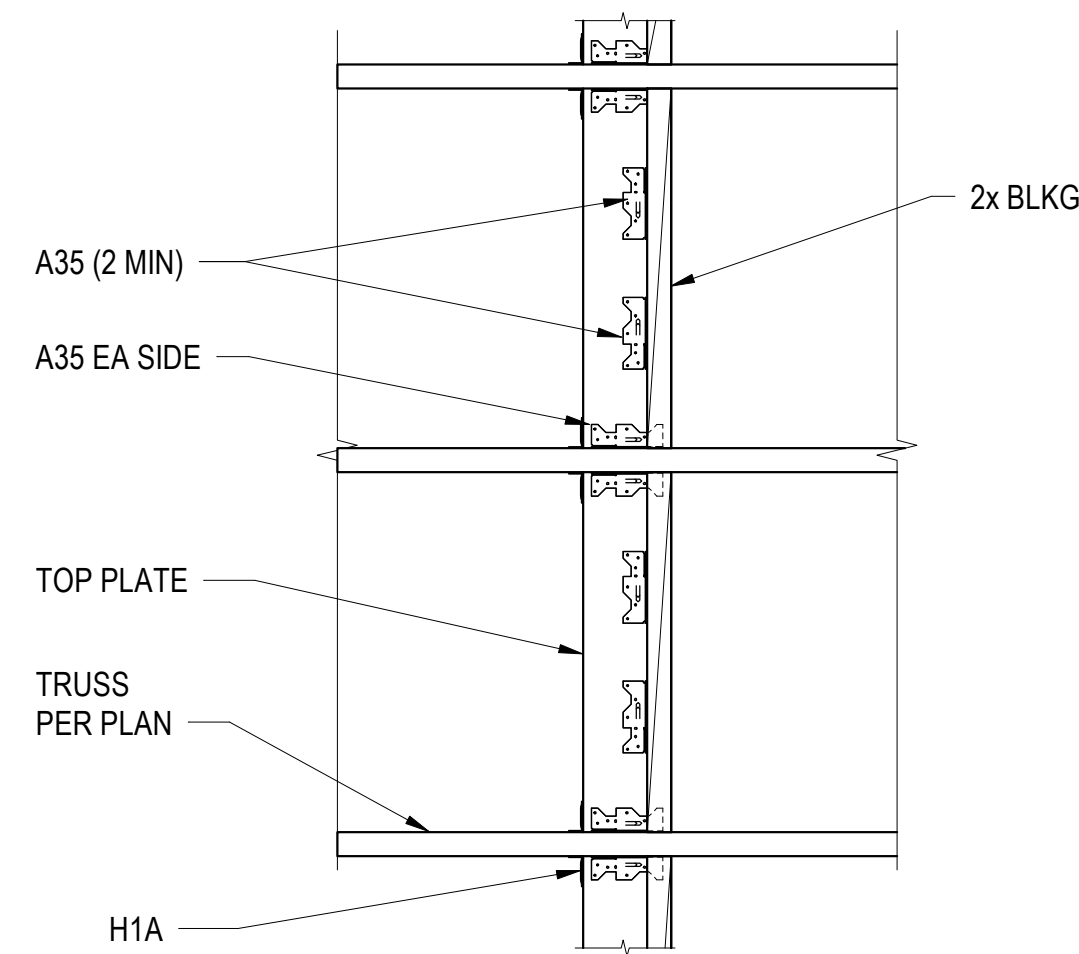
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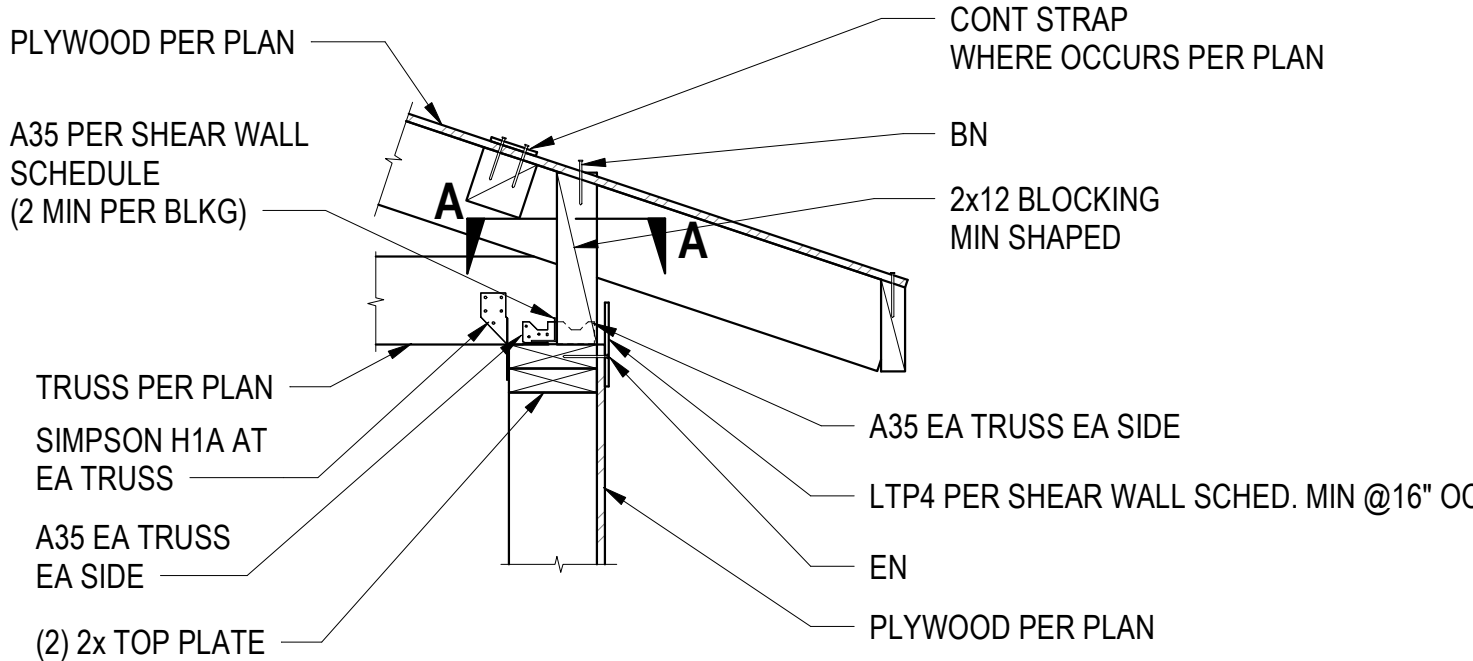
TYPICAL TRUSS CONNECTION AT INTERIOR WALL WHERE A DEEPER JOIST OCCURS

SCALE: NTS

2



PLAN VIEW A-A



TYPICAL TRUSS CONNECTION AT EXTERIOR WALL WITH AN EAVE

SCALE: NTS

1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

SO
structural engineers

725 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.sofibouquet.com
(213) 315-2277
Project #25534

TYPICAL WOOD DETAILS

FIRE STATION 46
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COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

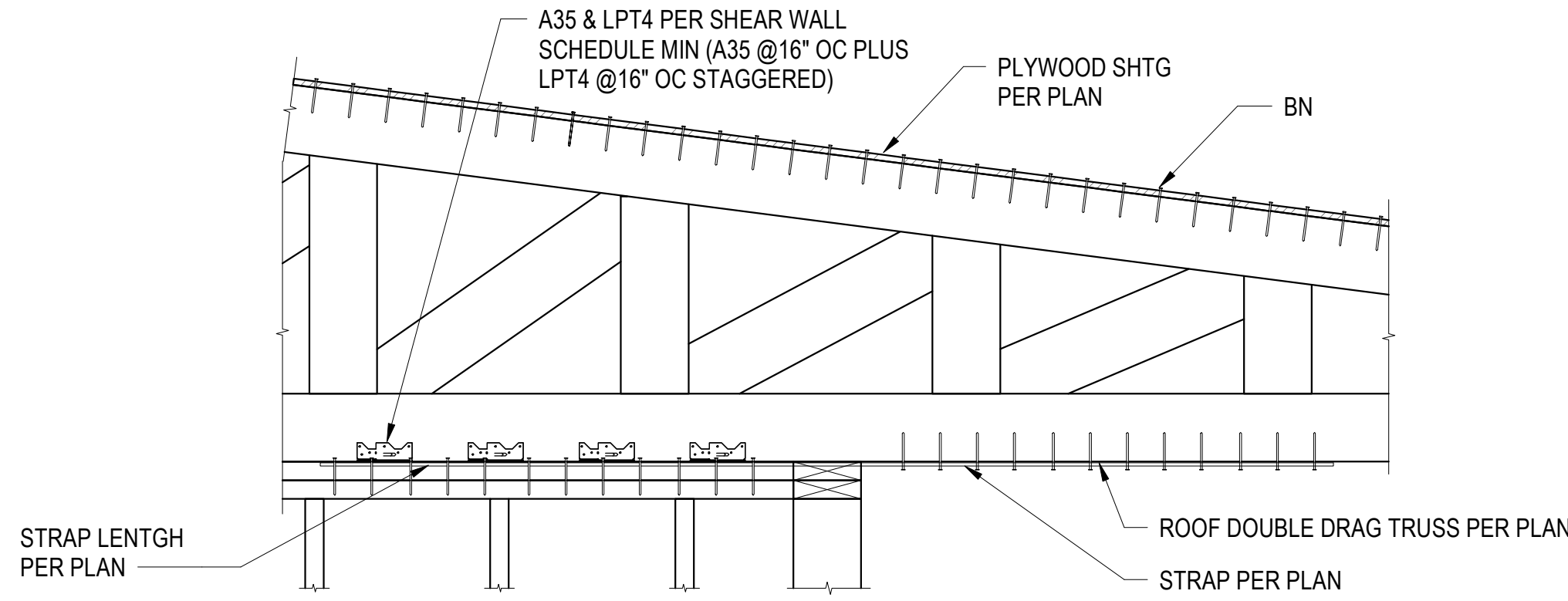


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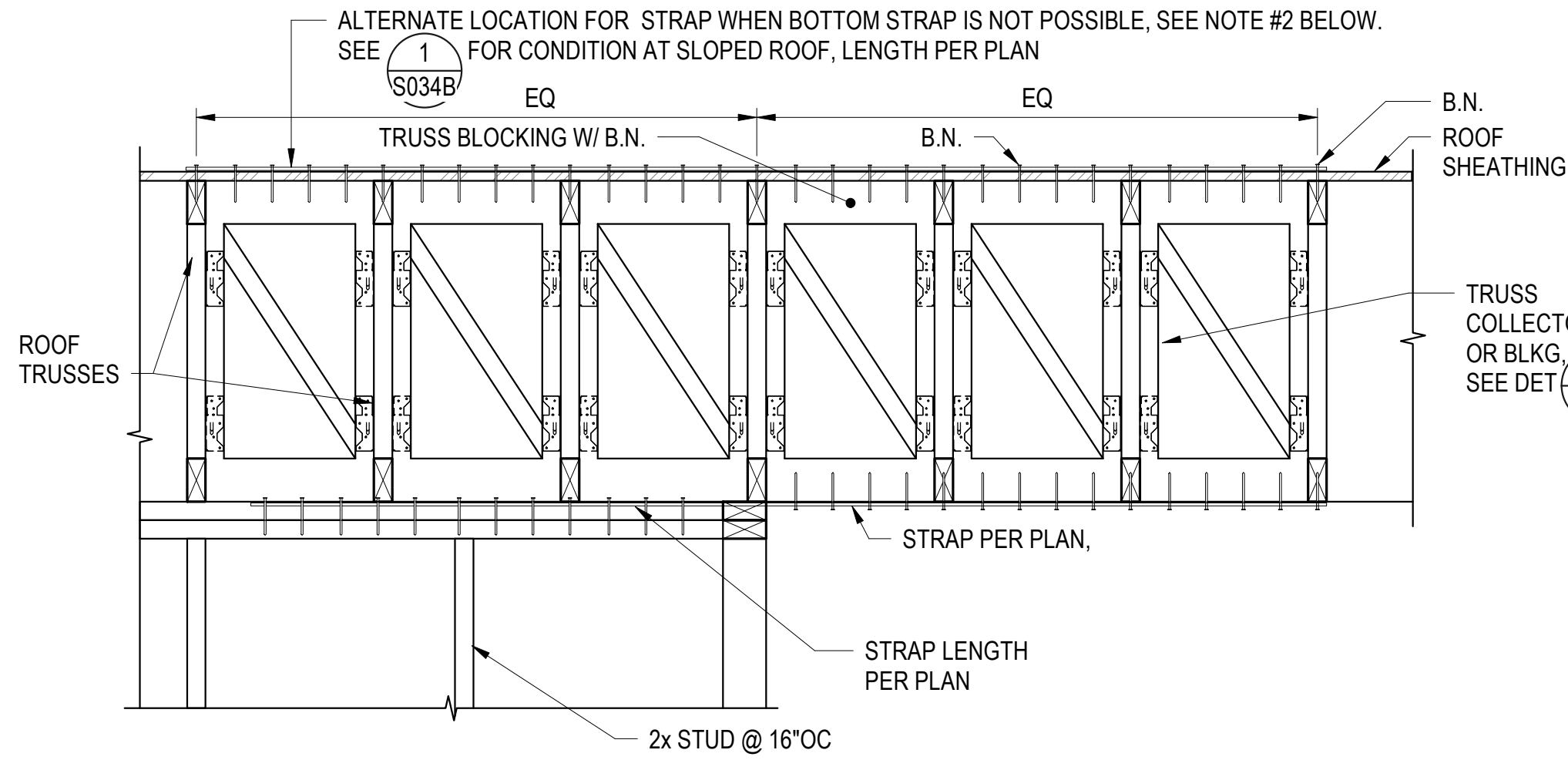
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PROJECT NUMBER: [Number]

ADDENDUM 5
S033

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CONDITION WHERE TRUSSES ARE PARALLEL TO THE SHEAR WALL



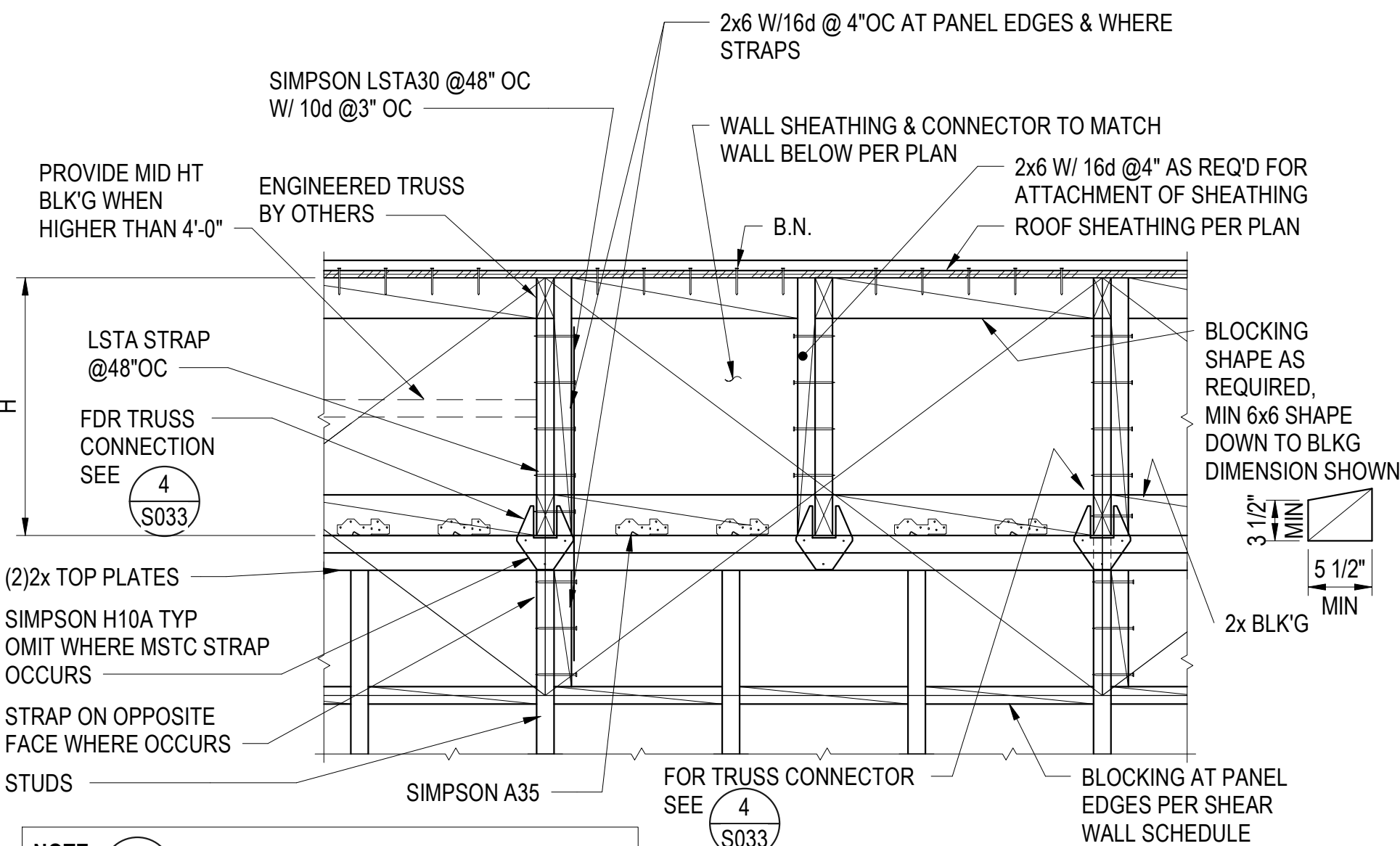
CONDITION WHERE TRUSSES ARE PERPENDICULAR TO SHEAR WALL

- NOTES:**
- FOR TRUSS BLOCKING SEE **1** S034 FOR BALANCE OF INFORMATION
 - PROVIDE MINIMUM (2) SISTERED TRUSSES OR TRUSS BLOCKING AT STRAPS TO ACCOMMODATE STRAPS NAILING. PROVIDE (2) ROWS OF TRUSSES OR TRUSS BLOCKING SPACED APART WHERE (2) CS STRAPS ARE INDICATED ON PLANS TO ACCOMMODATE SHEAR WALL HOLD DOWN TIE RODS AND PLATE ASSEMBLY WHERE OCCURS. PLACE STRAPS ON ROOF SHEATHING WHEN BOTTOM STRAP PLACEMENT IS OBSTRUCTED OR NOT POSSIBLE (NOTIFY EOR).
 - SEE DET **1** S037 FOR STRAP LENGTH AND NAILING.
 - SEE PLANS AND DETAIL **1** S035 AND **1** S034 FOR REQUIRED INPLANE TRUSS LOAD

TYPICAL TRUSS DRAG / STRUT STRAP DETAIL IN LINE WITH SHEAR WALL

SCALE: NTS

3



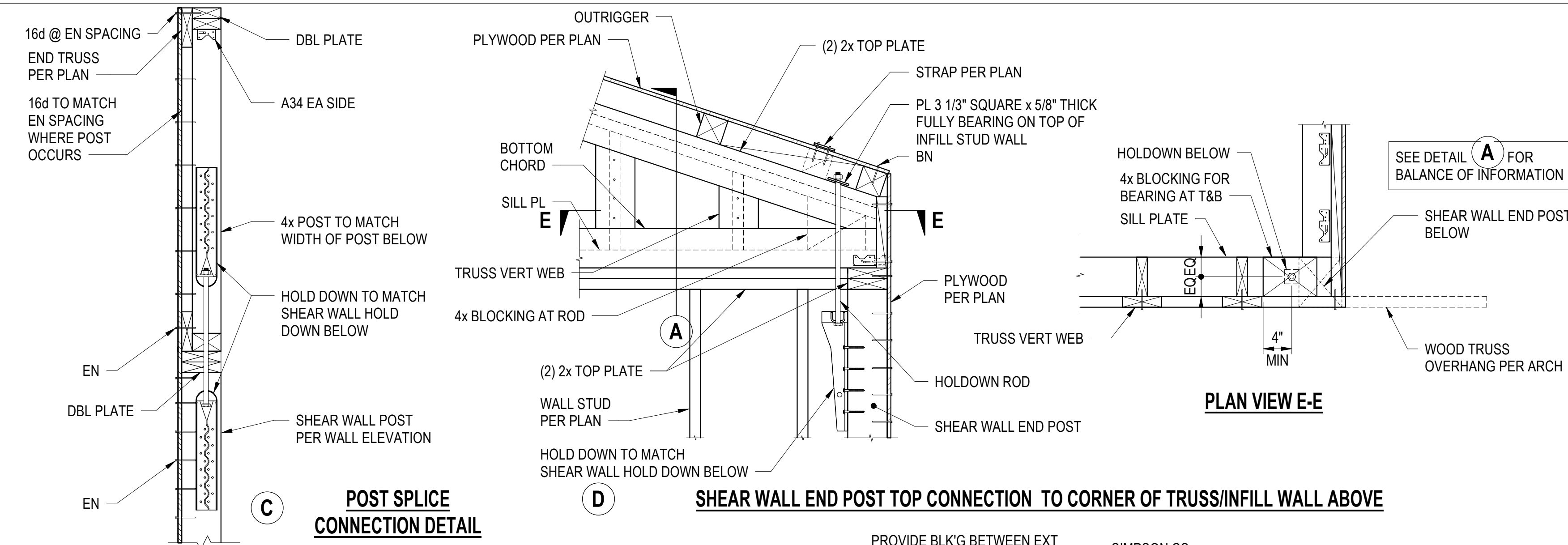
BLOCKING CONDITION WHERE PERPENDICULAR TRUSS OCCURS ON ONE SIDE OF WALL & TERMINATES ON WALL

- NOTE:**
- OPTION **A** IS ONLY ALLOWED WHEN H IS 3'-6" & LESS IN HEIGHT WHERE H EXCEEDS 3'-6" USE CONDITION **B** OR **C**
 - WHERE TRUSS IS PERPENDICULAR TO WALL FROM ONE SIDE ONLY & TERMINATES ON THE WALL USE CONDITION **C** UNO.
 - THE BLOCKING OPTIONS SHALL NOT BE MIXED OVER A GIVEN LINE OF SHEAR WALLS. ONLY THE SAME TYPE OF BLOCKING, SHALL BE USED ALONG ANY GIVEN LINES OF SHEAR WALL.
 - SEE **1** S035 FOR ADDITIONAL INFO.
 - FOR EXTERIOR WALL CONDITION AT TRUSS SEE **2** S034

TYPICAL ROOF TRUSS BLOCKING WHERE TRUSS IS PERPENDICULAR TO WALL OR IS CONTINUOUS OVER WALL

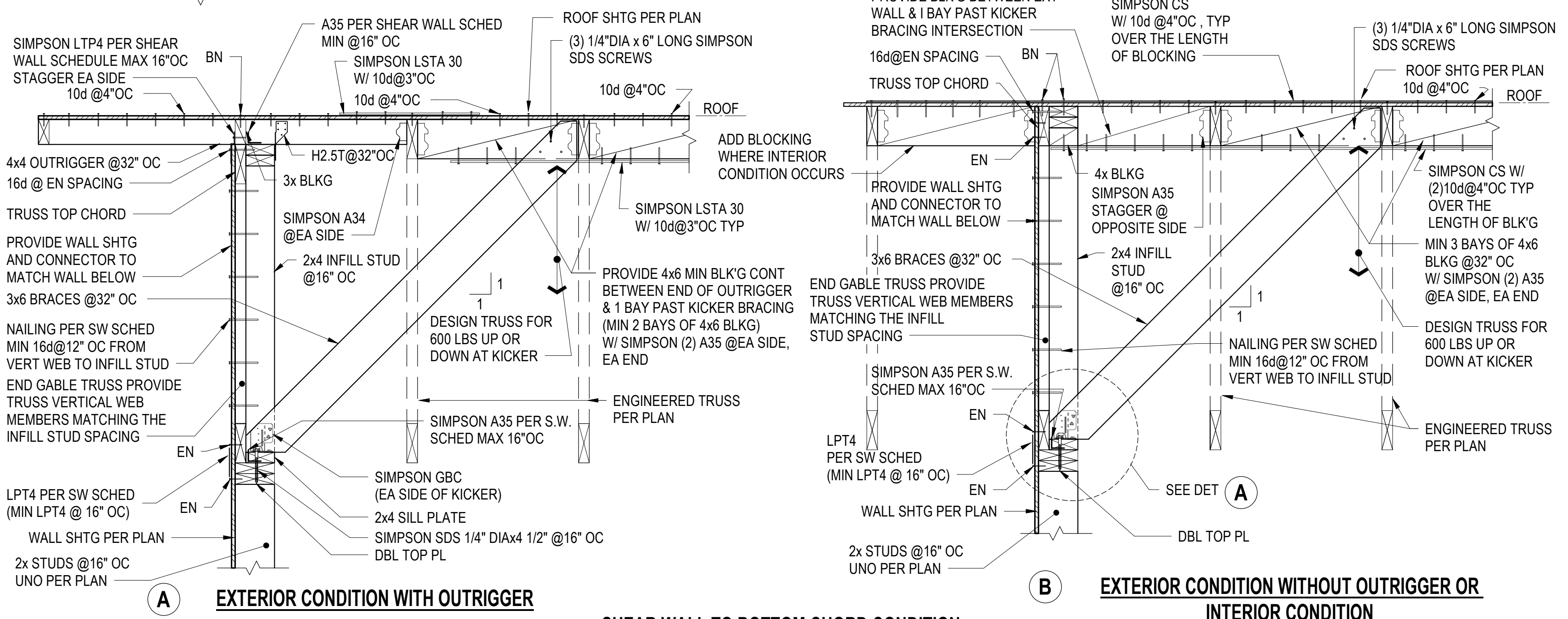
SCALE: NTS

1



POST SPLICE CONNECTION DETAIL

SHEAR WALL END POST TOP CONNECTION TO CORNER OF TRUSS/INFILL WALL ABOVE



EXTERIOR CONDITION WITH OUTRIGGER

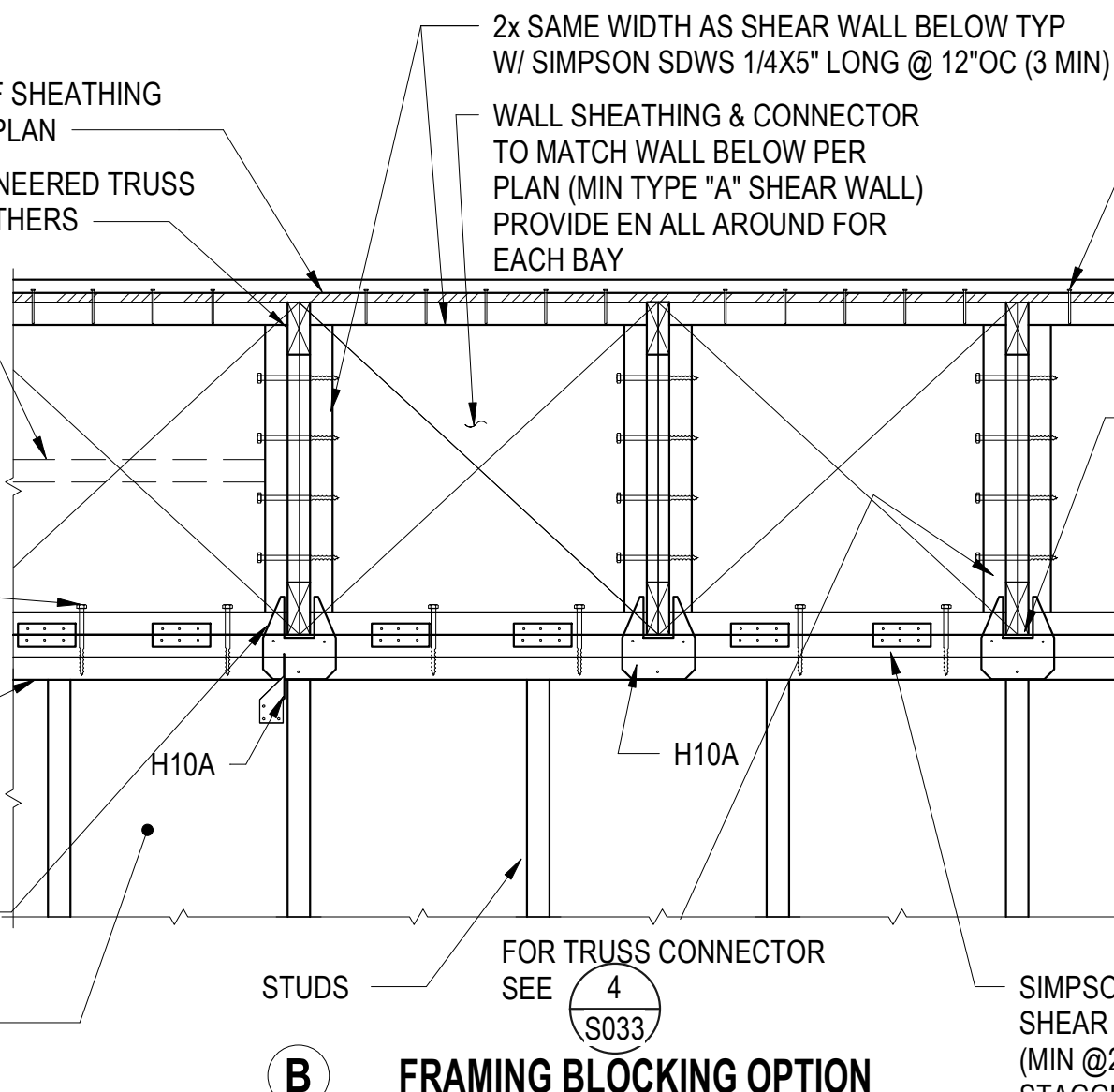
SHEAR WALL TO BOTTOM CHORD CONDITION

EXTERIOR CONDITION WITHOUT OUTRIGGER OR INTERIOR CONDITION

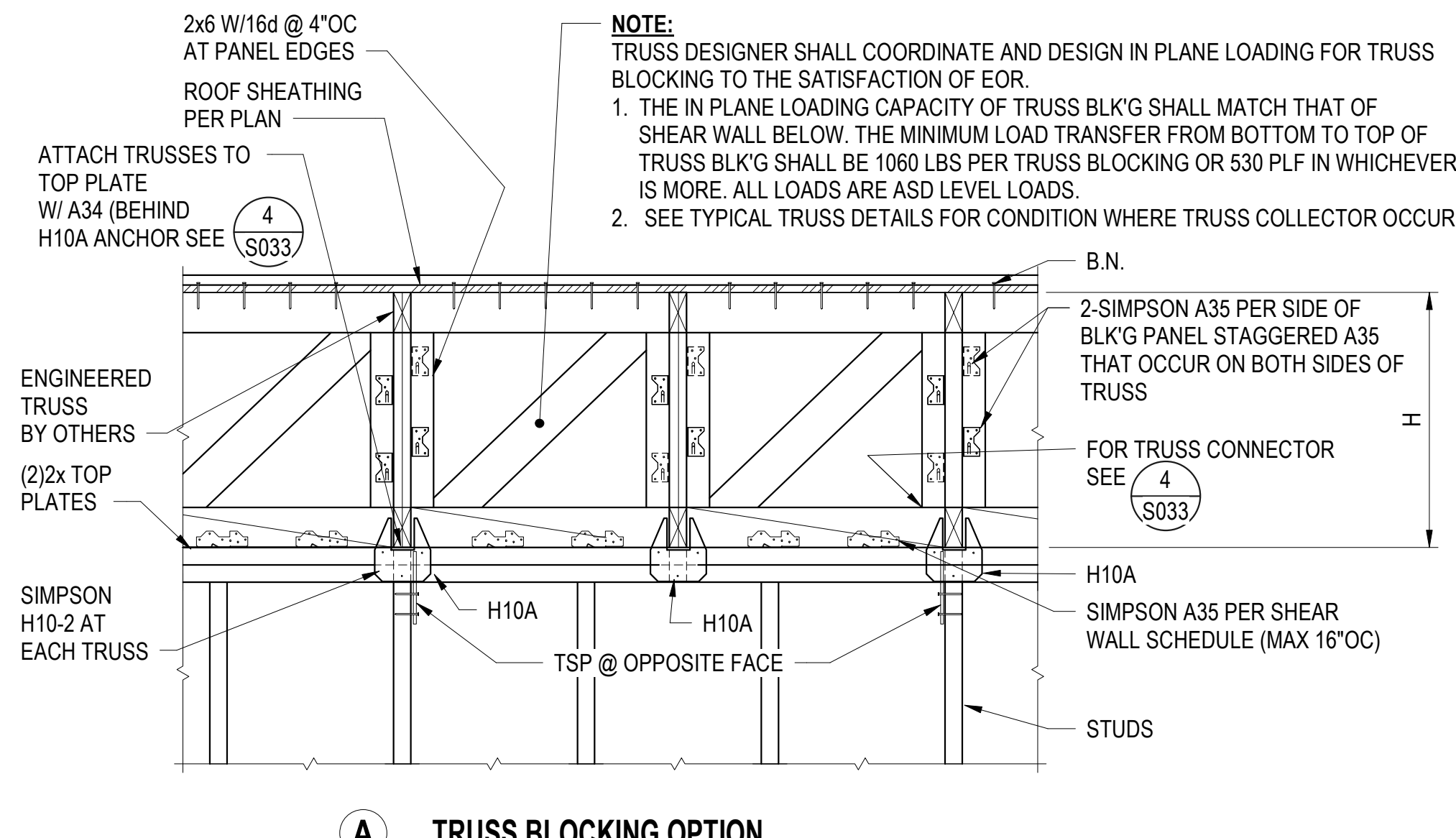
TYPICAL DETAIL AT TOP OF SHEAR WALL PARALLEL TO TRUSS

SCALE: NTS

2



FRAMING BLOCKING OPTION



TRUSS BLOCKING OPTION

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

so
saiful-bouquet
structural engineers

726 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
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TYPICAL WOOD DETAILS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

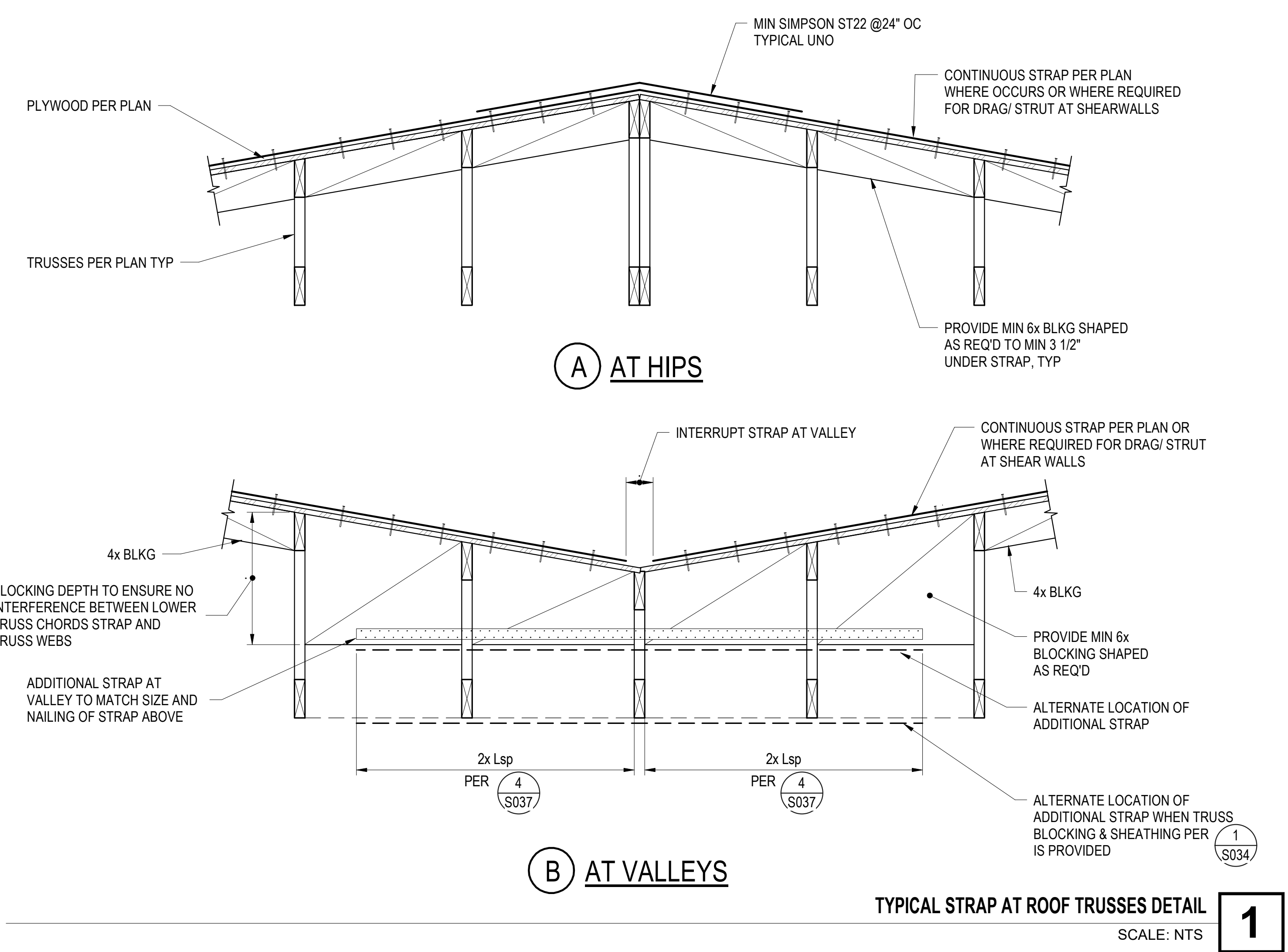
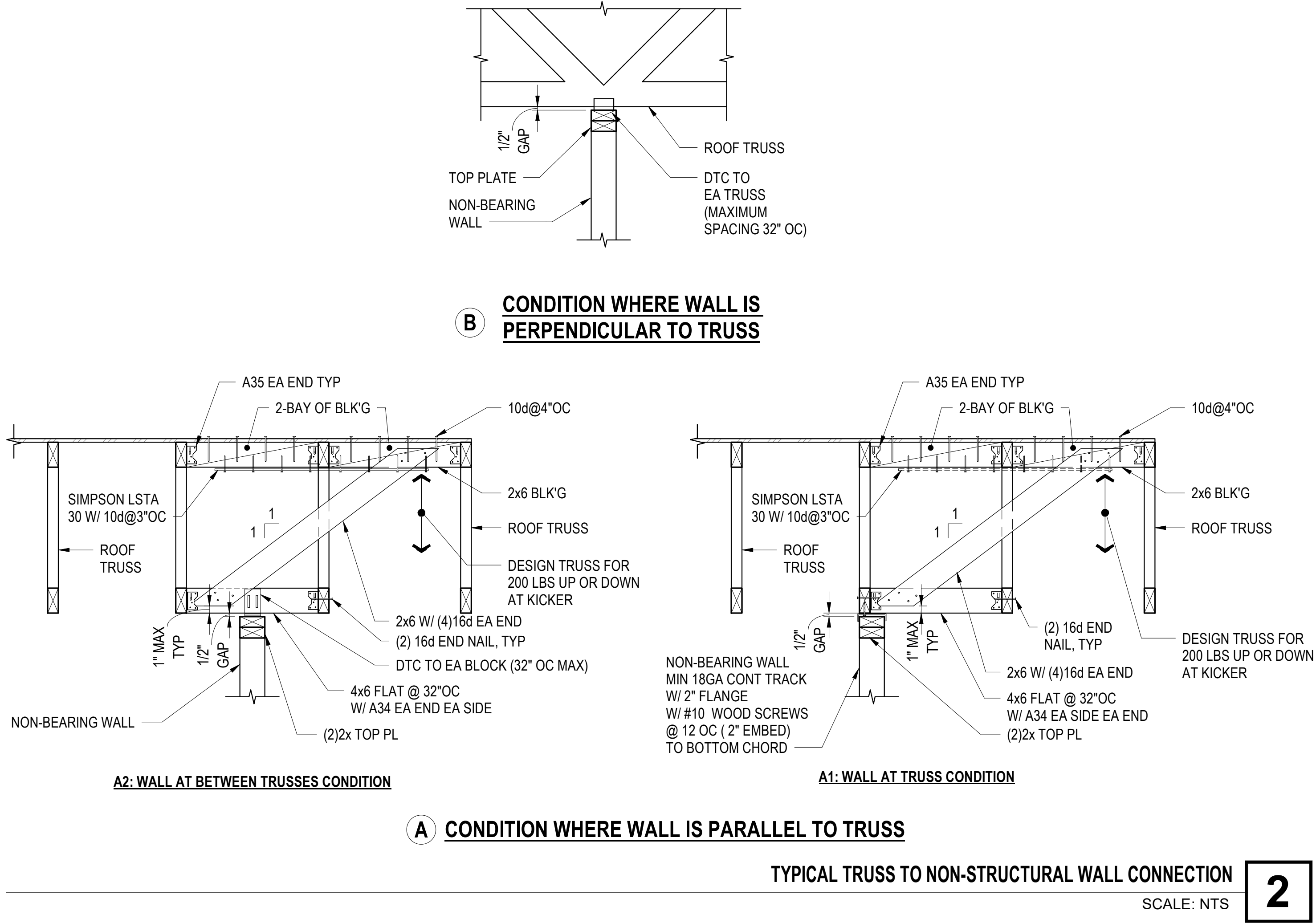
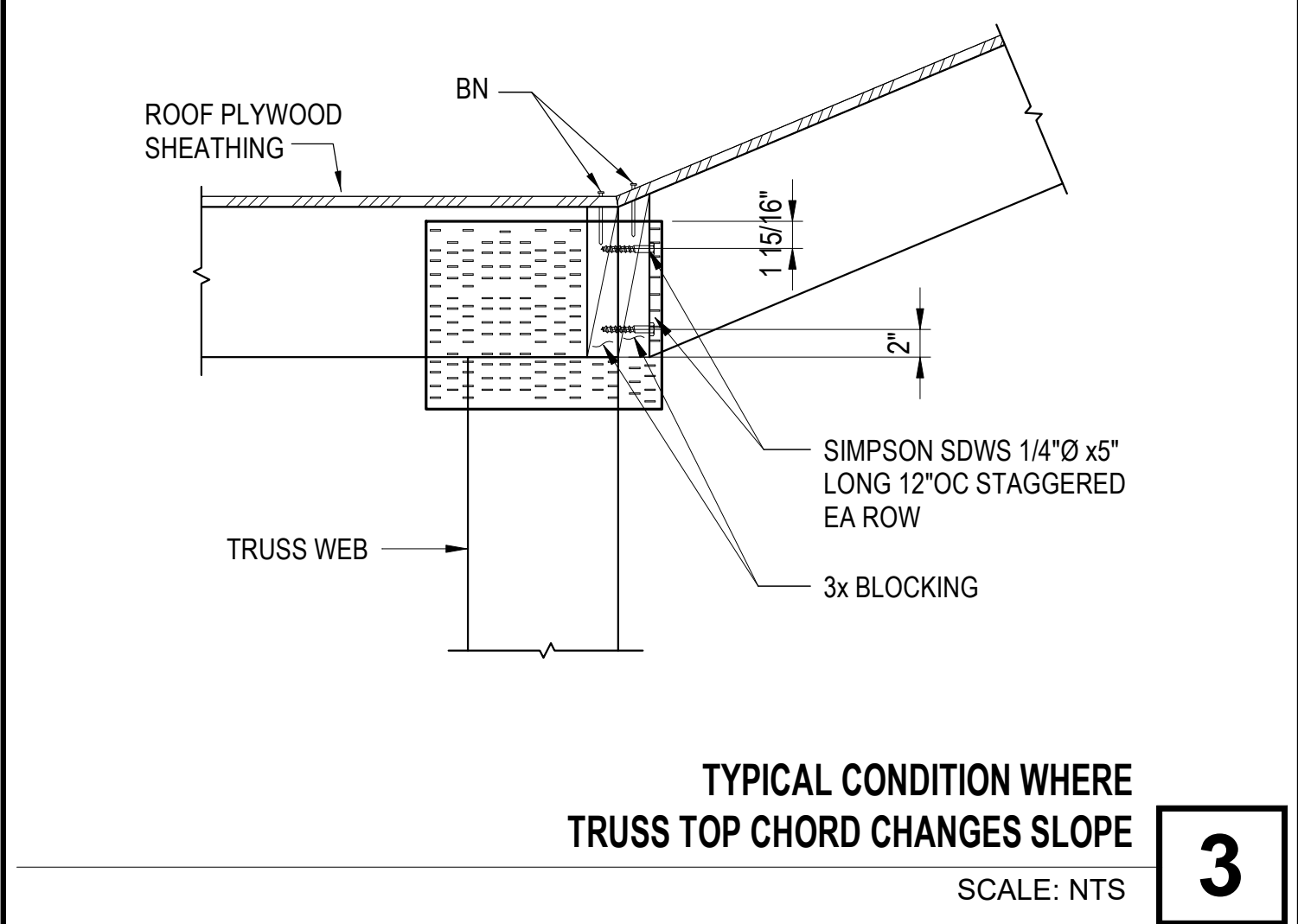
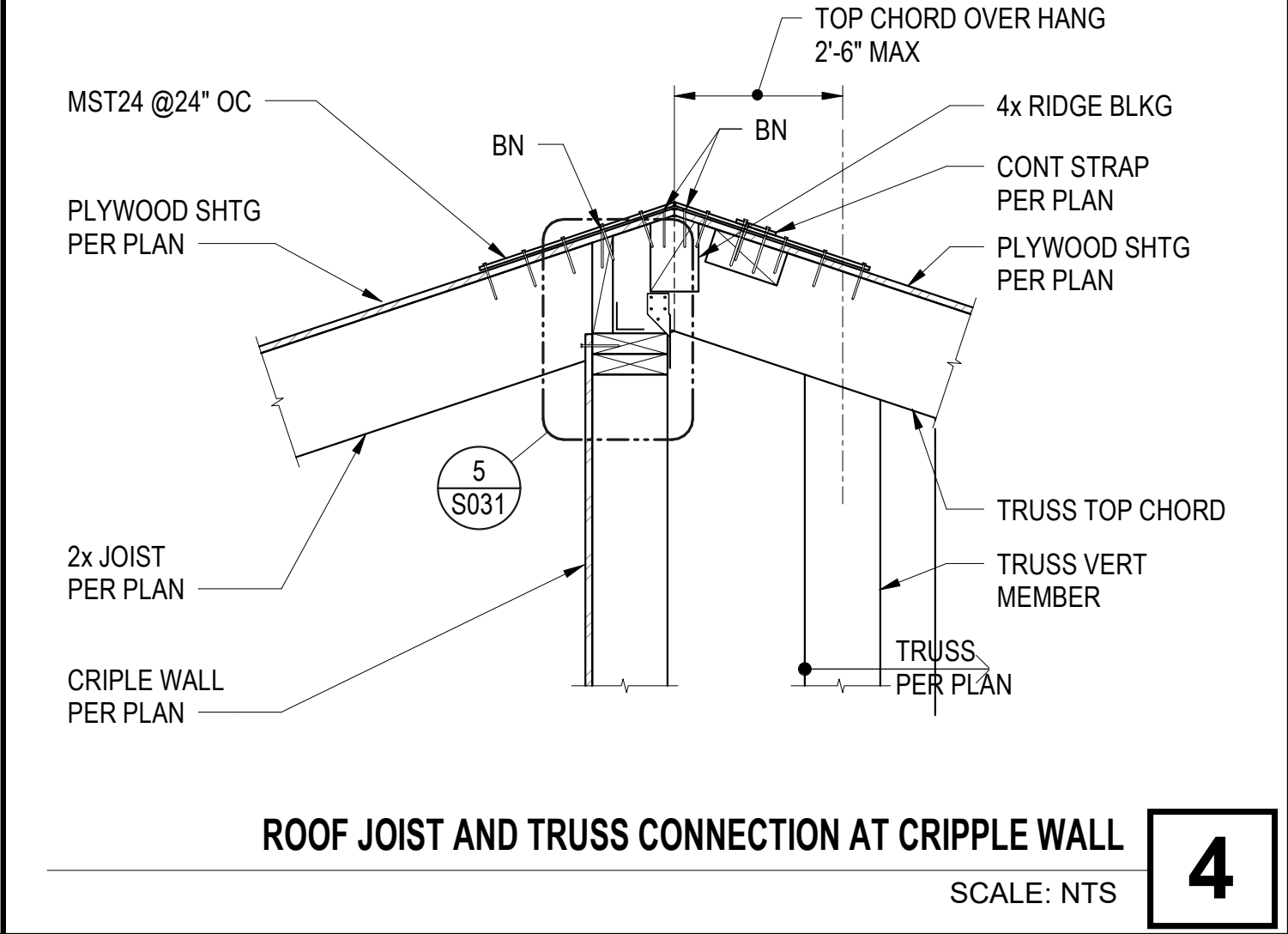
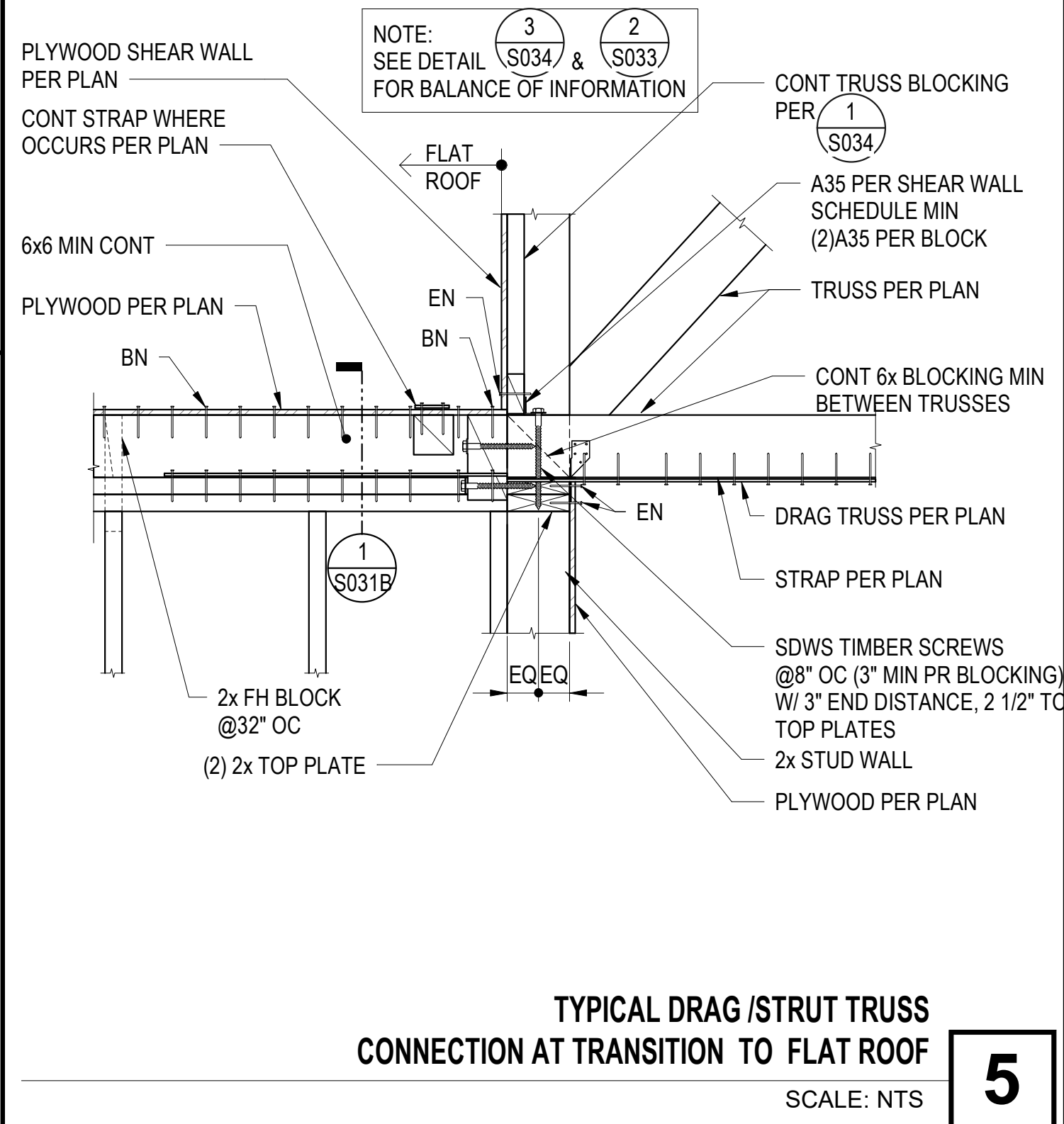


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WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232
TEL 310 392 3995

so
saiful-bouquet
structural engineers

725 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

TYPICAL WOOD DETAILS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S3288
Exp. 12/31/27
STRUCTURAL
STATE OF CALIFORNIA

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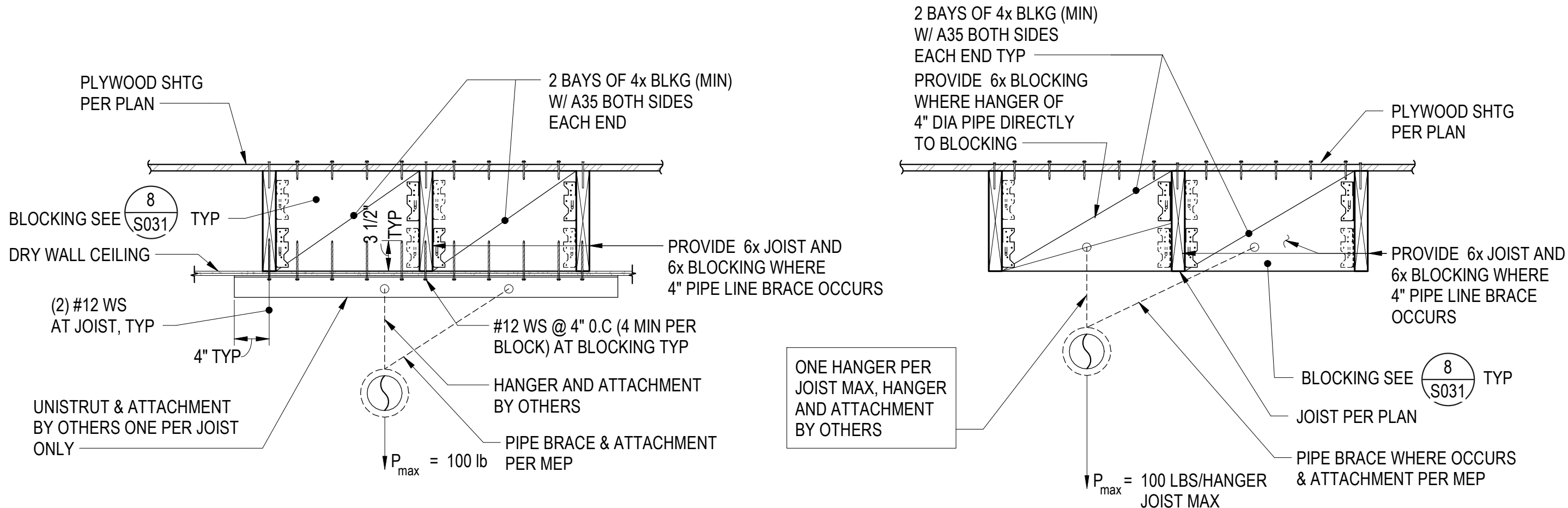
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PROJECT NUMBER

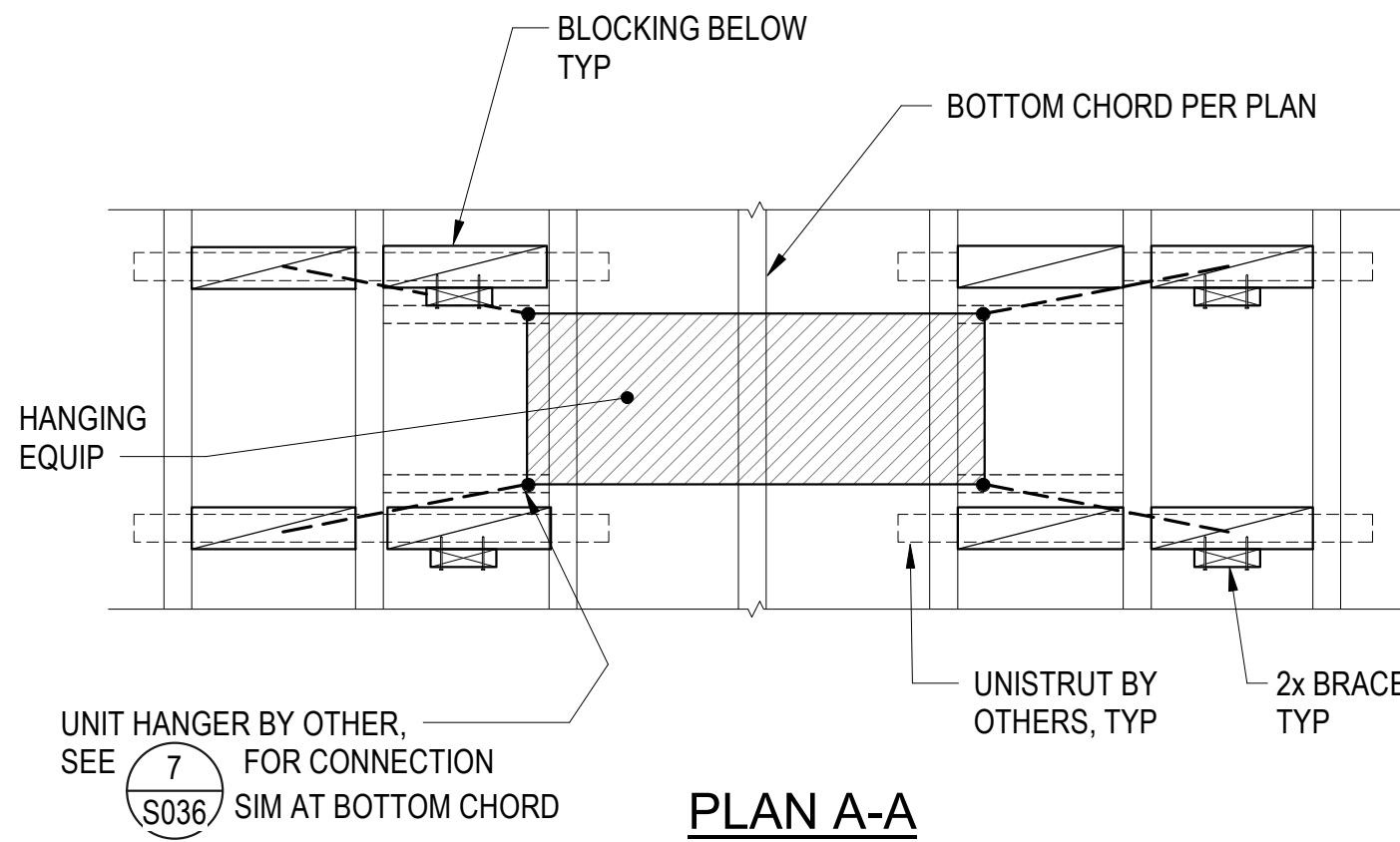
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APPENDIX 5

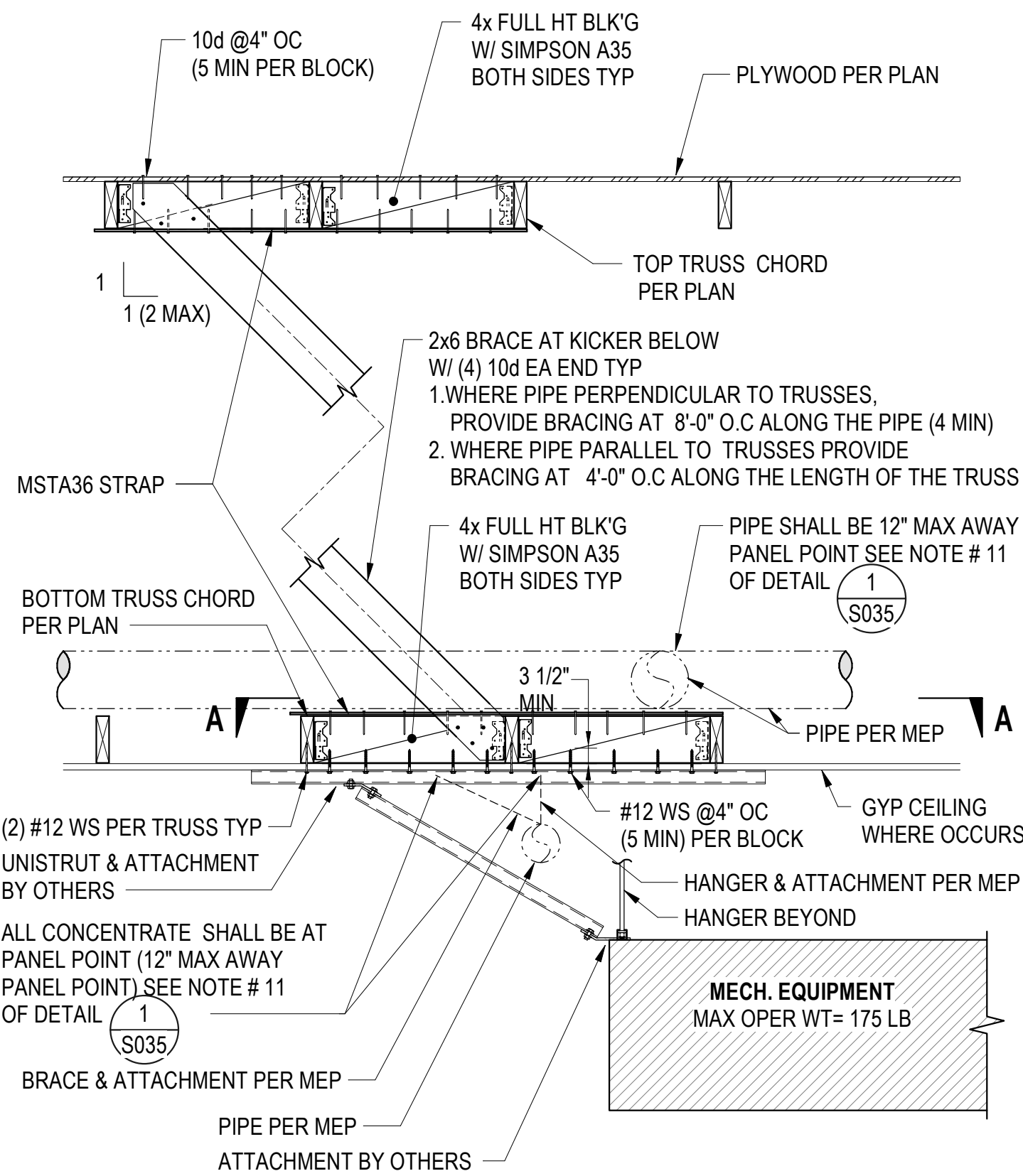
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ALTERNATE ATTACHMENT OPTION



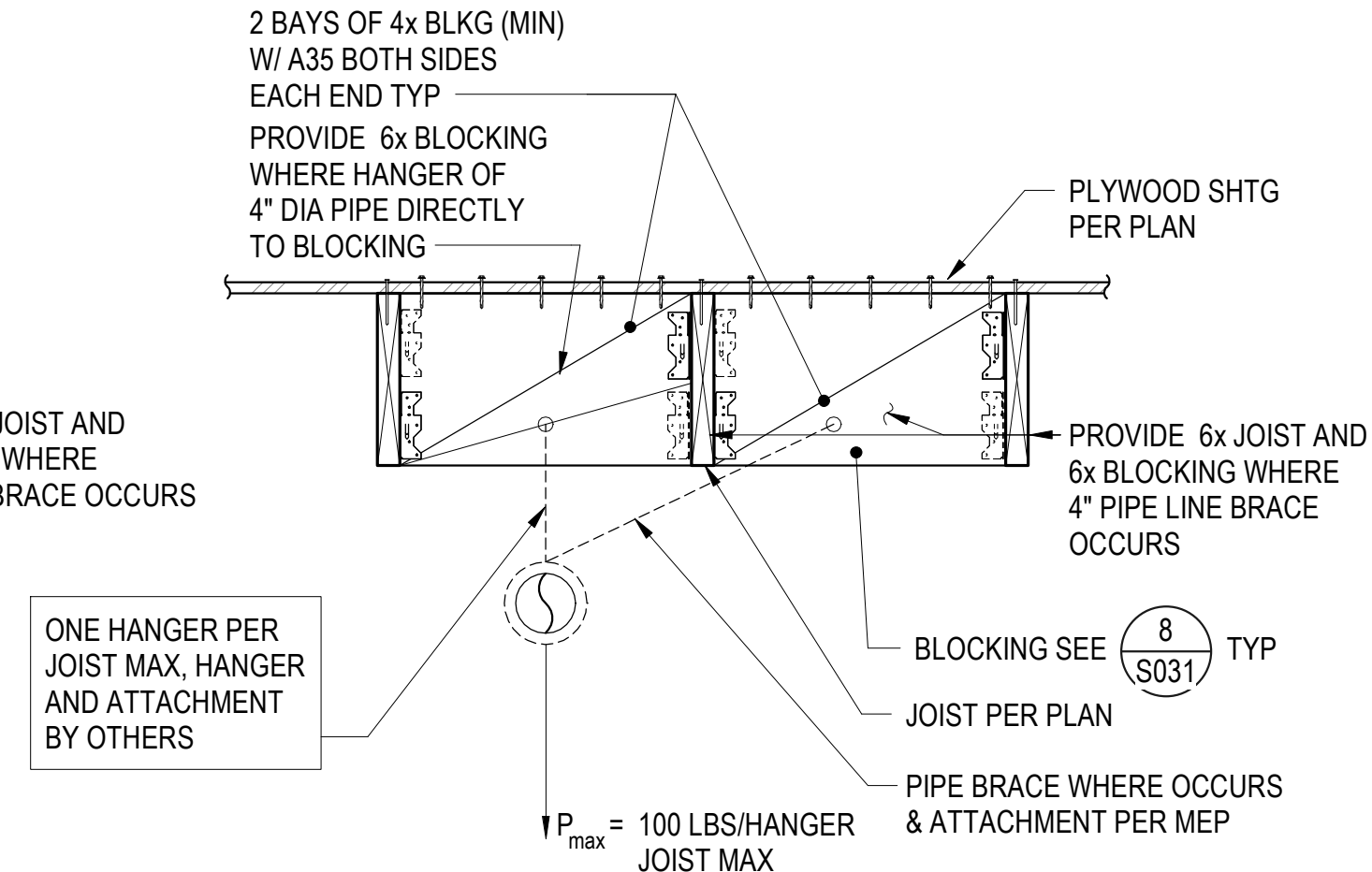
PLAN A-A



TYP HANGING PIPE OR MEP EQUIPMENT SUPPORT DETAIL AT TRUSSES

SCALE: NTS

8

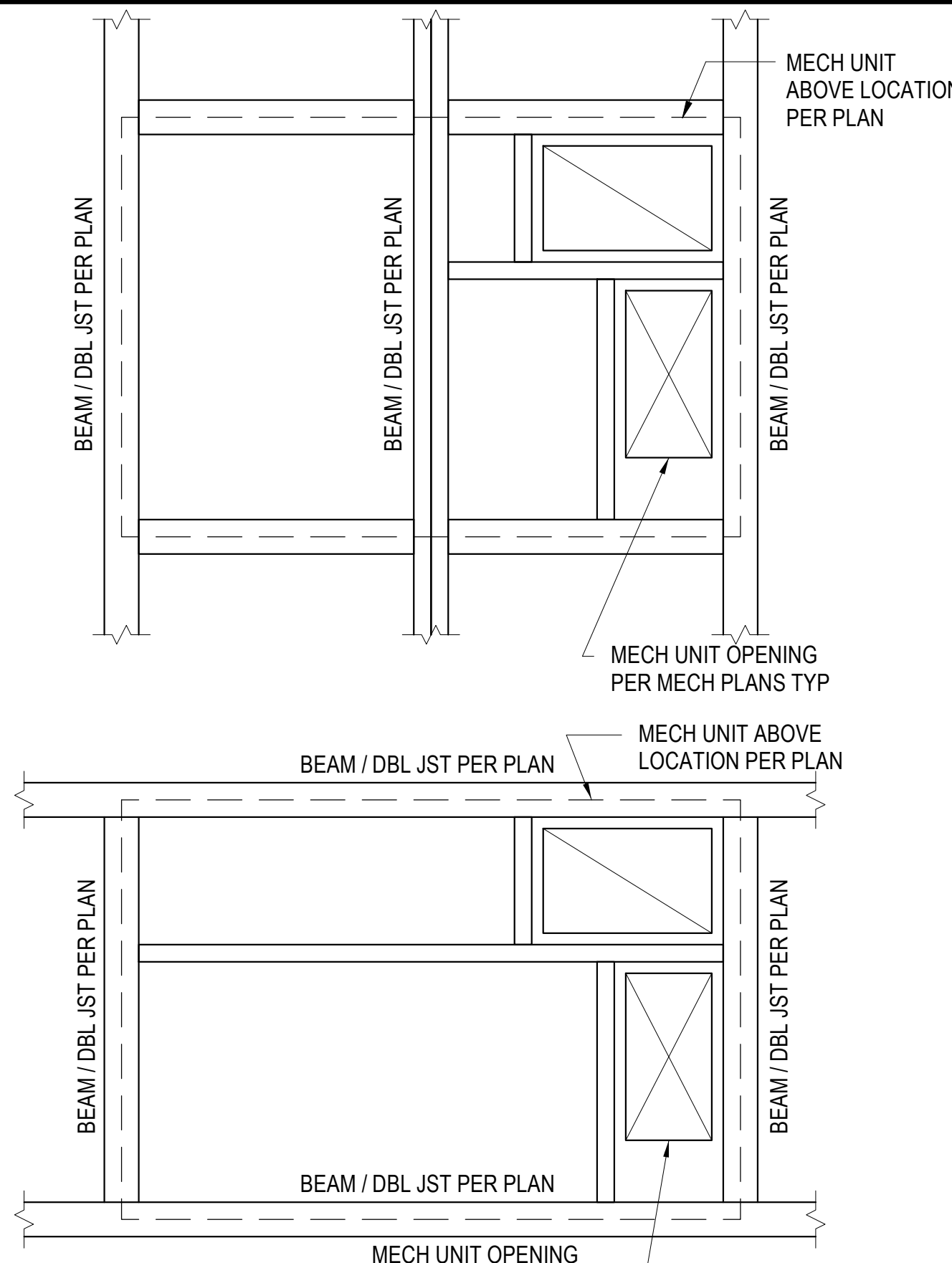


CONDITION AT SAWN LUMBER JOIST OR TOP CHORD TRUSS FRAMING

TYPICAL PIPE HANGER DETAIL

SCALE: NTS

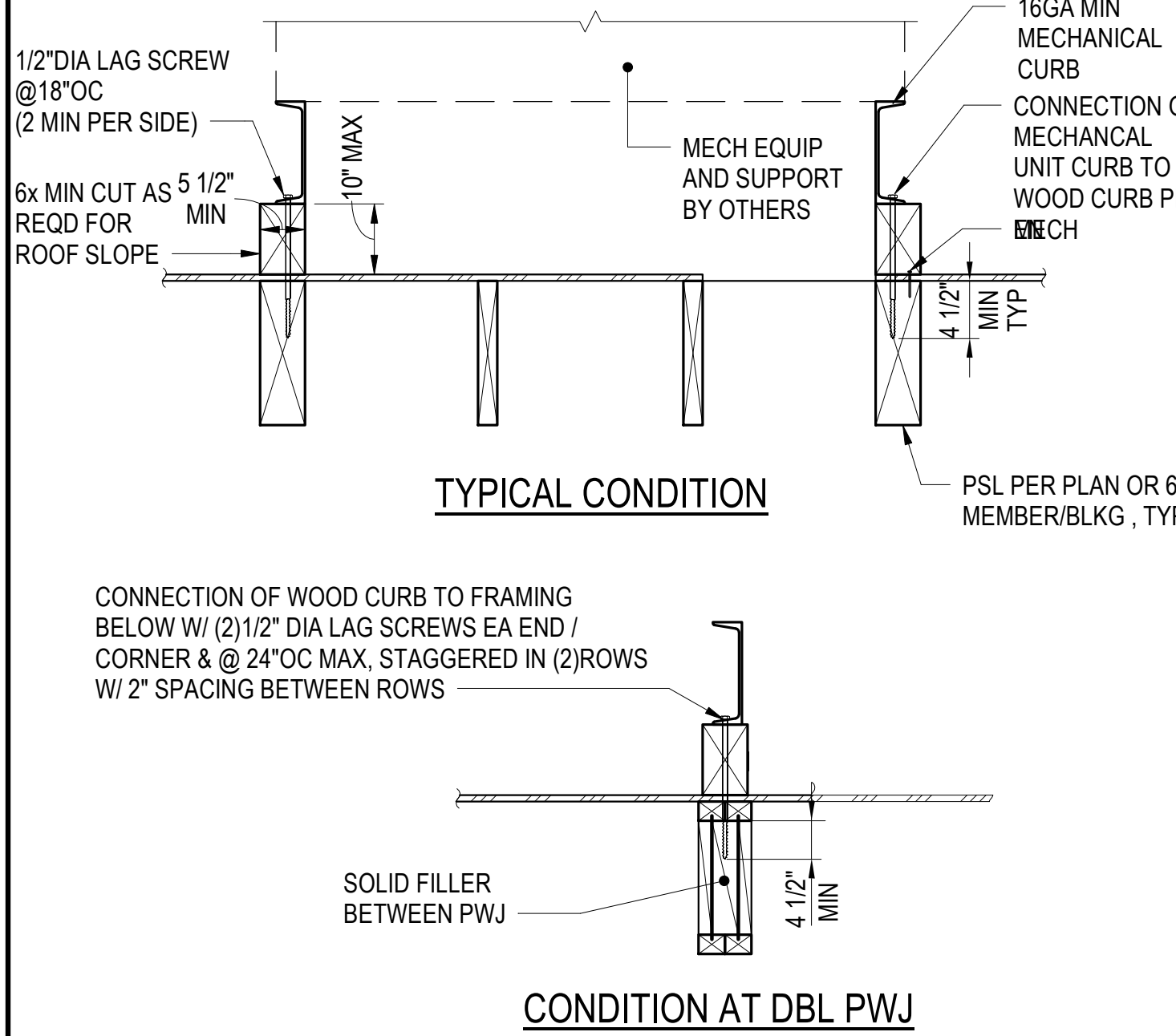
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TYP ROOF FRAMING AT MECHANICAL UNIT AT OPEN ROOF

SCALE: NTS

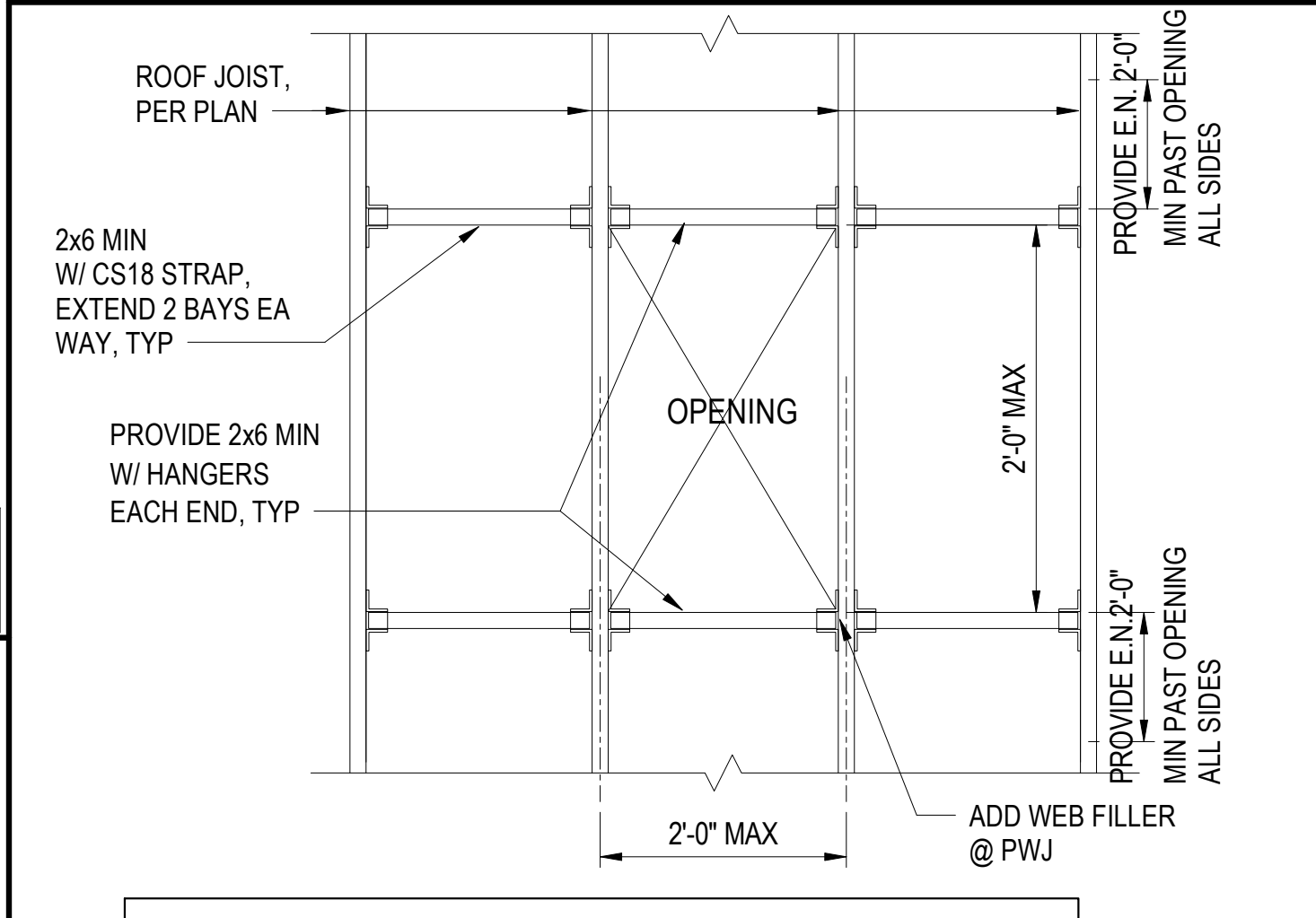
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TYPICAL MECHANICAL UNIT ANCHORAGE DETAIL

SCALE: NTS

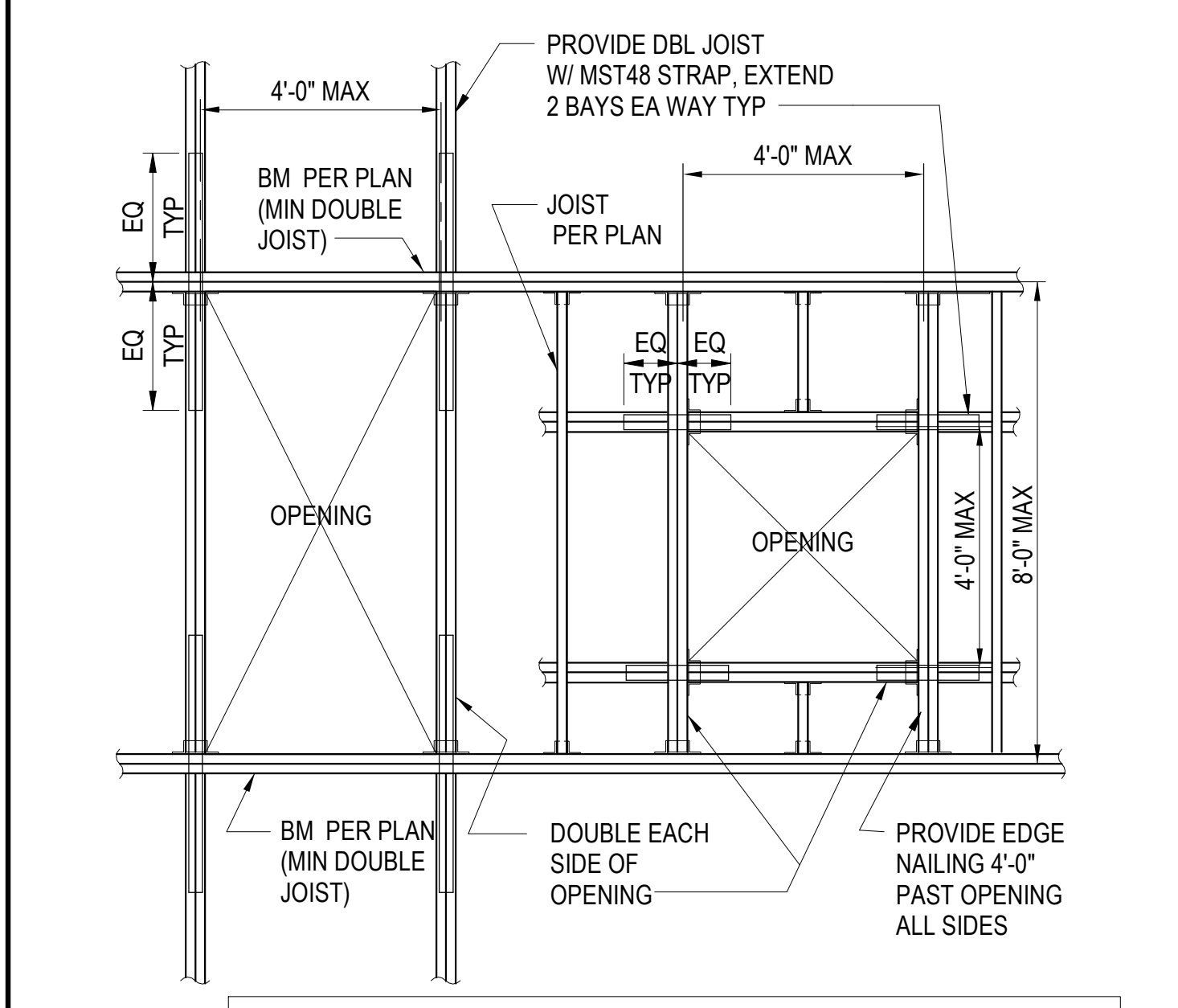
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TYPICAL ROOF FRAMING AT
OPENING BETWEEN JOIST

SCALE: NTS

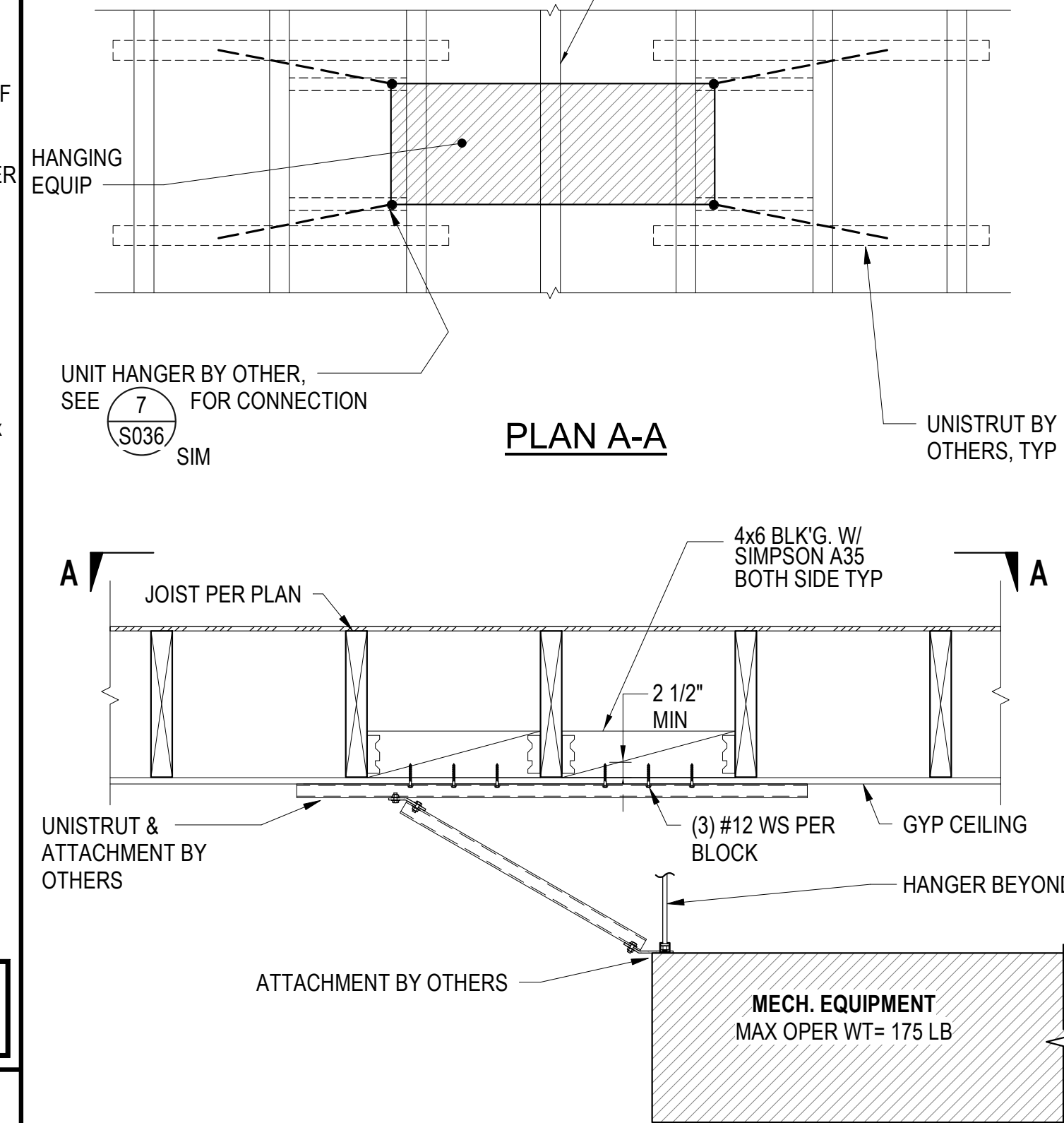
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TYPICAL ROOF FRAMING OPENING DETAIL

SCALE: NTS

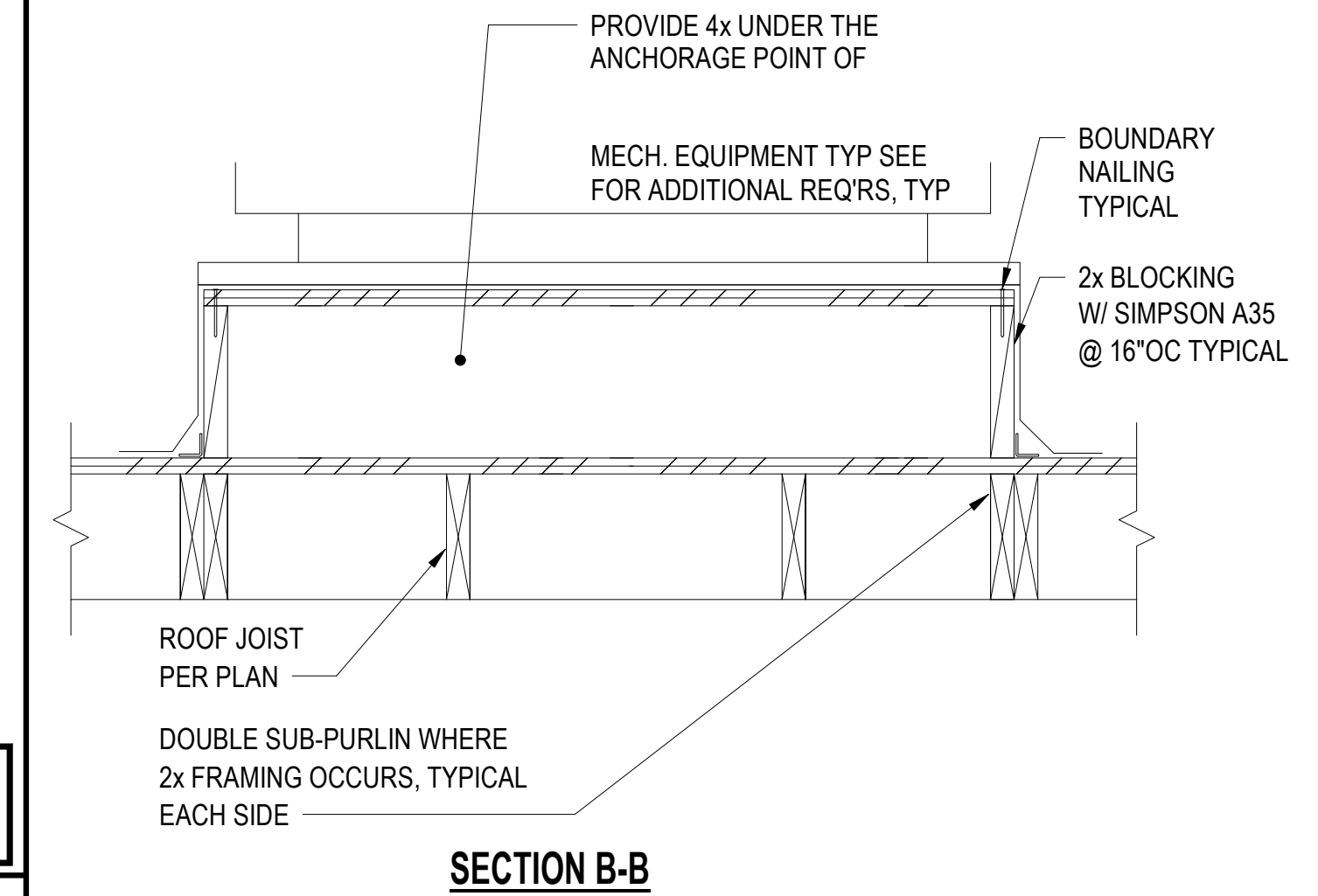
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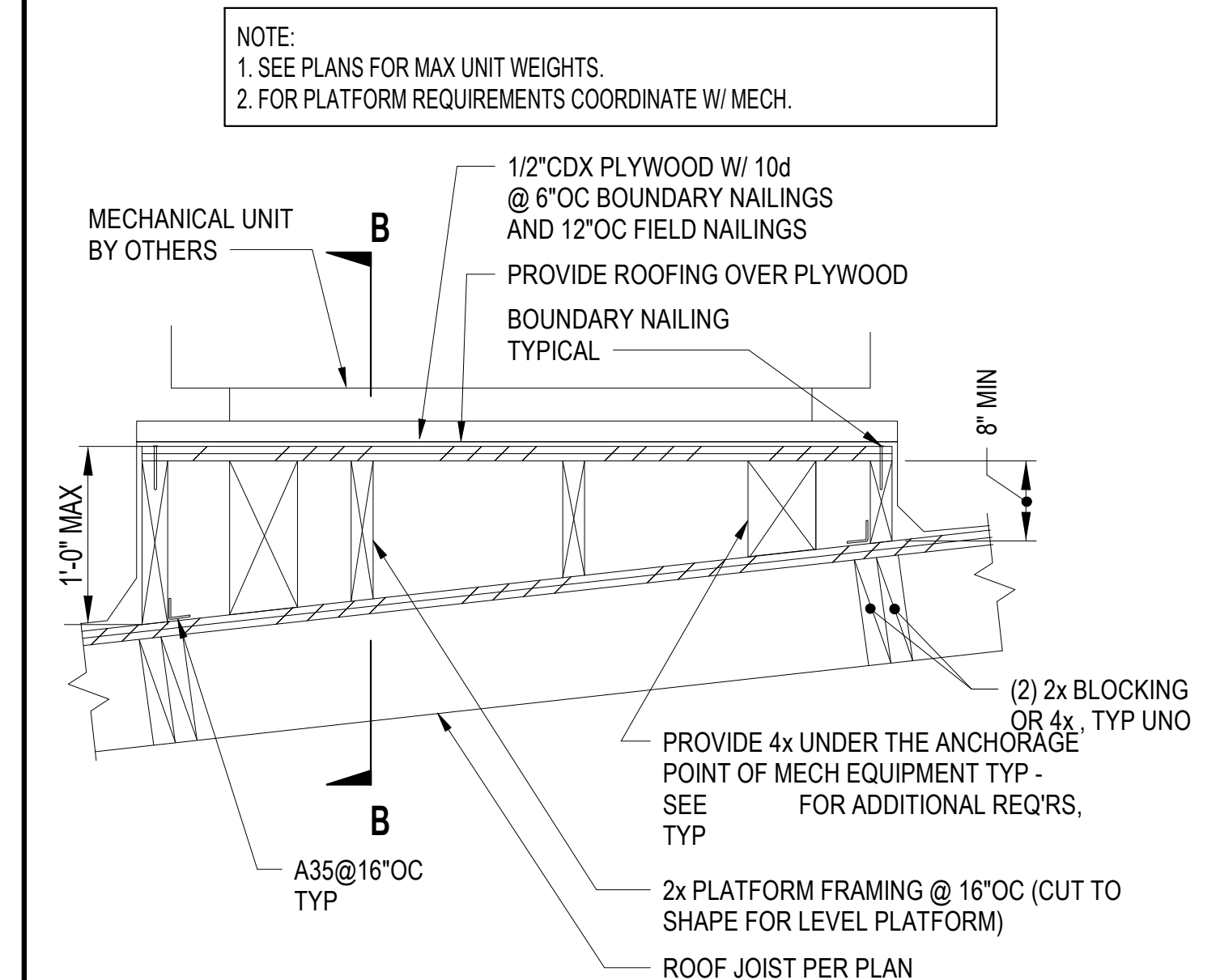
TYP HANGING MEP EQUIPMENT SUPPORT DETAIL AT 2x JOIST

SCALE: NTS

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SECTION B-B



CONDITION AT SAWN LUMBER / ROOF TRUSSES

TYPICAL MECHANICAL UNIT PLATFORM DETAIL

SCALE: NTS

1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232
TEL 310 392 3995

SO
saiful-bouquet
structural engineers

725 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
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TYPICAL WOOD DETAILS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

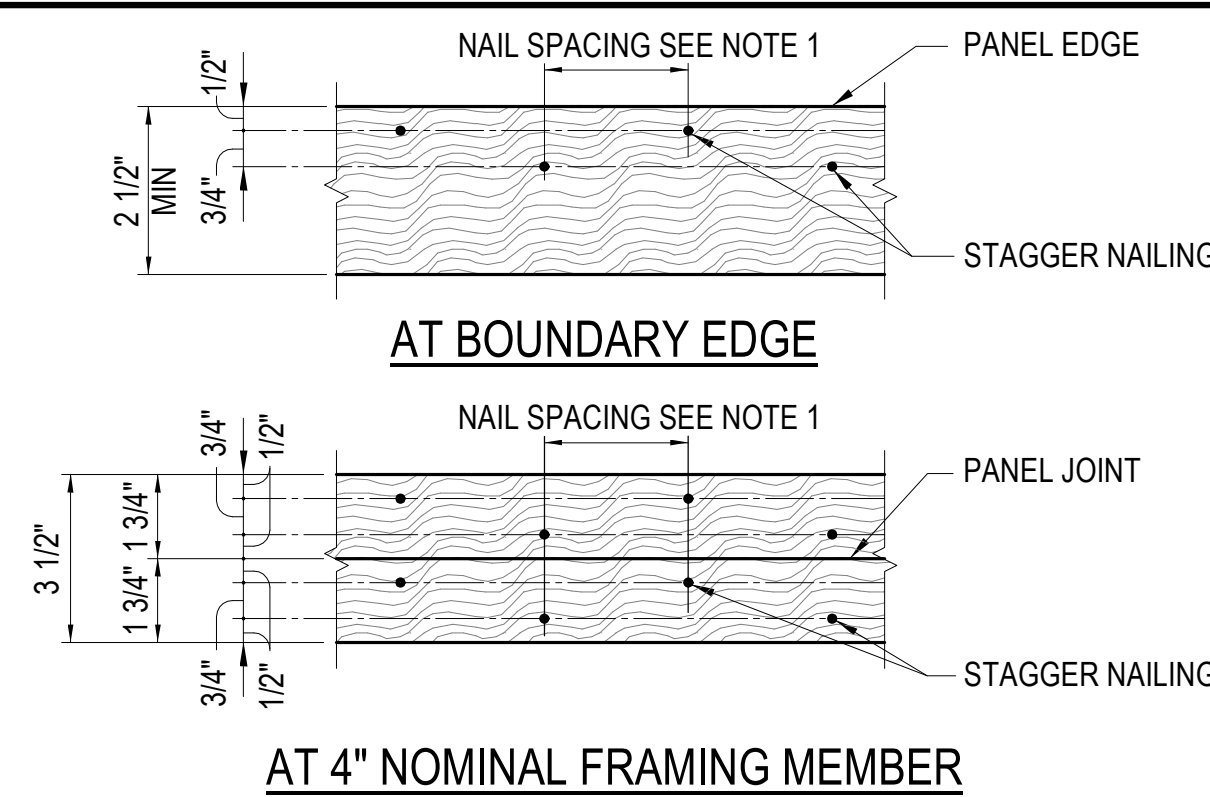
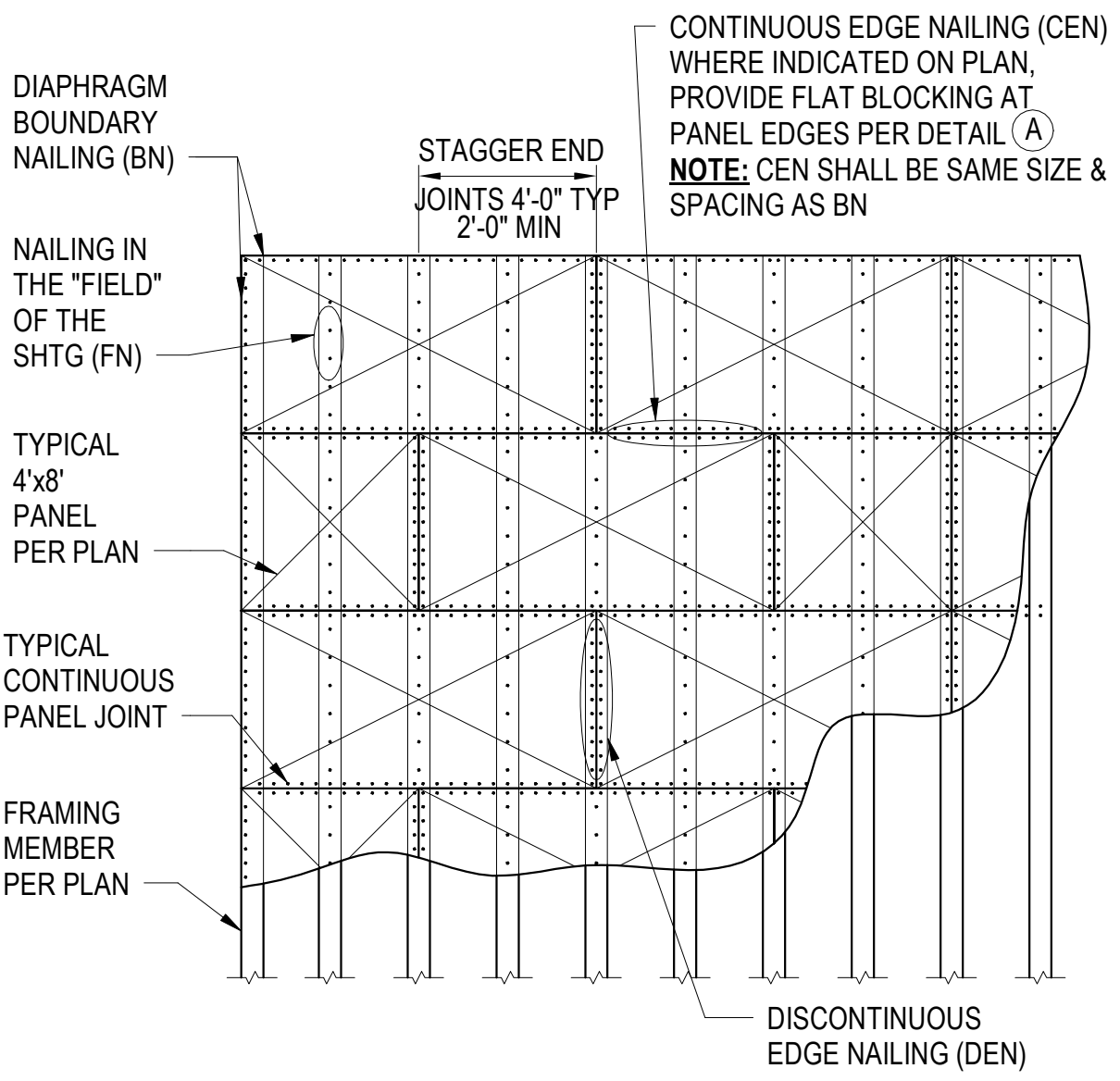
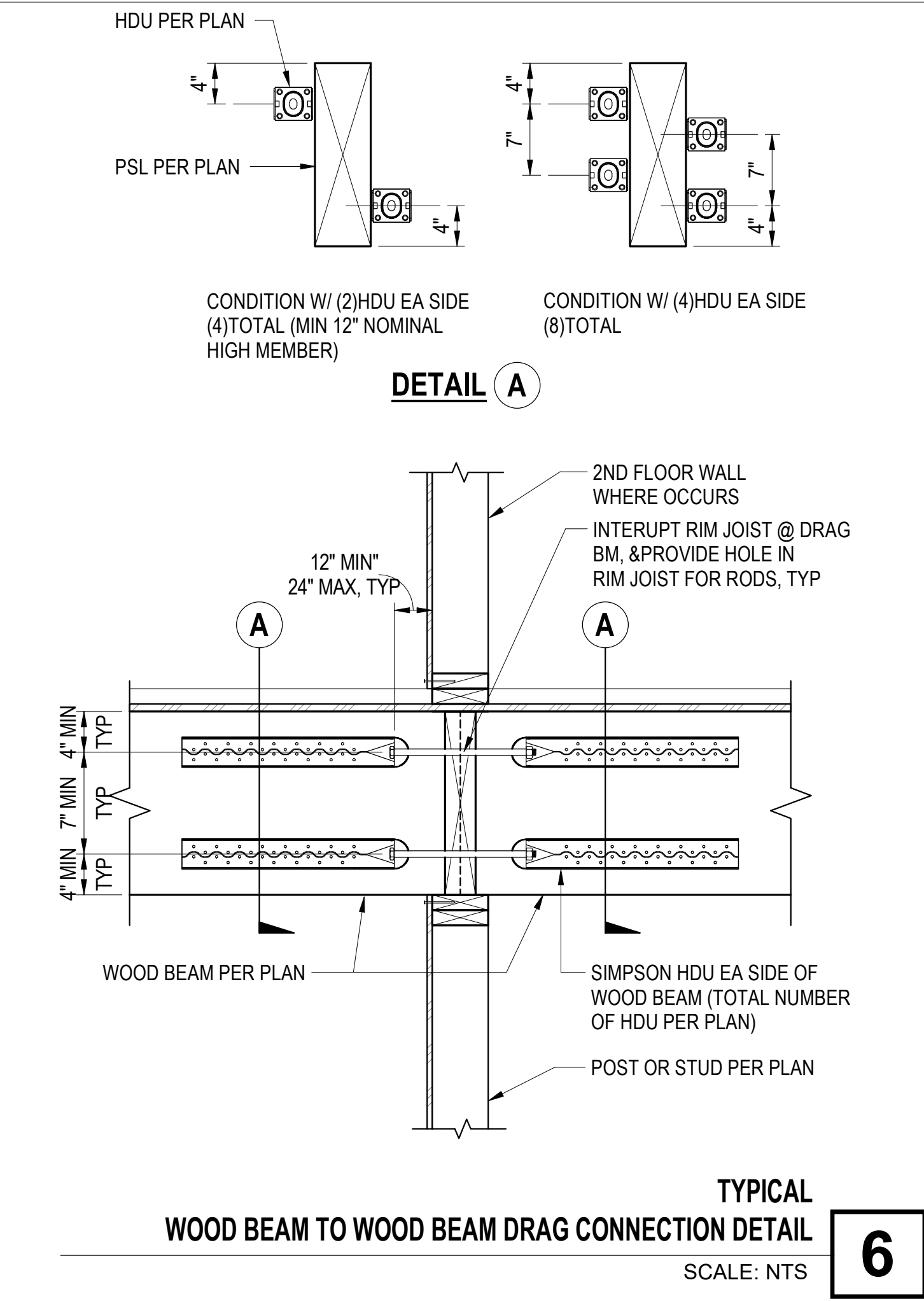
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APPENDIX M 5

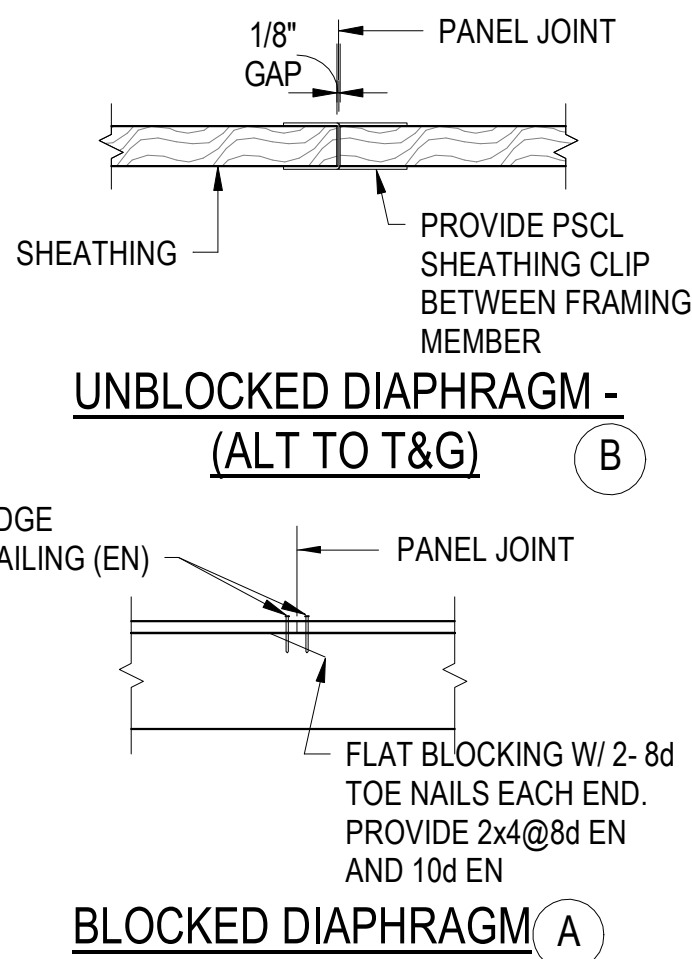
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TYPICAL DIAPHRAGM PANEL NAILING WHERE NAILING SPACING IS LESS THAN 3" OC
SCALE: NTS



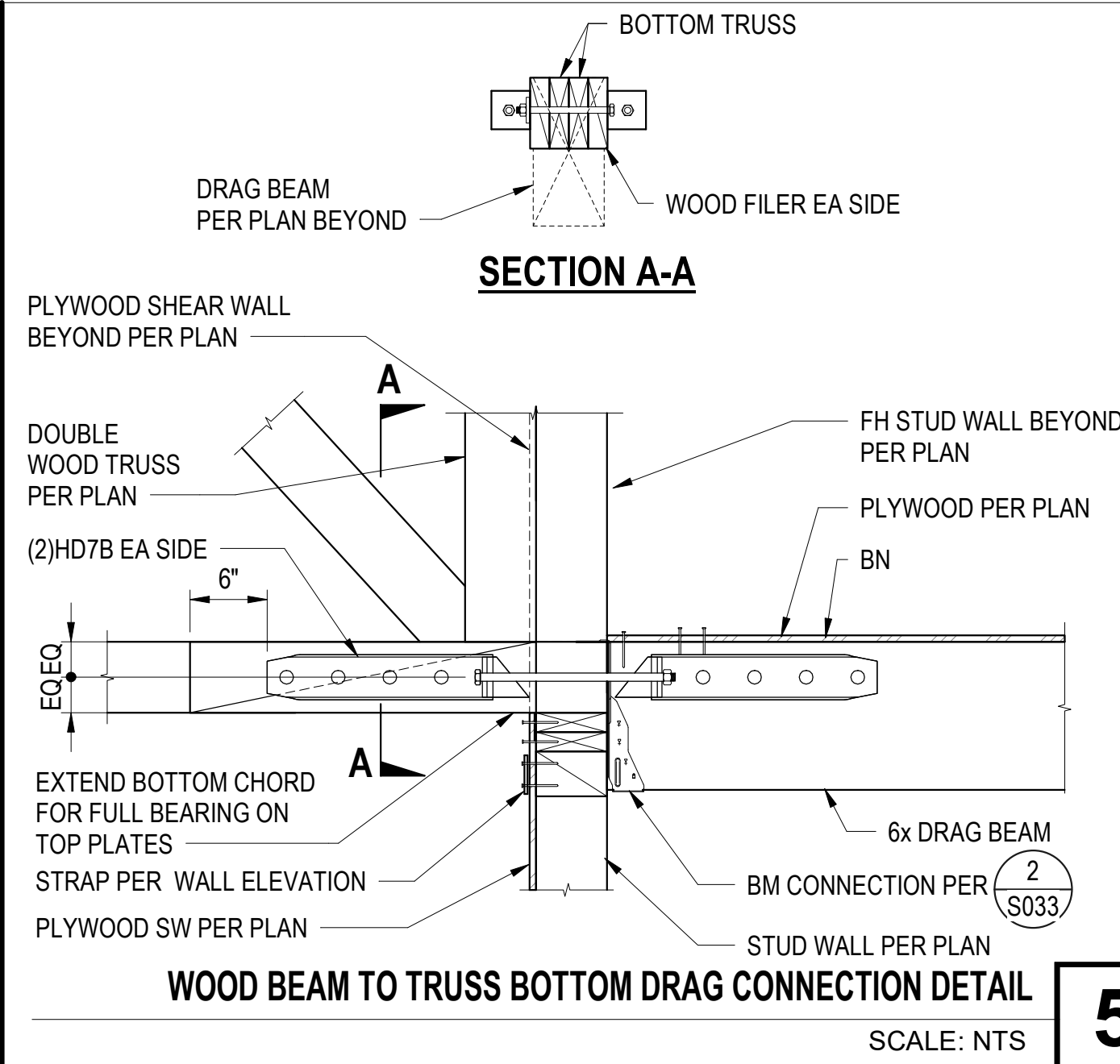
WOOD STRUCTURAL PANEL (WSP)

1. RUN LONG DIMENSION OF WSP PERPENDICULAR TO FRAMING MEMBERS.
2. NAILING SIZE AND SPACING AS NOTED ON PLAN.
3. NAILS SHALL HAVE A MINIMUM 3/8" EDGE DISTANCE.
4. LAY OUT JOISTS IN A 4 FOOT MODULE TO COINCIDE WITH WSP PATTERN.
5. USE TONGUE & GROOVE WSP AT UNBLOCKED FLOOR ON DETAIL (B)
6. USE BOUNDARY NAILING AT FRAME, DRAG BEAMS & SHEAR WALL LOCATIONS. EXTEND BN ALONG ENTIRE DIAPHRAGM LENGTH.
7. PROVIDE EDGE NAILING TO INTERIOR BRACING SUPPORT.
8. WSP SHALL BE GRADE PER NIST PS1-07 AND SHALL BE INTERIOR TYPE SHEATHING C-D GRADE (STRUCT. I) WITH EXTERIOR GLUE.
9. EACH SHEET SHALL HAVE A MINIMUM AREA OF 8 SQUARE FEET WITH A MINIMUM DIMENSION OF 2 FEET.
10. NAIL & GLUE FLOOR SHEATHING TO FRAMING MEMBERS.

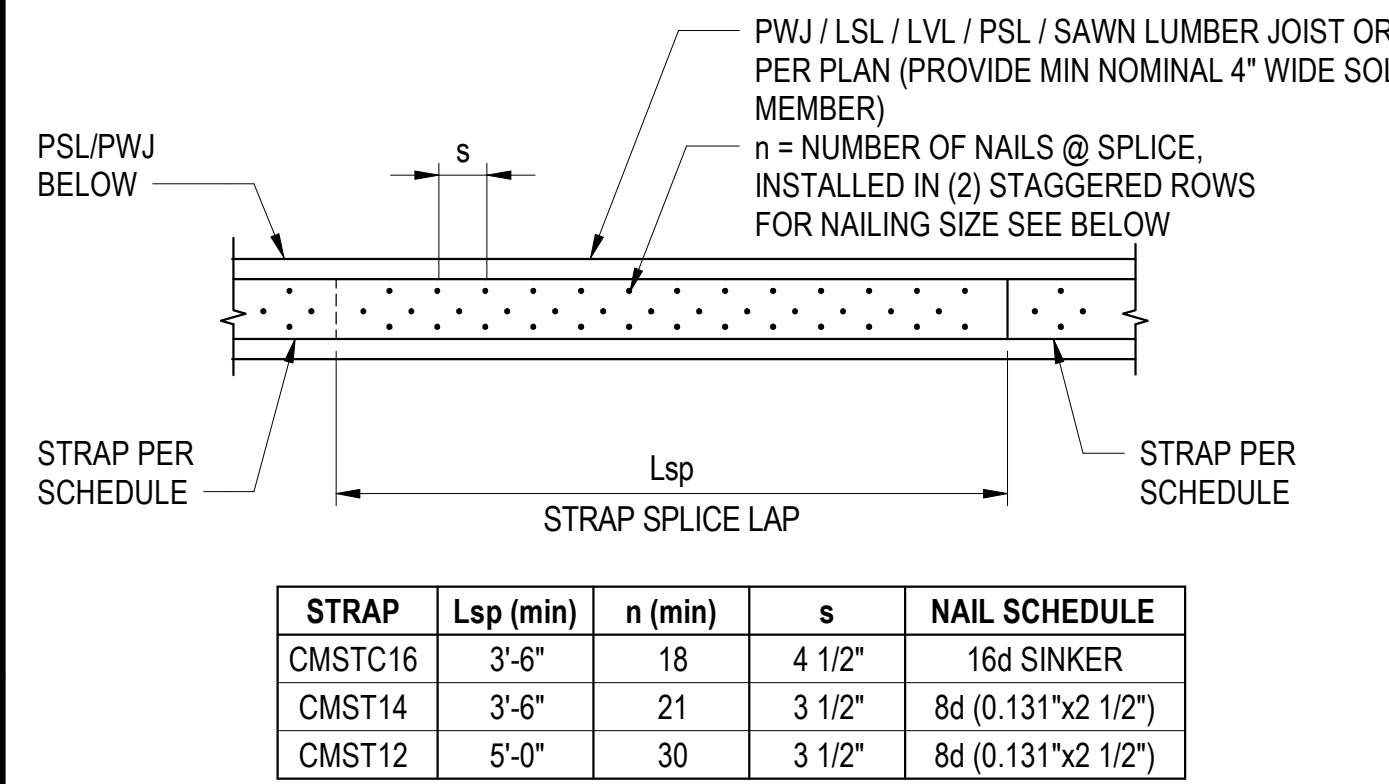
TYPICAL PLYWOOD DIAPHRAGM CONSTRUCTION

SCALE: NTS

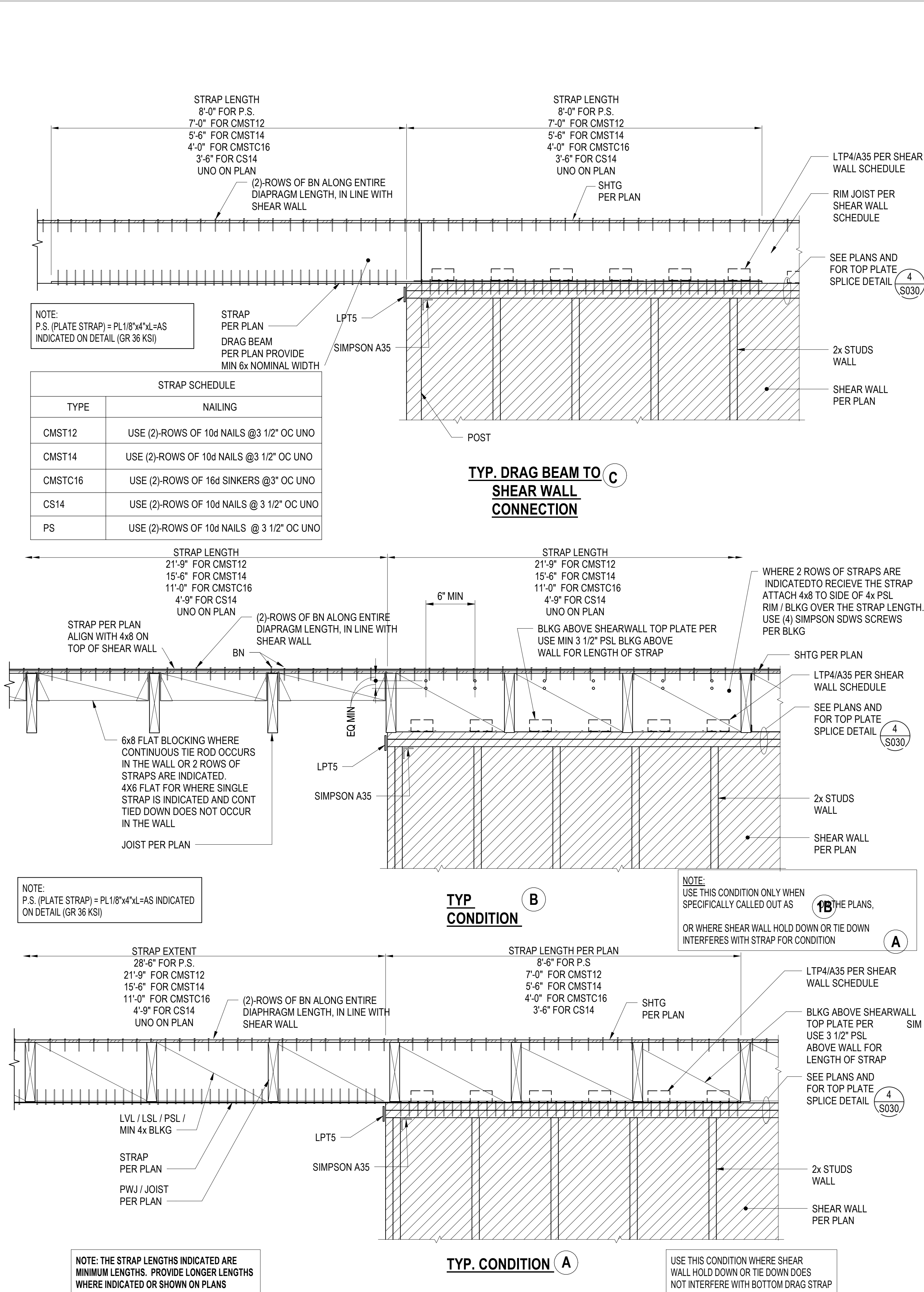
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5



4



1

WILLIAM LOYD JONES ARCHITECT

9415 culver boulevard
culver city, california 90230
www.willbouquet.com
(213) 315-2277
Project #25534

725 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.willbouquet.com
(213) 315-2277
Project #25534

TYPICAL WOOD DETAILS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S3289
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APPENDIX M 5

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Drawn	AS NOTED

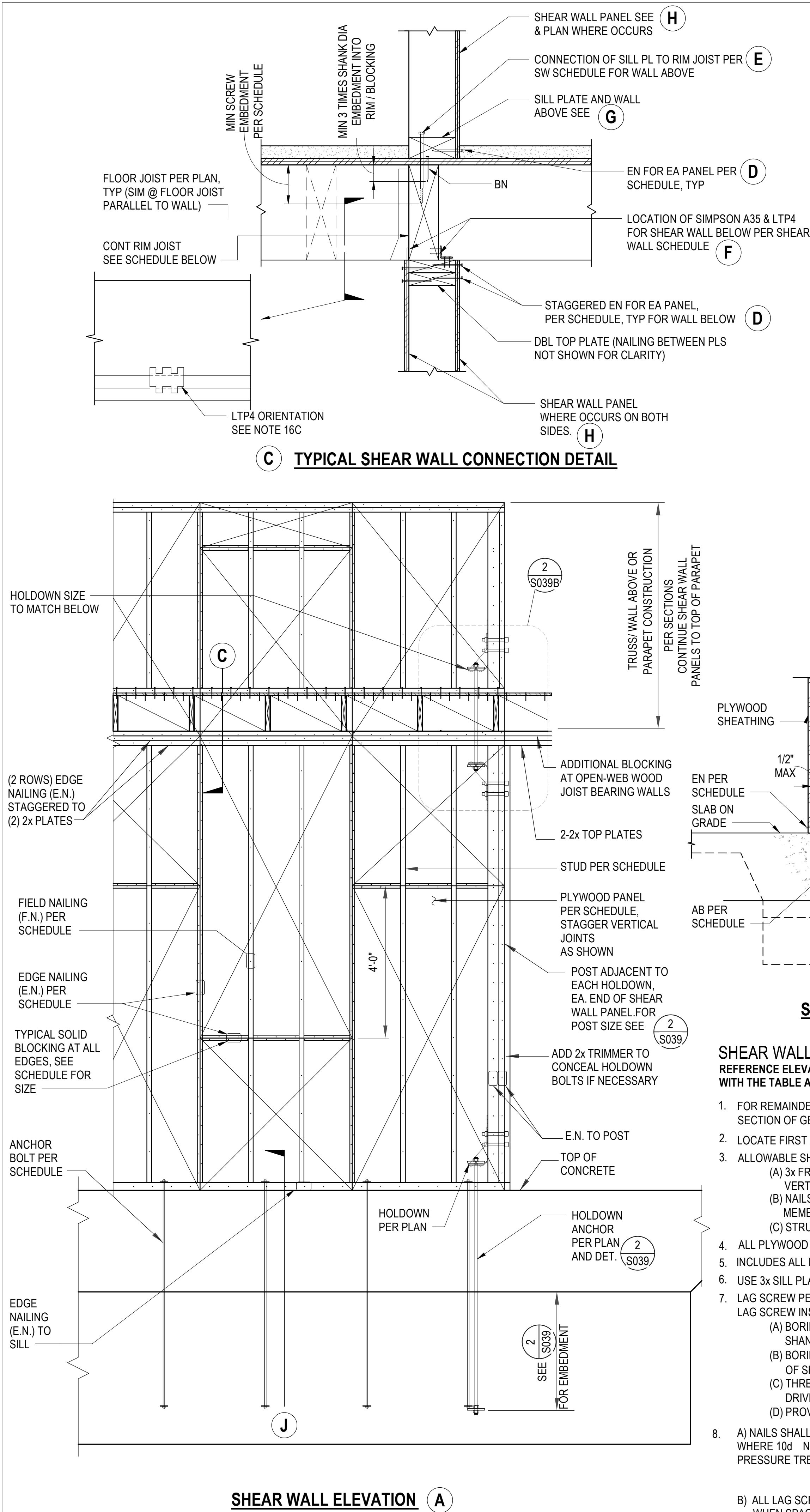
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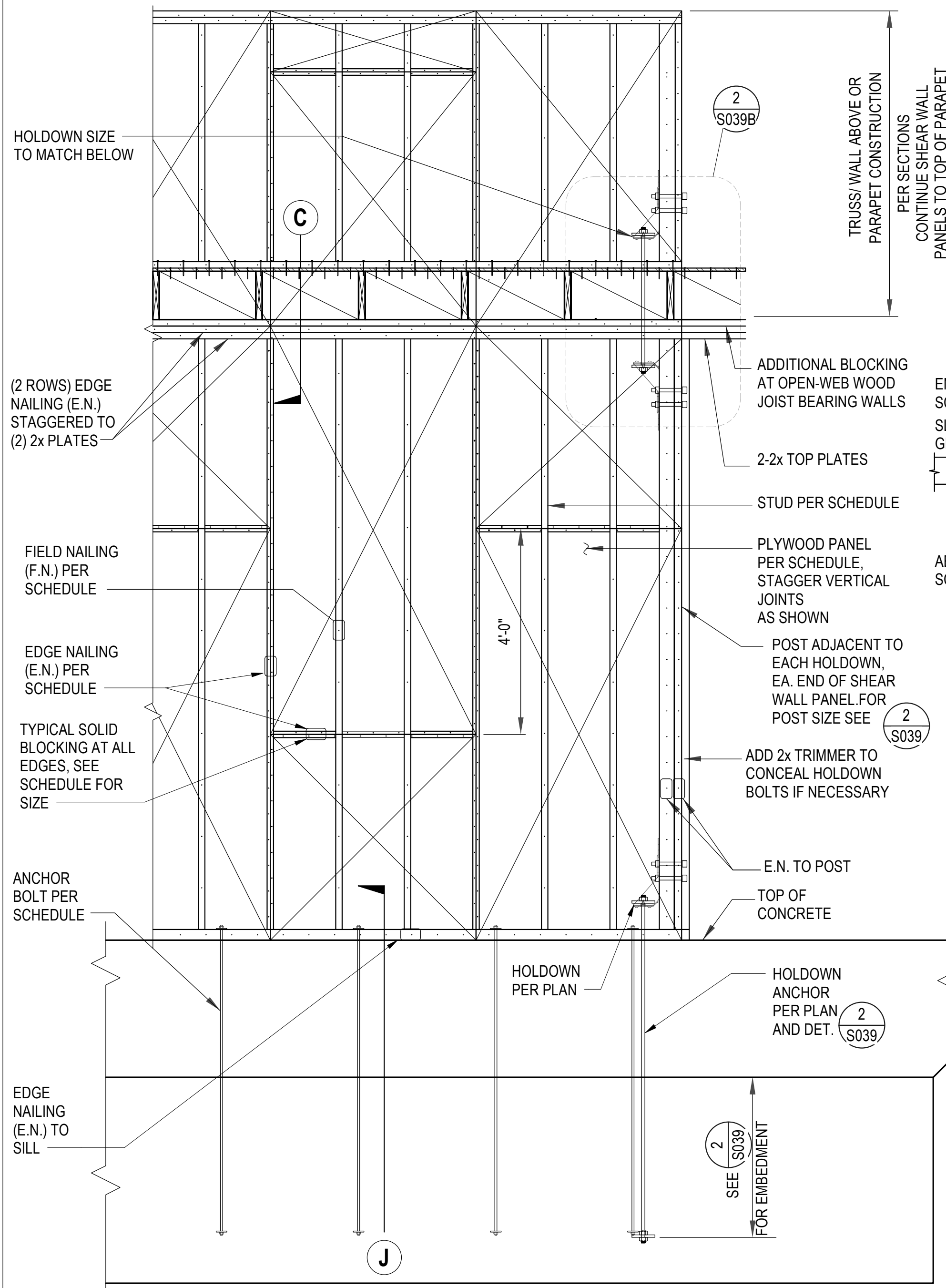
Job. No.	

S037

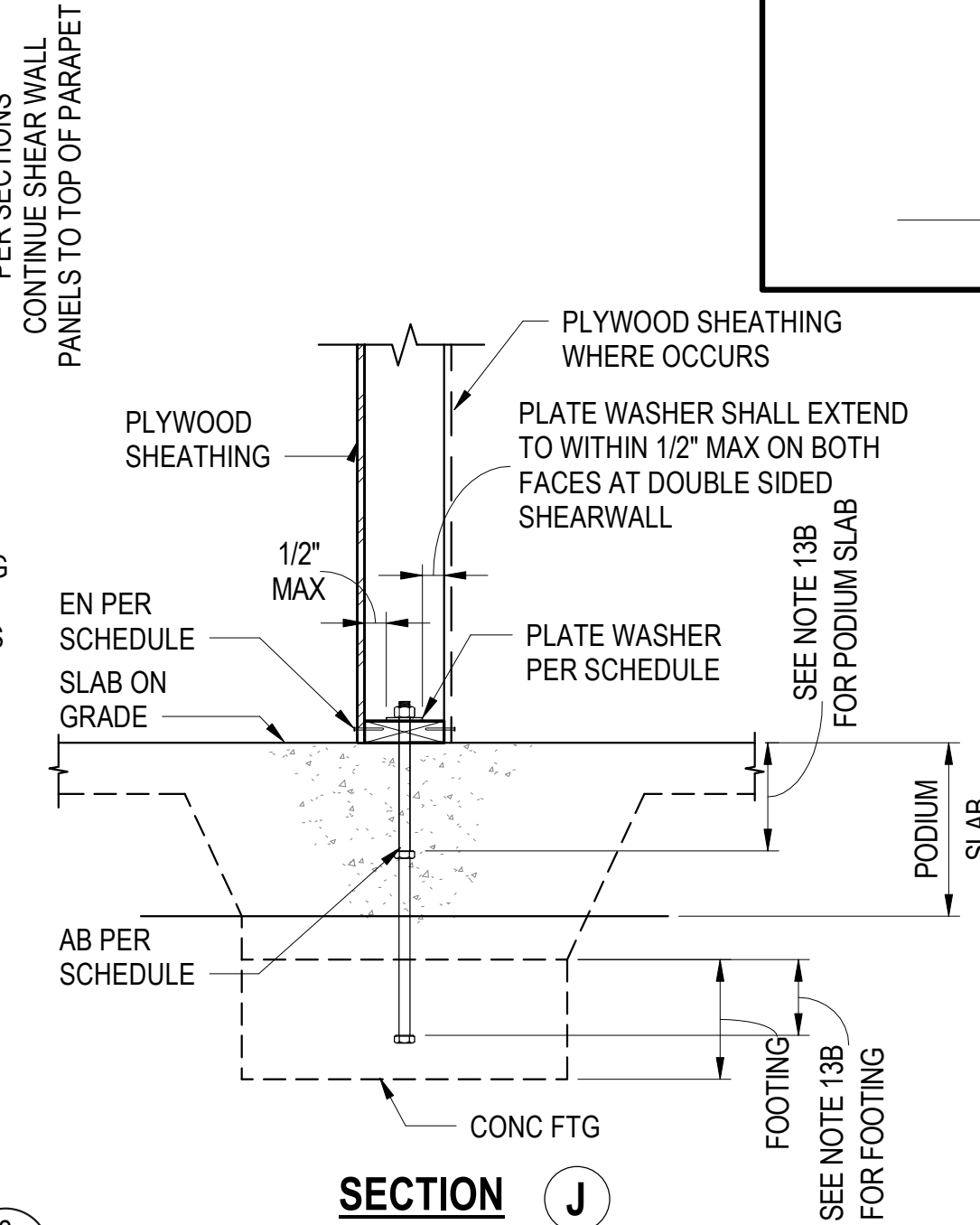
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C TYPICAL SHEAR WALL CONNECTION DETAIL



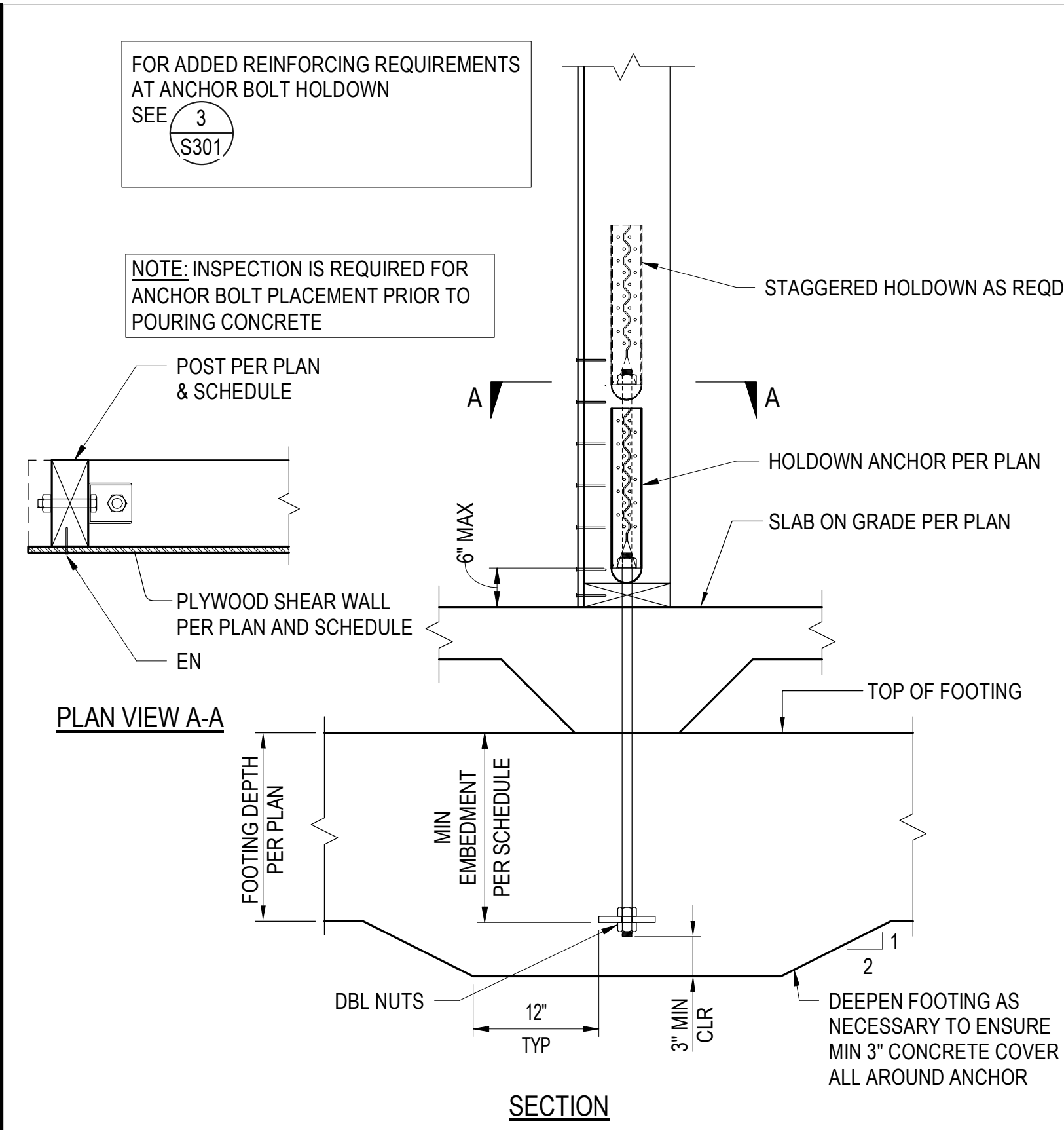
A SHEAR WALL ELEVATION



J SECTION

SHEAR WALL SCHEDULE NOTES:
REFERENCE ELEVATION A AND TYPICAL CONNECTION DETAIL C IN CONJUNCTION WITH THE TABLE ABOVE AND NOTES BELOW

- FOR REMAINDER OF INFORMATION SEE ROUGH CARPENTRY AND TIMBER SPECIFICATIONS NOTES SECTION OF GENERAL NOTES.
- LOCATE FIRST ANCHOR BOLT 9" MAXIMUM FROM ENDS OF SILL PLATE.
- ALLOWABLE SHEAR WALL CAPACITIES GREATER THAN 350 PLF REQUIRE:
(A) 3x FRAMING MEMBERS AT BOTTOM SILL PLATE AND ADJOINING HORIZONTAL AND VERTICAL PANEL EDGES.
(B) NAILS SHALL BE PLACED NOT LESS THAN 3/8 INCH FROM EDGES OF 2x FRAMING MEMBERS AND NOT LESS THAN 1/2 INCH FROM PANEL EDGES FOR 3x.
(C) STRUCTURAL OBSERVATION BY A LICENCED ENGINEER OR ARCHITECT.
- ALL PLYWOOD SHALL BE STR 1 4 PLY MINIMUM AND HAVE A PLYWOOD INDEX OF 32/16.
- INCLUDES ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS.
- USE 3x SILL PLATE @ FOUNDATION AND /OR PODIUM SLAB.
- LAG SCREW PENETRATION SHALL BE PROVIDED AS SHOWN ON DETAILS.
LAG SCREW INSTALLATION REQUIRE:
(A) BORING OF CLEARANCE HOLE FOR SHANK PORTION SHALL HAVE SAME DIAMETER OF SHANK AND SAME DEPTH OF PENETRATION LENGTH OF UNTHREADED SHANK.
(B) BORING OF LEAD HOLE FOR THREADED PORTION SHALL HAVE DIAMETER EQUAL TO 50% OF SHANK DIAMETER.
(C) THREADED PORTION SHALL BE INSTALLED IN LEAD HOLE BY TURNING WITH WRENCH. DRIVING WITH HAMMER SHALL NOT BE PERMITTED.
(D) PROVIDE LEAD HOLES FOR SIMPSON SDS W/ SPACING LESS THAN 6" OC
- A) NAILS SHALL BE STAGGERED AT ADJACENT HORIZONTAL AND VERTICAL PANEL EDGES WHERE 10d NAILS ARE PLACED 3 INCHES OR LESS ON CENTER. ALL NAILS APPLIED TO PRESSURE TREATED SILL SHALL BE GALVANIZED, SEE (2) S037
B) ALL LAG SCREWS INSTALLED INTO 4x WOOD MEMBER SHALL BE STAGGERED AS SHOWN WHEN SPACED 4" OC OR LESS

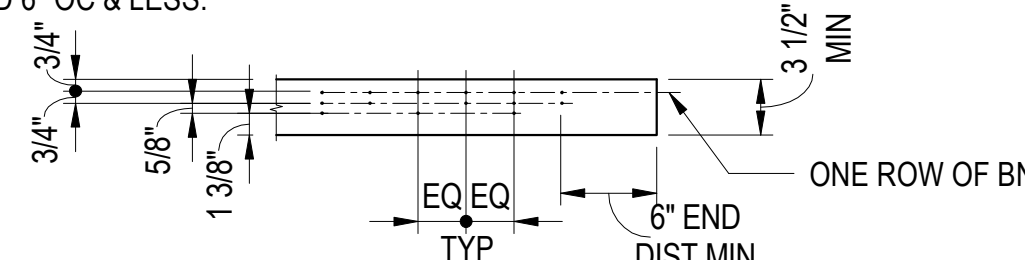


2 TYPICAL HOLDOWN SCHEDULE AND DETAIL

SCALE: NTS

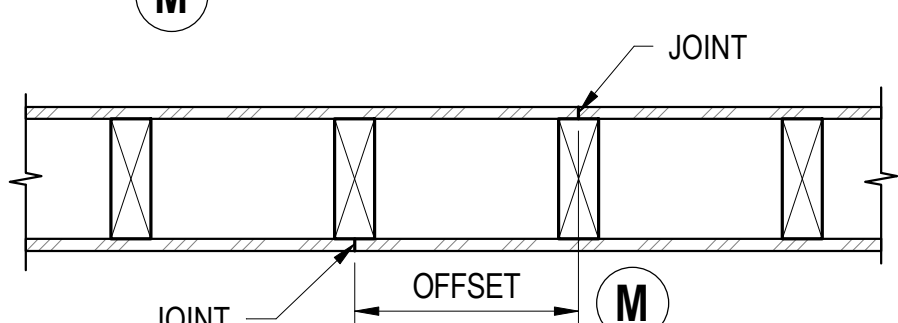
WOOD STRUCTURE PANEL SHEAR WALL SCHEDULE (REFERENCE ELEVATION A & TYPICAL DETAIL C THAT ARE PART OF THIS DETAIL)										
SHEAR WALL MARK (H)	NO. OF SIDES OF SHTG	DESIGN SHEAR	STRUCT'L WSP THICKNESS	SILL PLATE SIZE (G)	STUD AND BLK'G SIZE AT ADJOINING PANEL EDGES (5)	ANCHORS AT SILL PLATE TO CONCRETE SLAB OR WALL	PANEL NAILING (3) (D)	FASTENER AT (7) & (8) SILL PL TO RIM JOIST OR BLOCKING MIN PENETRATIONS SHOWN ARE INTO JOIST / OR BLOCK (E)	A35/LTP-4 AT RIM JOIST OR BLOCKING TO DBL TOP PLATE (F)	MIN WIDTH OF RIM JOIST / BLK'G
A	1	340 PLF	1/2"	2x6	2x6	5/8"Ø @ 32" OC AB W/ 1/4"x3"x3" WASHER PL	10d @ 6" OC EDGE NAILING (EN) 10d @ 12" OC FIELD NAILING (FN)	NAIL: 16d @ 4" OC LAG SCREWS: 1/4"Ø W/ 2" MIN PENET @ 8" SIMPSON SDS 1/4"x6" (2" MIN PENET) @ 12" OC	16" OC	3x
B	1	510 PLF	1/2"	3x6	3x6	5/8"Ø @ 16" OC AB W/ 1/4"x3"x3" WASHER PL	10d @ 4" OC EDGE NAILING (EN) 10d @ 12" OC FIELD NAILING (FN)	LAG SCREWS: 1/4"Ø W/ 2" MIN PENET @ 4" OC SDS 1/4"x6" (2" MIN PENET) @ 8" OC	12" OC	4x
C	1	665 PLF	1/2"	3x6	3x6	5/8"Ø @ 16" OC AB W/ 1/4"x3"x3" WASHER PL	10d @ 3" OC EDGE NAILING (EN) 10d @ 4" OC FIELD NAILING (FN)	LAG SCREWS: 1/4"Ø W/ 2" MIN PENET @ 3" OC SDS 1/4"x6" (2" MIN PENET) @ 6" OC	9" OC	4x
D	1	870 PLF	1/2"	3x6	3x6	5/8"Ø @ 16" OC AB W/ 1/4"x3"x3" WASHER PL	10d @ 2" OC EDGE NAILING (EN) 10d @ 12" OC FIELD NAILING (FN)	LAG SCREWS: 1/4"Ø W/ 2" MIN PENET @ 3" OC SDS 1/4"x6" (2" MIN PENET) @ 5" OC	BOTH SIDES @ 14" OC (STAG)	4x
E	2	1020 PLF	1/2"	3x6	3x6	5/8"Ø @ 16" OC AB W/ 1/4"x3"x3" WASHER PL	10d @ 4" OC EDGE NAILING (EN) 10d @ 12" OC FIELD NAILING (FN)	LAG SCREWS: 3/8"Ø W/ 3" MIN PENET @ 4" OC SDS 1/4"x6" (2" MIN PENET) @ 4" OC	BOTH SIDES @ 12" OC (STAG)	4x
F	2	1330 PLF	1/2"	3x6	3x6	3/4" Ø @ 16" OC AB W/ 5/16x3"x3" WASHER PL	10d @ 3" OC EDGE NAILING (EN) 10d @ 12" OC FIELD NAILING (FN)	LAG SCREWS: 3/8"Ø W/ 3" MIN PENET @ 2 1/2" OC SDS 1/4"x6" (2" MIN PENET) @ 3" OC	BOTH SIDES @ 9" OC (STAG)	4x

C) ALL SIMPSON SCREWS INSTALLED INTO 4x MEMBER SHALL BE STAGGERED AS SHOWN WHEN SPACED 6" OC & LESS.



- ALL LAG SCREWS SHALL BE FULL BODIED AND THE SCREW SHANK SHALL EXTENT MIN 3D INTO RIM JOIST
- ALL PLYWOOD EDGES SHALL BE BLOCKED WITH 3x MEMBERS, (USE 1.5x3.5 TIMBERSTAND PLYWOOD EDGE BLOCKING.)
- SEE (1) S039B FOR OPENINGS SMALLER THAN 2'-6"x4'-0".
- WHERE NAIL SPACING IS 3" OR LESS REFER TO DETAIL (2) S037
- SPECIAL INSPECTION IS REQUIRED FOR ALL SIMPSON STRONG WALLS & FOR ALL SHEAR WALLS.
- ALL FASTENERS APPLIED TO PRESSURE TREATED SILL SHALL BE HOT DIP GALVANIZED.
- A) MIN WASHER SIZE SHALL BE 3"x3". PLATE WASHER SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE BOTTOM PLATE ON THE SIDES RECEIVING PLYWOOD SHEATHING - ENLARGE WASHER SIZE AS REQUIRED AT DOUBLE SIDED SHEAR WALL SO WASHER EXTENDS TO 1/2" OF THE BOTTOM PLATE EDGE ON BOTH SIDES. THE HOLE IN THE PLATE WASHER SHALL BE PERMITTED TO BE DIAGONALLY SLOTTED WITH A WIDTH OF UP TO 3/16" LARGER THAN THE BOLT DIAMETER AND A SLOT LENGTH NOT TO EXCEED (1)-3/4". PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND THE NUT. SEE DETAIL (J)
- EMBED ANCHOR BOLTS A MIN 8" INTO PODIUM SLAB OR FOOTING. EXTEND ANCHOR BOLTS THRU CURBS OR STEM WALLS TO ACHIEVE THE EMBEDMENT INTO SLAB AND FOOTING. EMBEDMENT IS TO TOP OF WASHER.
- PROVIDE FULL DEPTH BLOCKING UNDER POST PER TYP DETAILS. (7) S032

- A) SOLID SAWN BLOCKING OR RIM NOT ALLOWED WITH PWJ OR ENGINEERED FRAMING.
- WHEN CMST STRAP IS NAILED TO THE TOP OR BOTTOM OF THE RIM JOIST, USE (1)-3" MIN WIDE MEMBER ((2)-1 3/4" MEMBERS NOT ALLOWED)
- PROVIDE WIDER RIM MEMBER WHEN REQUIRED BY DIAPHRAGM NAILING AND DETAILS.
- A) STAGGER SHEAR CONNECTOR WHEN THEY OCCUR ON BOTH SIDES OF RIM/BLKG.
- WHEN CONNECTOR IS APPLIED OVER SHEATHING, USE MIN 2 1/2" LONG NAILS.
- ORIENT LTP-4 WITH THE LONG SIDE PARALLEL TO THE WALL DOUBLE PLATE, AS SHOWN ON (C)
- FOR 2-SIDED SHEAR WALLS OFFSET PANEL JOINTS AT OPPOSING FACES OF THE STUDS SO JOINTS DO NOT OCCUR AT THE SAME STUD. SHOWN ON (M)



1 TYPICAL SHEAR WALL SCHEDULE AND ELEVATION

SCALE: NTS

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2
TEL 310 392 3995

SO
saiful-bouquet
structural engineers

726 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.saifulbouquet.com
Project #25534

TYPICAL WOOD DETAILS

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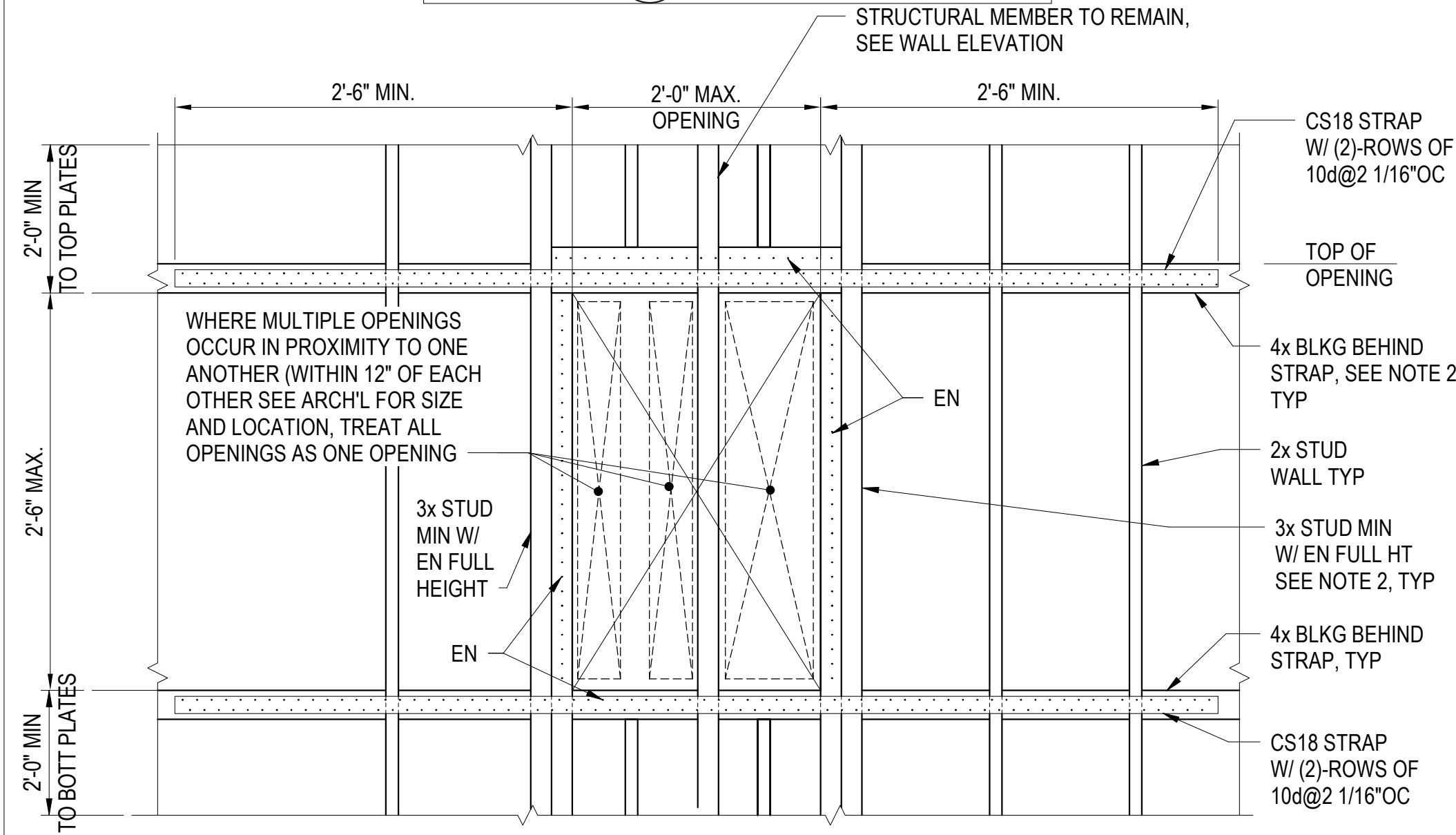
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Job No.: Project Number:

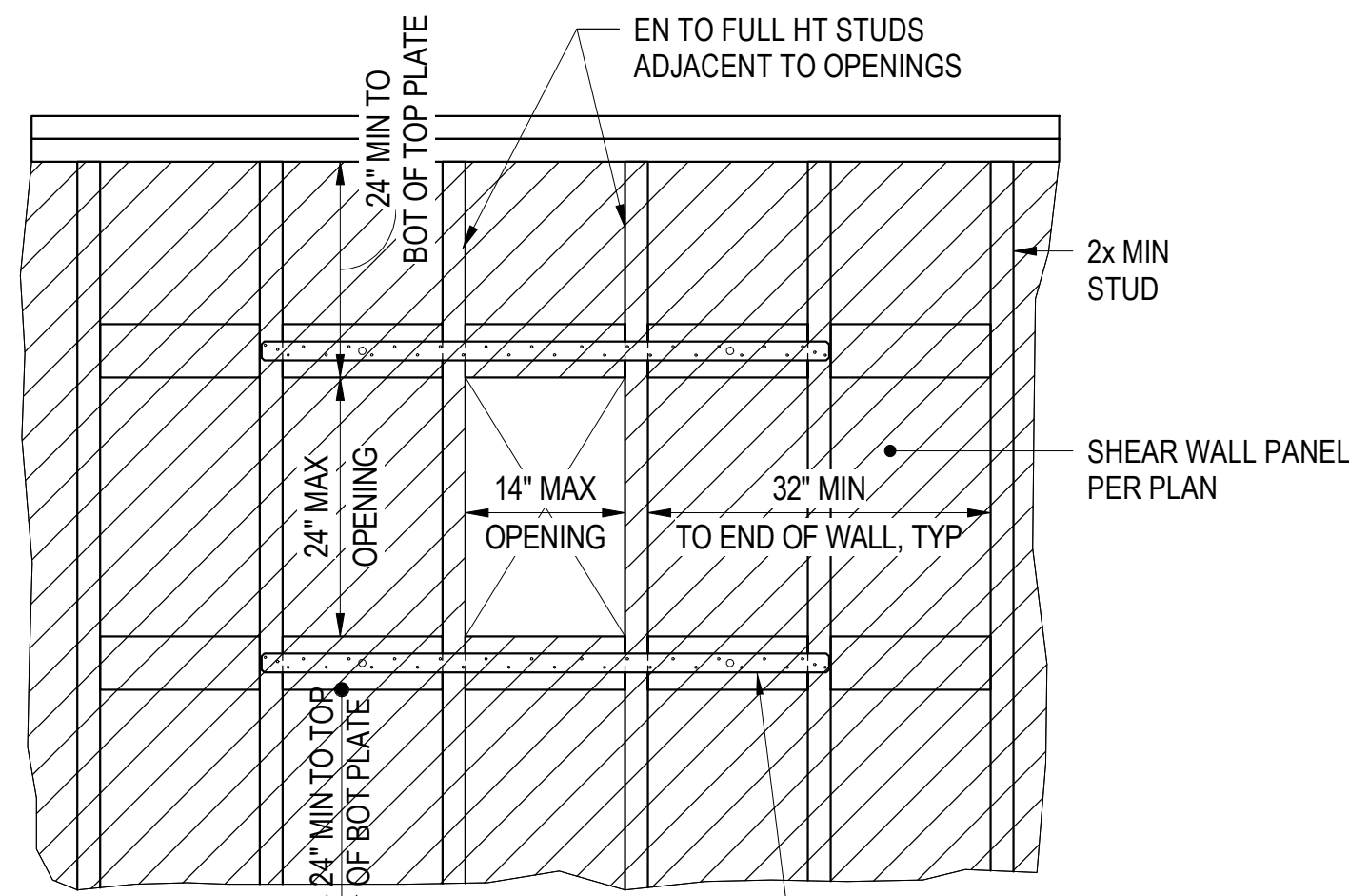
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ADDENDUM 5

PLAN CHECK SUBMITTAL - October 31 2025

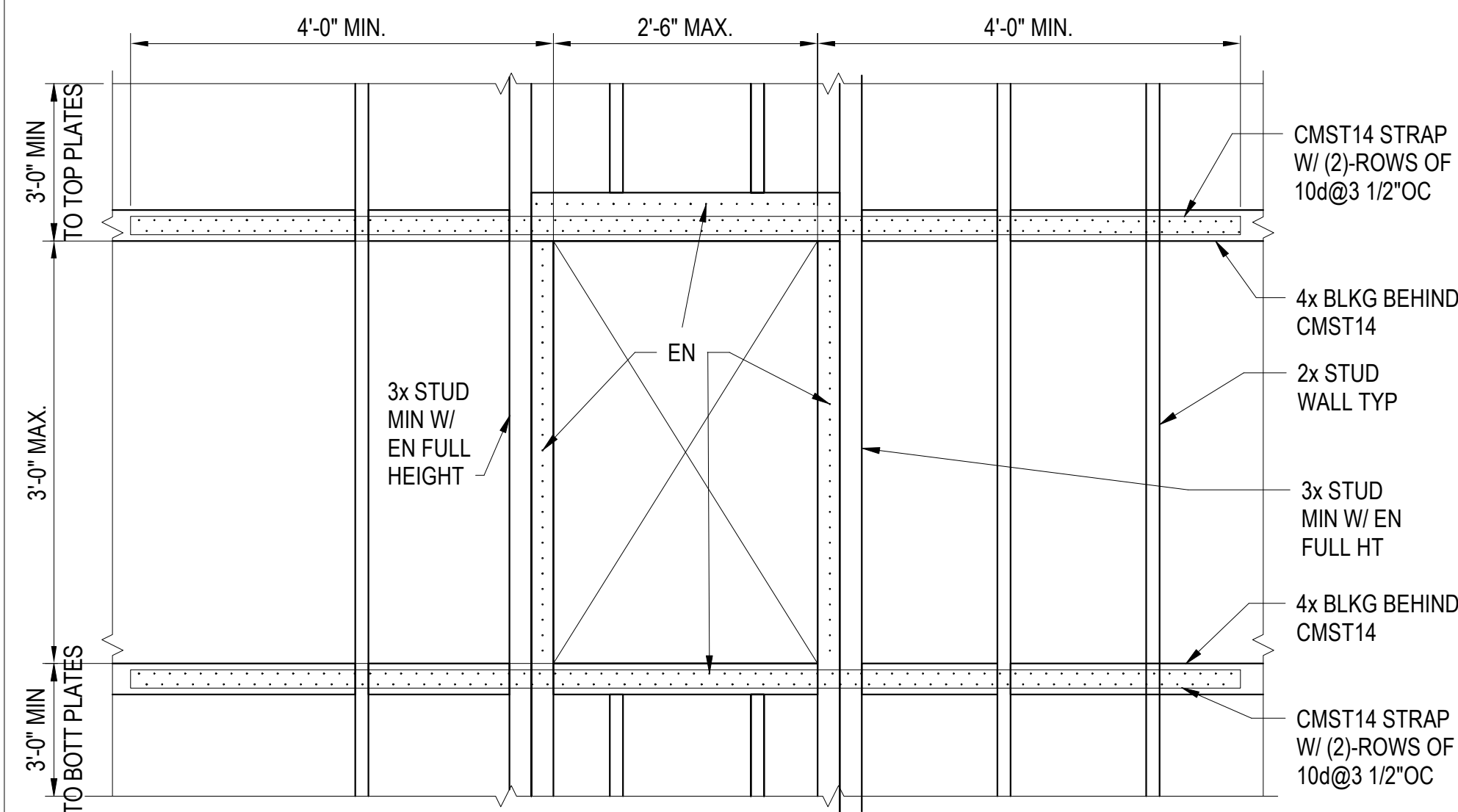


TYPICAL MULTIPLE OPENINGS TREAT AS ONE OPENING (D)



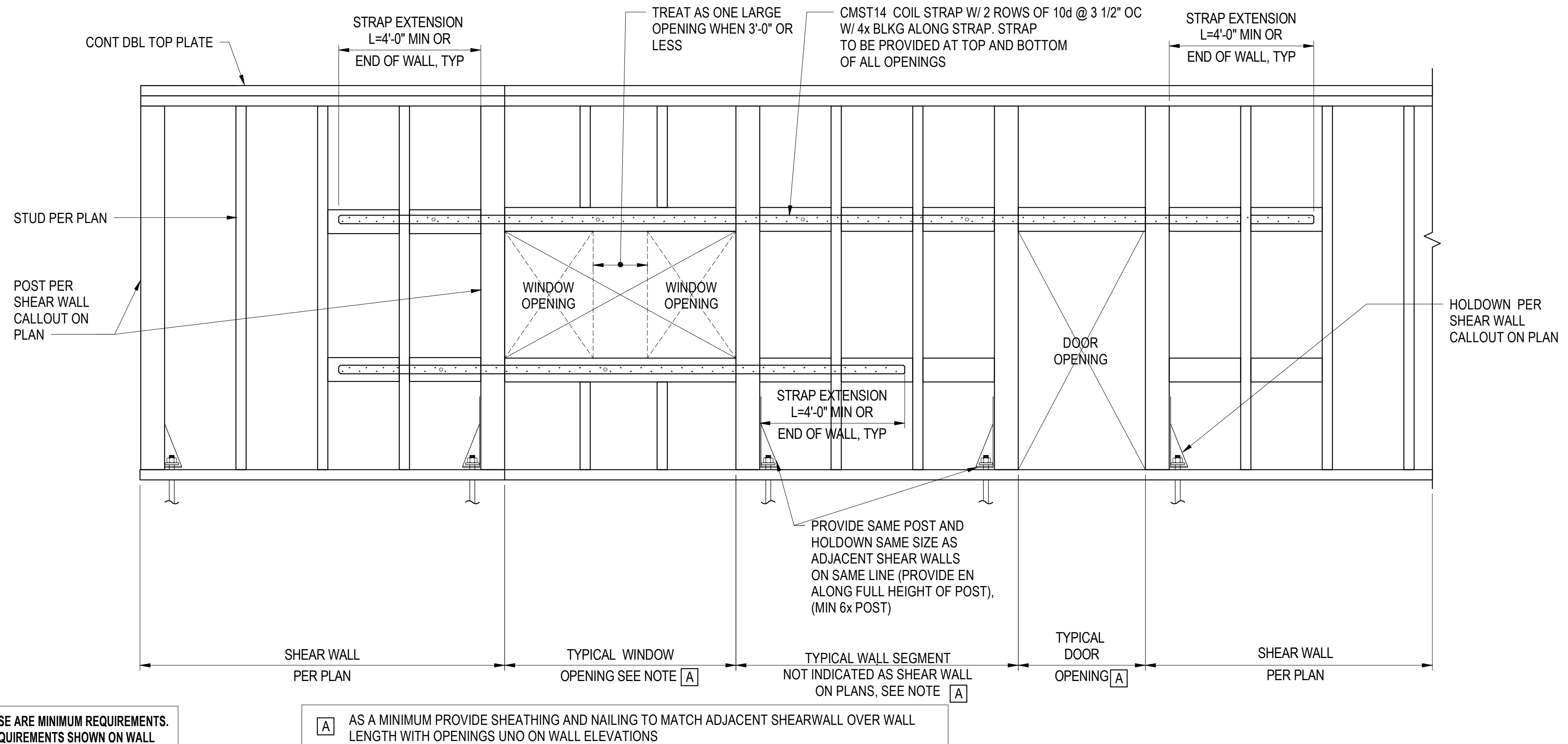
- NOTE:**
1. MIN DISTANCE BETWEEN ADJACENT OPENINGS SHALL BE 48"
 2. NO MORE THAN 1 OPENING IS ALLOWED IN A WALL 6'-0" LONG OR SHORTER
 3. NO MORE THAN 2 OPENING ARE ALLOWED IN ANY SHEAR WALL

TYPICAL ELEVATION AT SMALL OPENING IN SHEAR WALL C

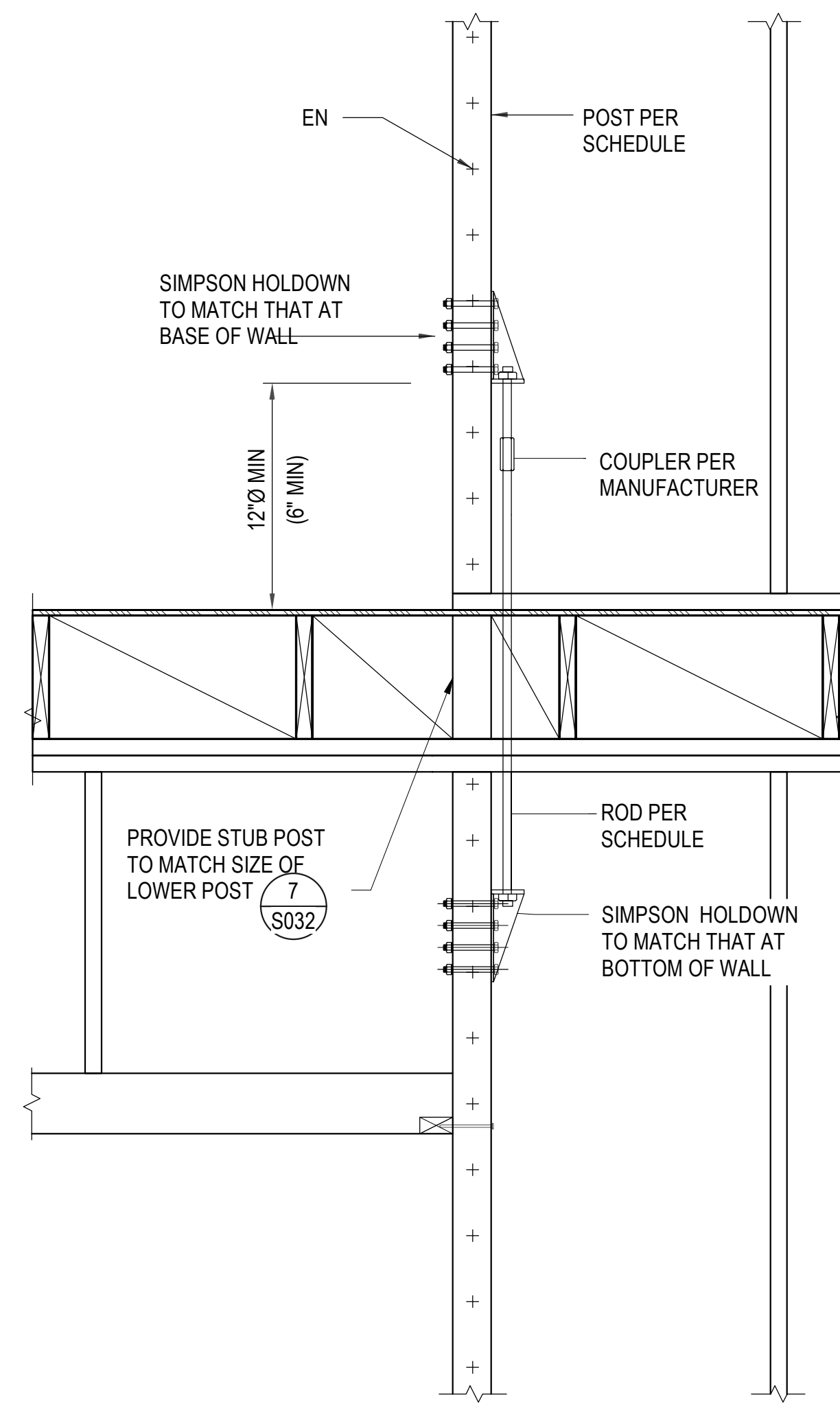


TYPICAL FRAMING AT SHEAR WALL OPENINGS

NOTE: THESE ARE MINIMUM REQUIREMENTS. WHERE REQUIREMENTS SHOWN ON WALL ELEVATIONS ARE MORE STRINGENT USE THOSE REQUIREMENTS AT THE SPECIFIC LOCATIONS SHOWN



TYPICAL ADDITIONAL REQUIREMENTS FOR CONT WALLS THAT INCLUDE SHEARWALS AS A SEGMENT OF THE WALL



TYPICAL HOLDOWN DETAIL AT CONTINUOUS POST

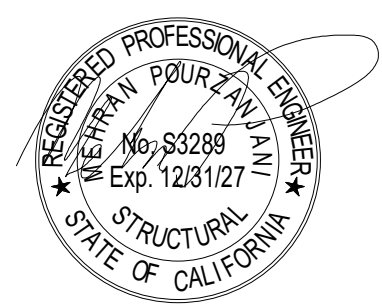
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2

TYPICAL WOOD SHEAR WALL DETAILS

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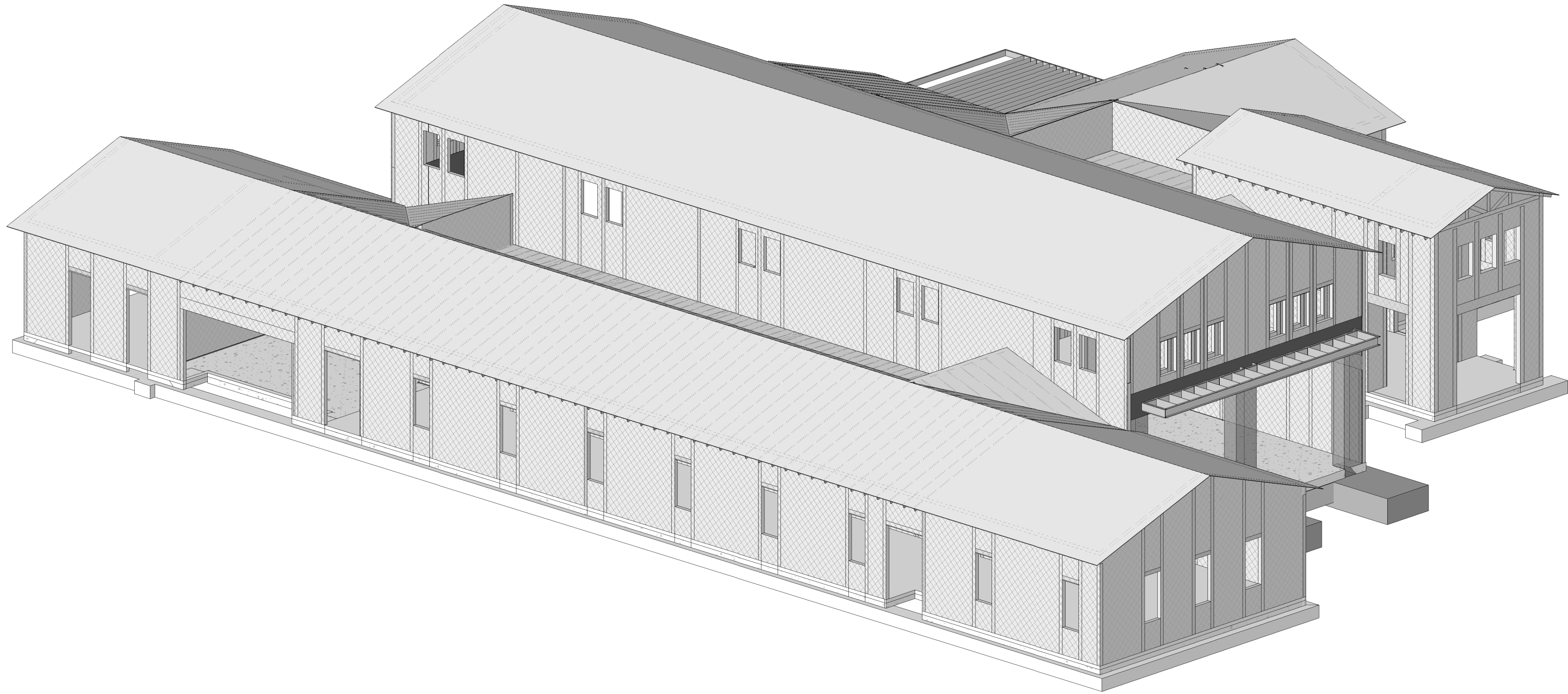
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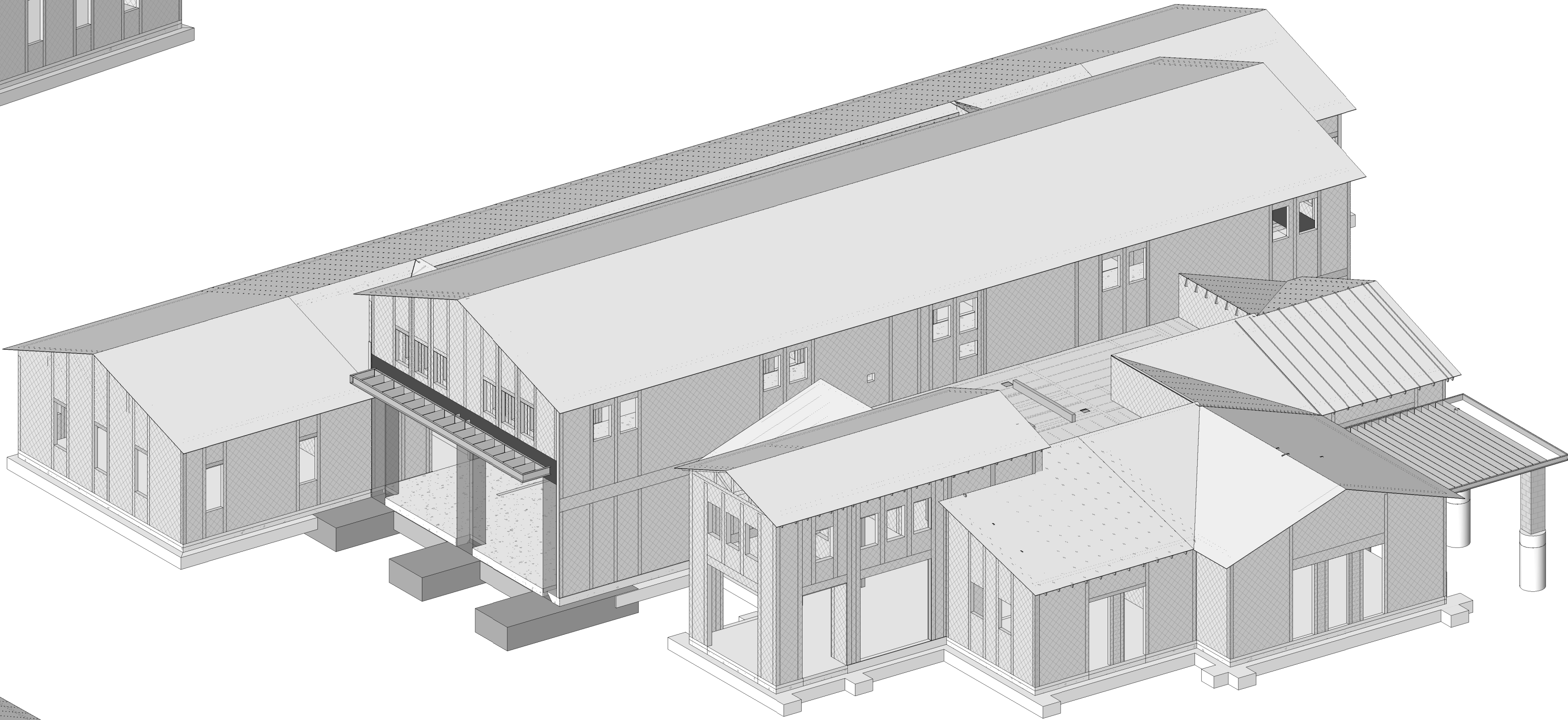
WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

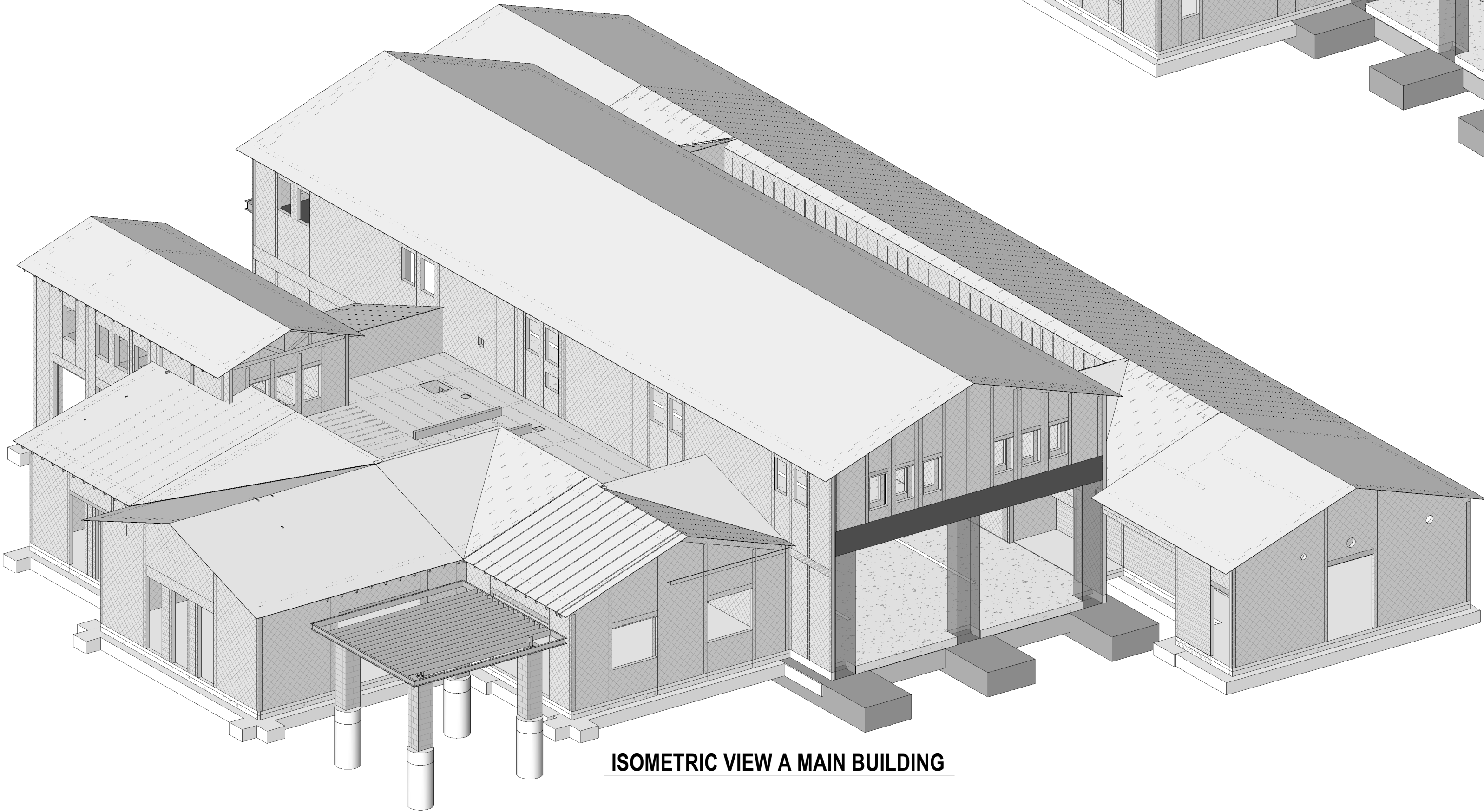
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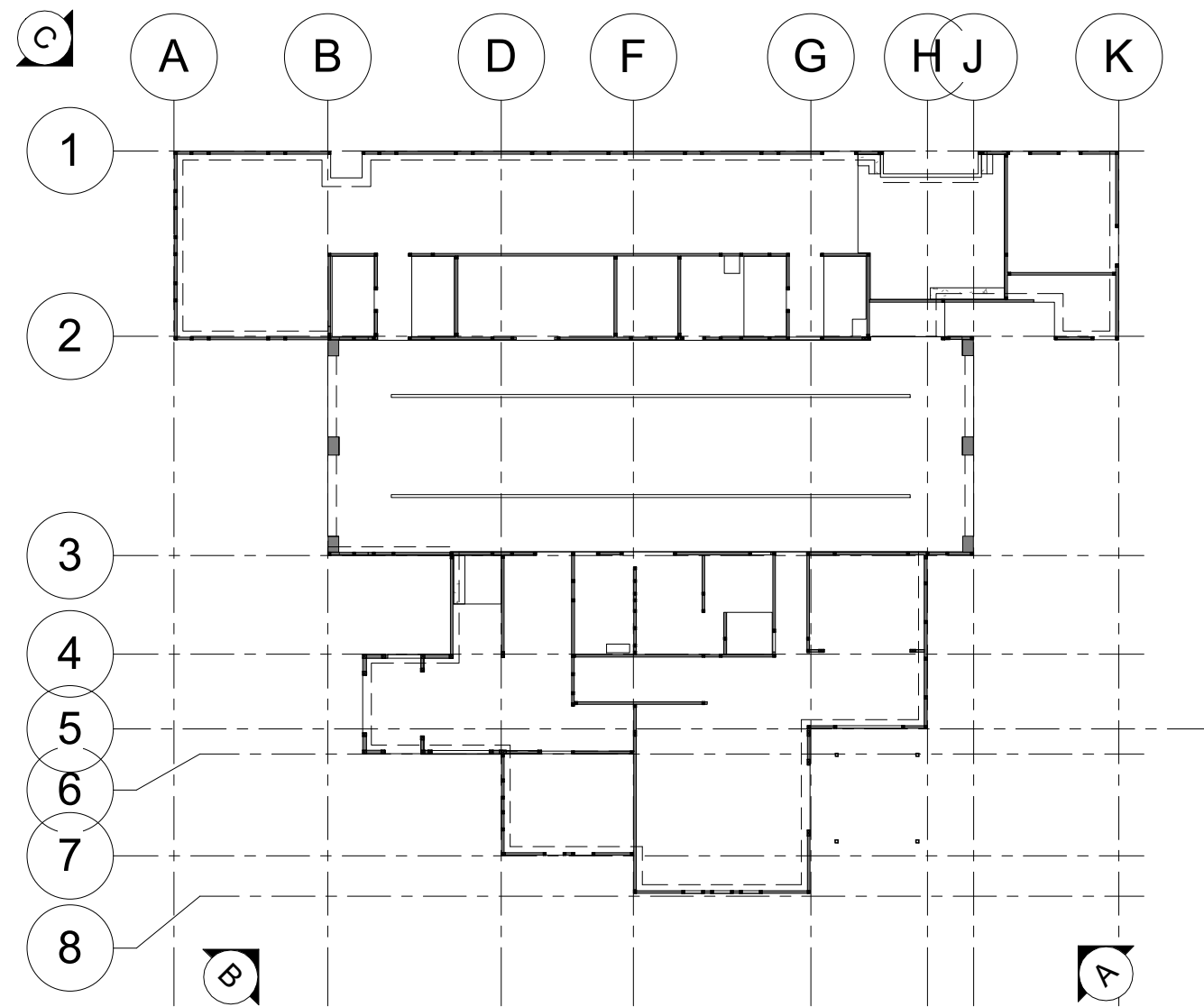
ISOMETRIC VIEW C MAIN BUILDING



ISOMETRIC VIEW B MAIN BUILDING



ISOMETRIC VIEW A MAIN BUILDING



KEY PLAN - ISOMETRIC VIEWS

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

so
saiful-bouquet
structural engineers

725 S. Figueroa St.,
3rd Floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

ISOMETRIC VIEWS

FIRE STATION 46
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VALENCIA, CALIFORNIA



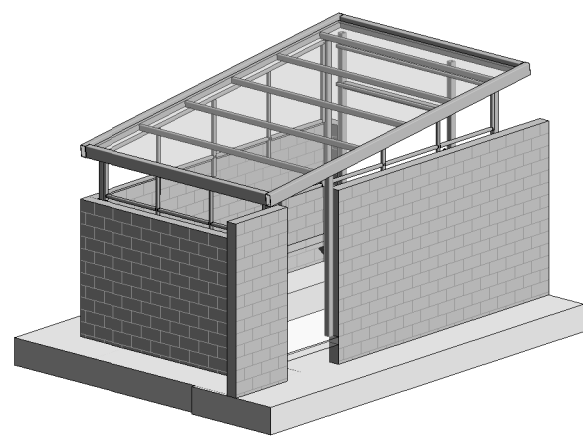
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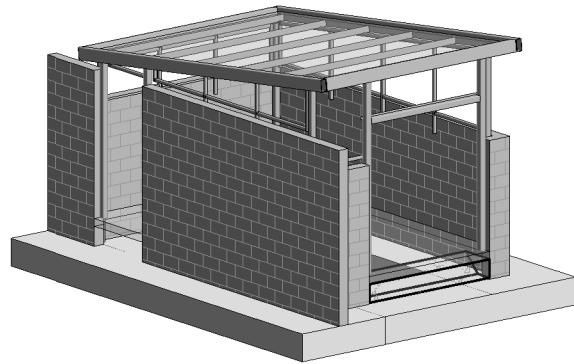
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ADDENDUM 5

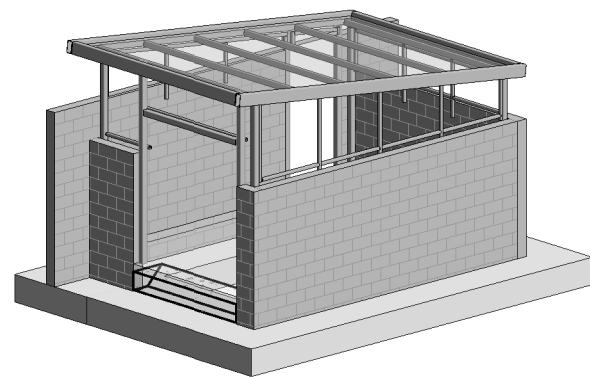
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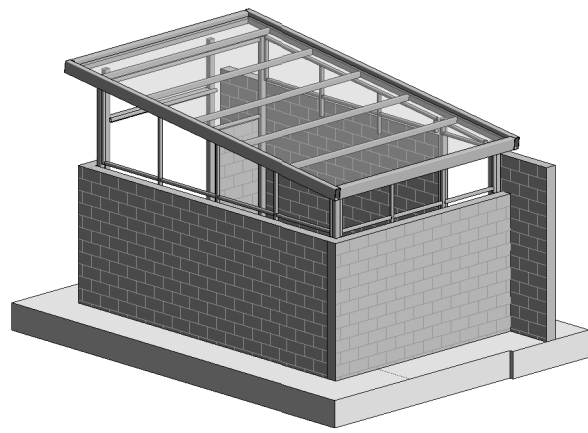
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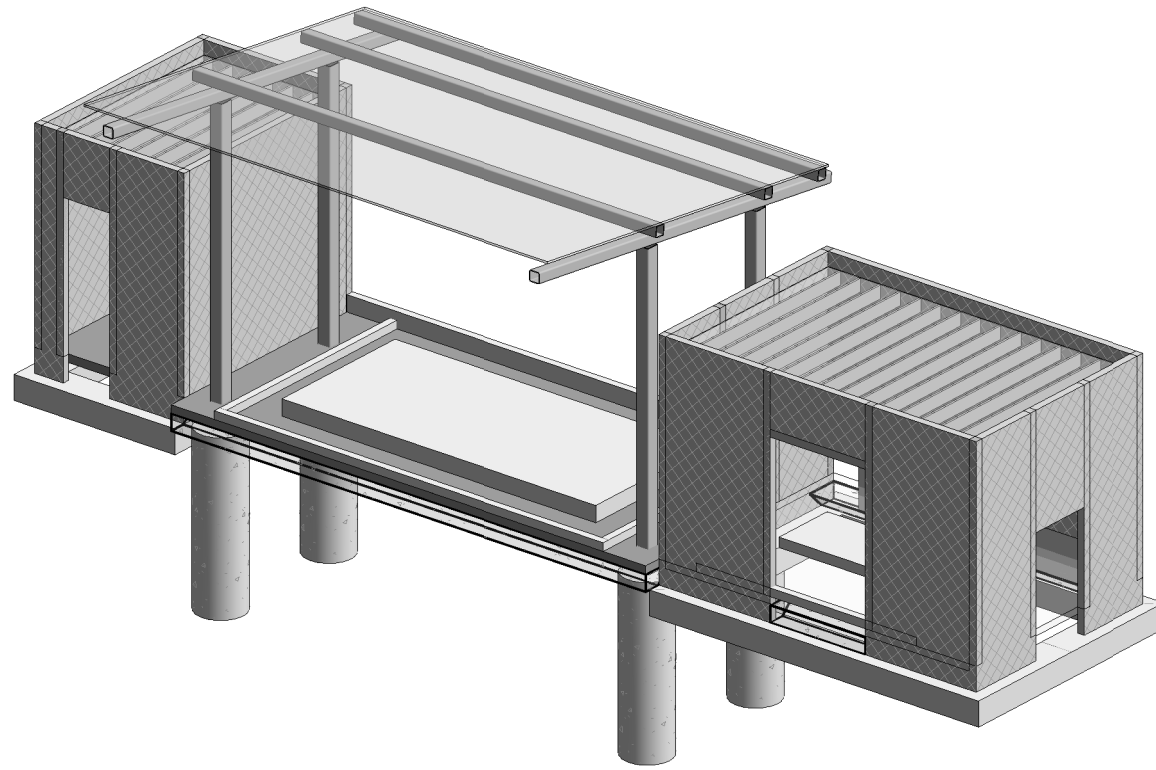
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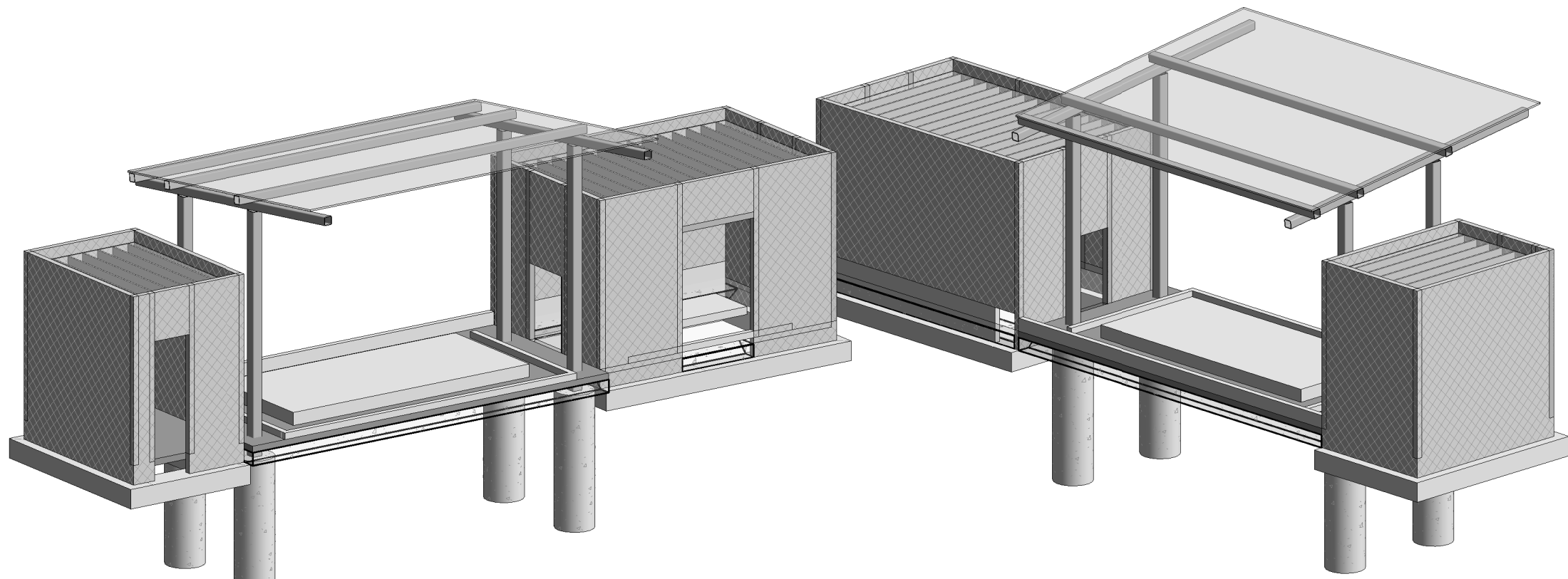
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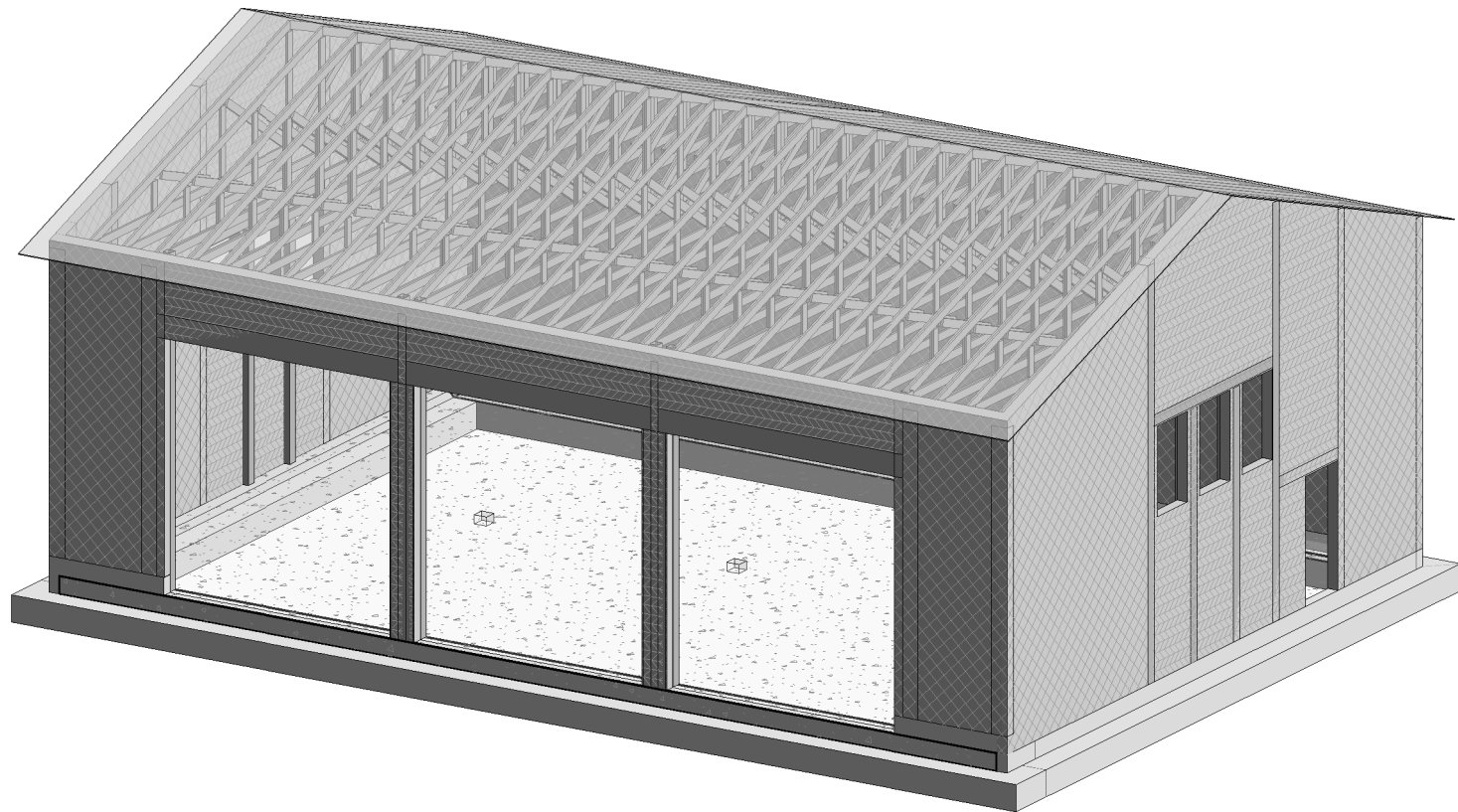
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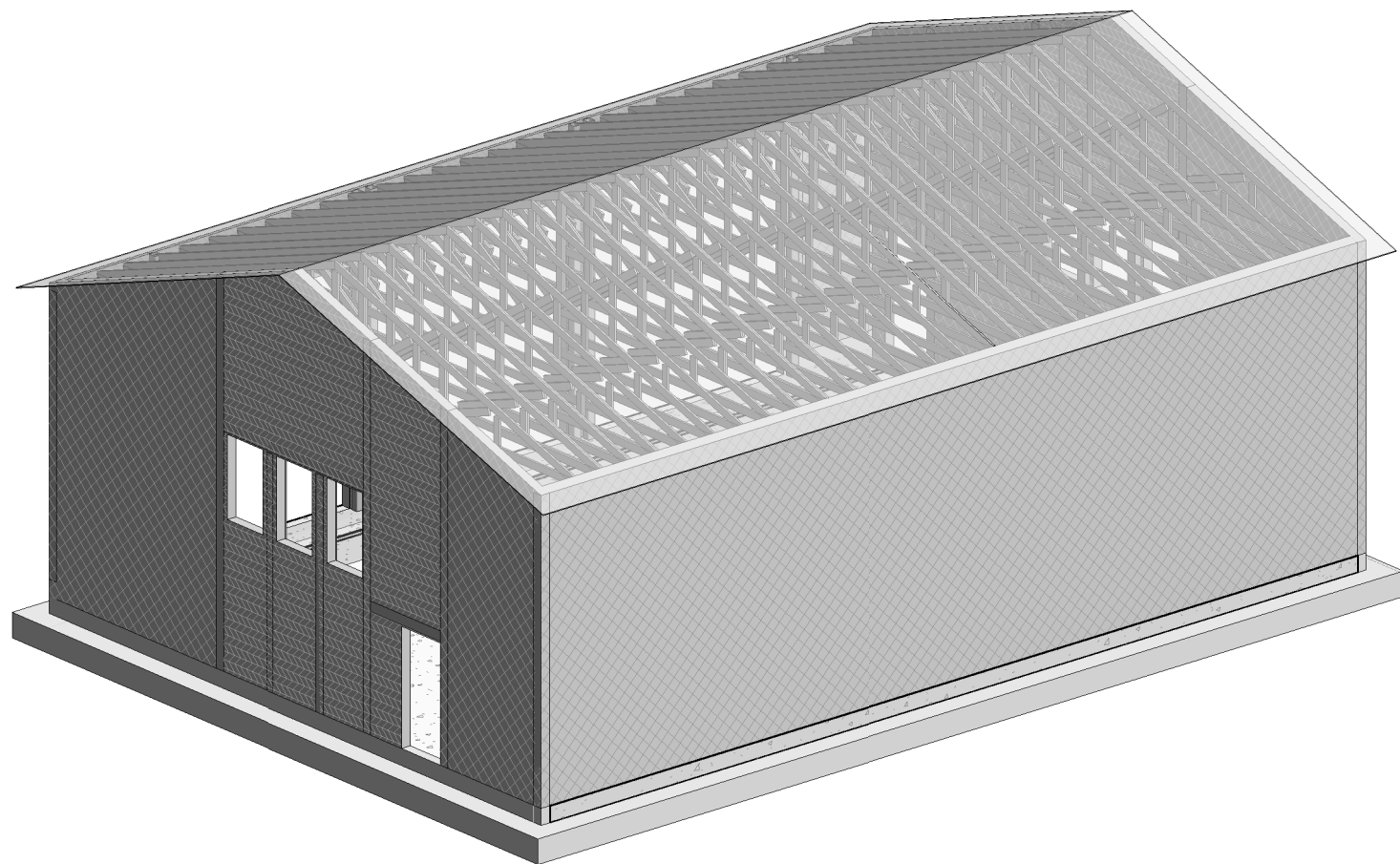
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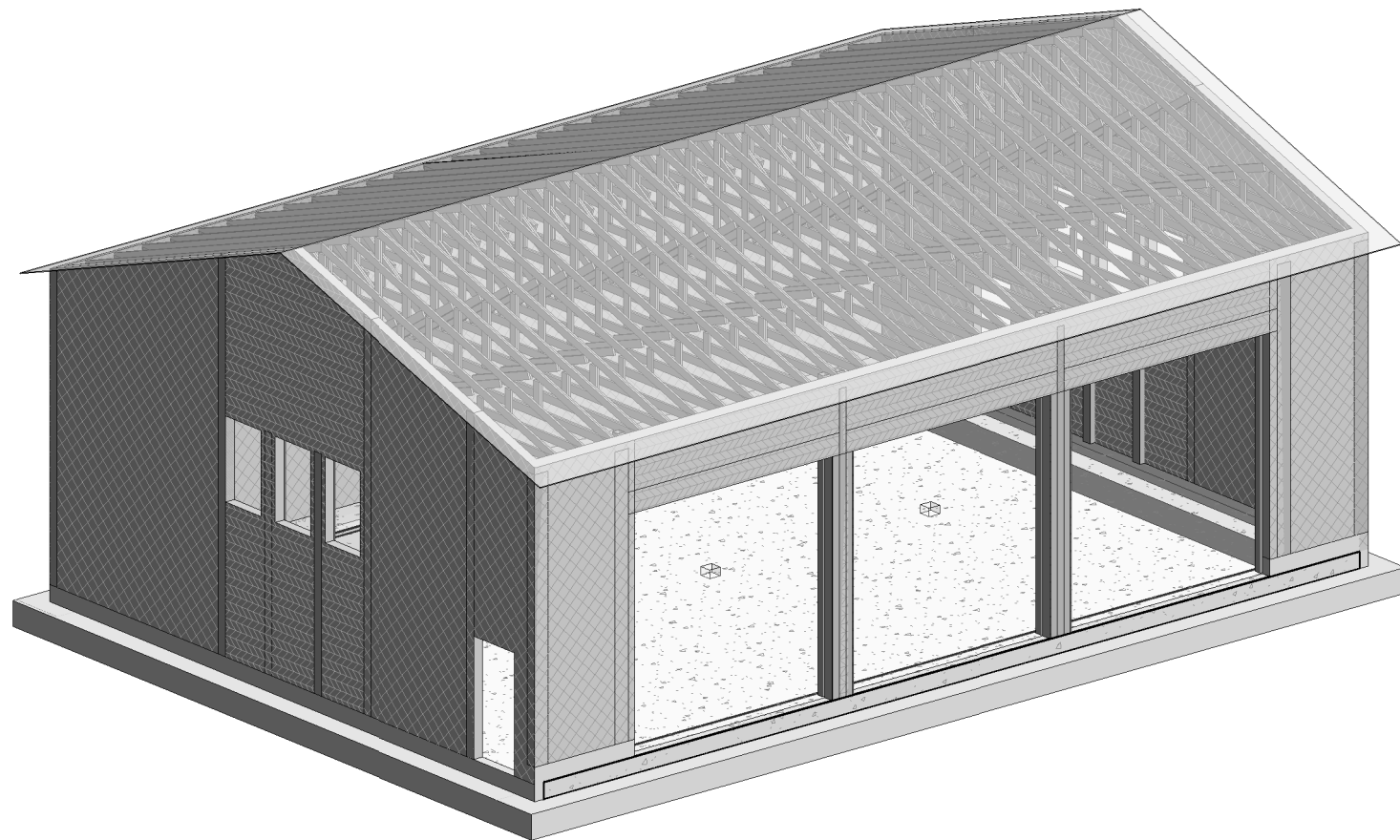
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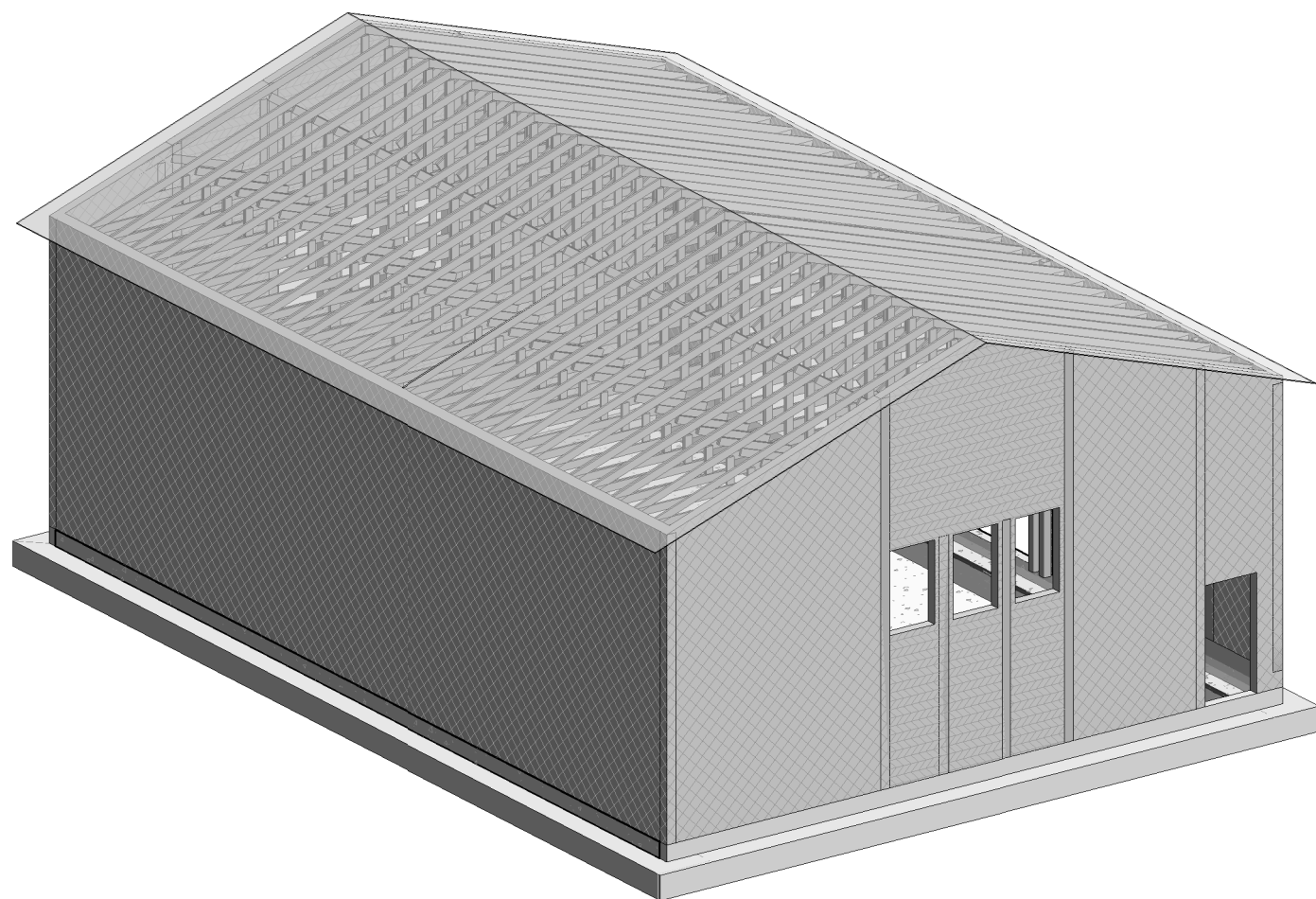
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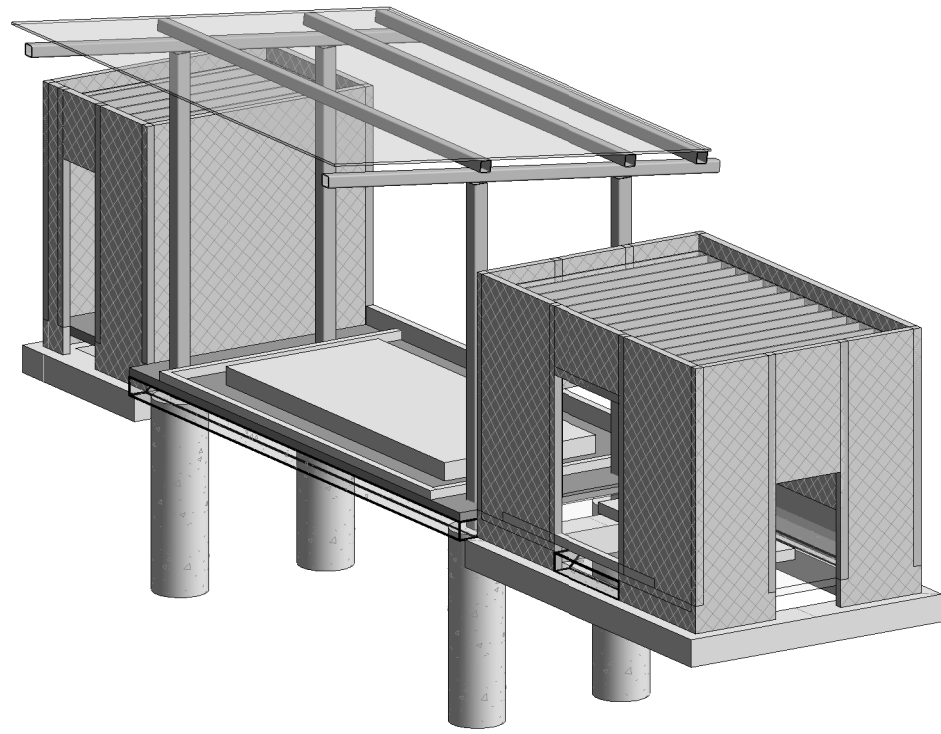
ISOMETRIC RESERVE APPARATUS BLDG - C



ISOMETRIC RESERVE APPARATUS BLDG - A



ISOMETRIC RESERVE APPARATUS BLDG - D



ISOMETRIC FUEL AREA BUILDING - A

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

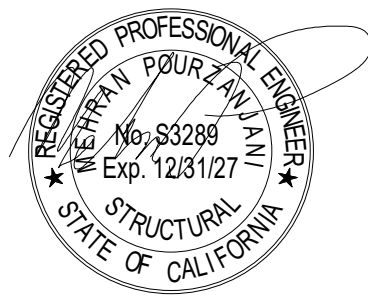
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structural engineers

725 S. Figueroa St.,
3rd Floor
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ISOMETRIC VIEWS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

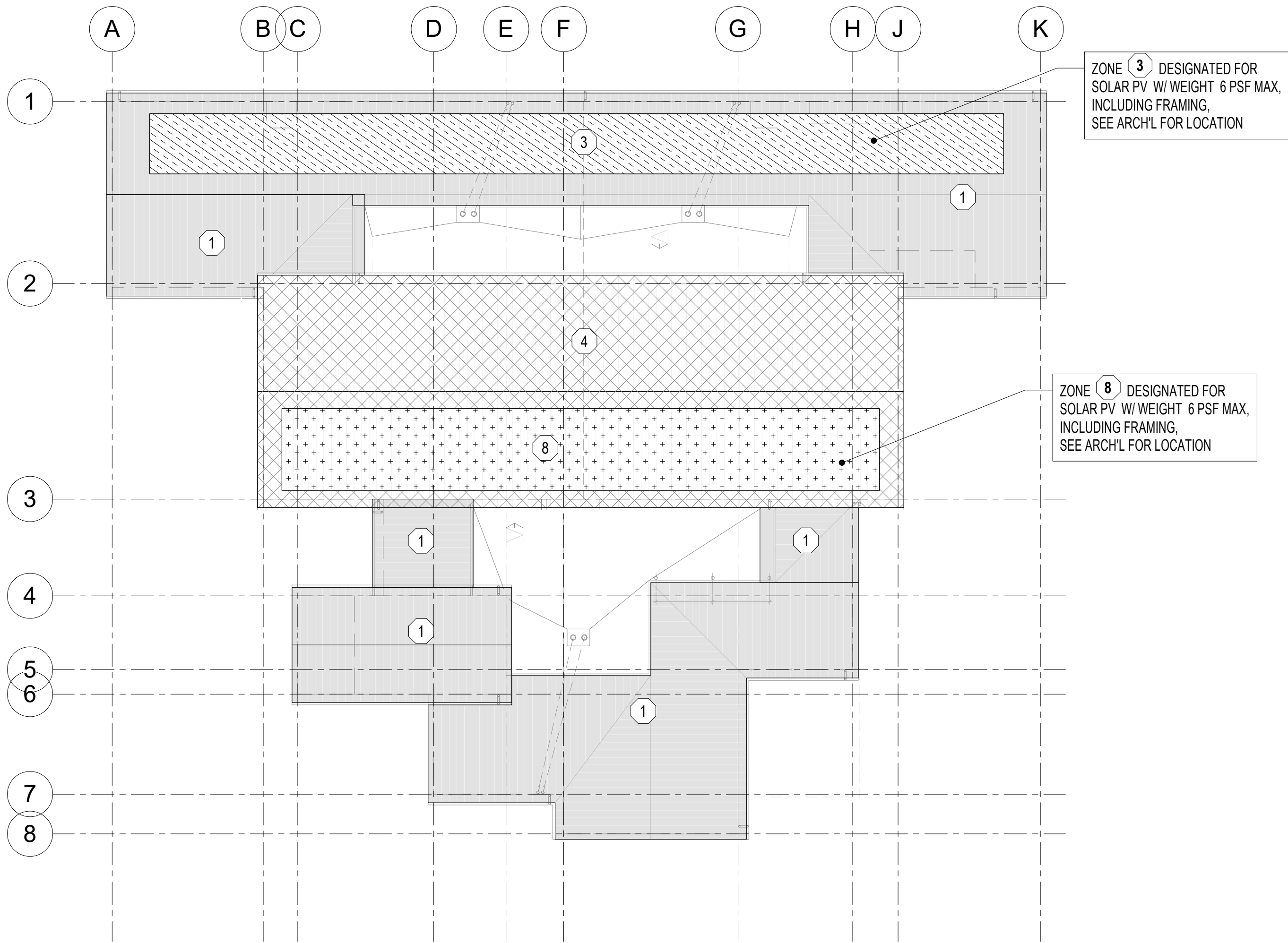


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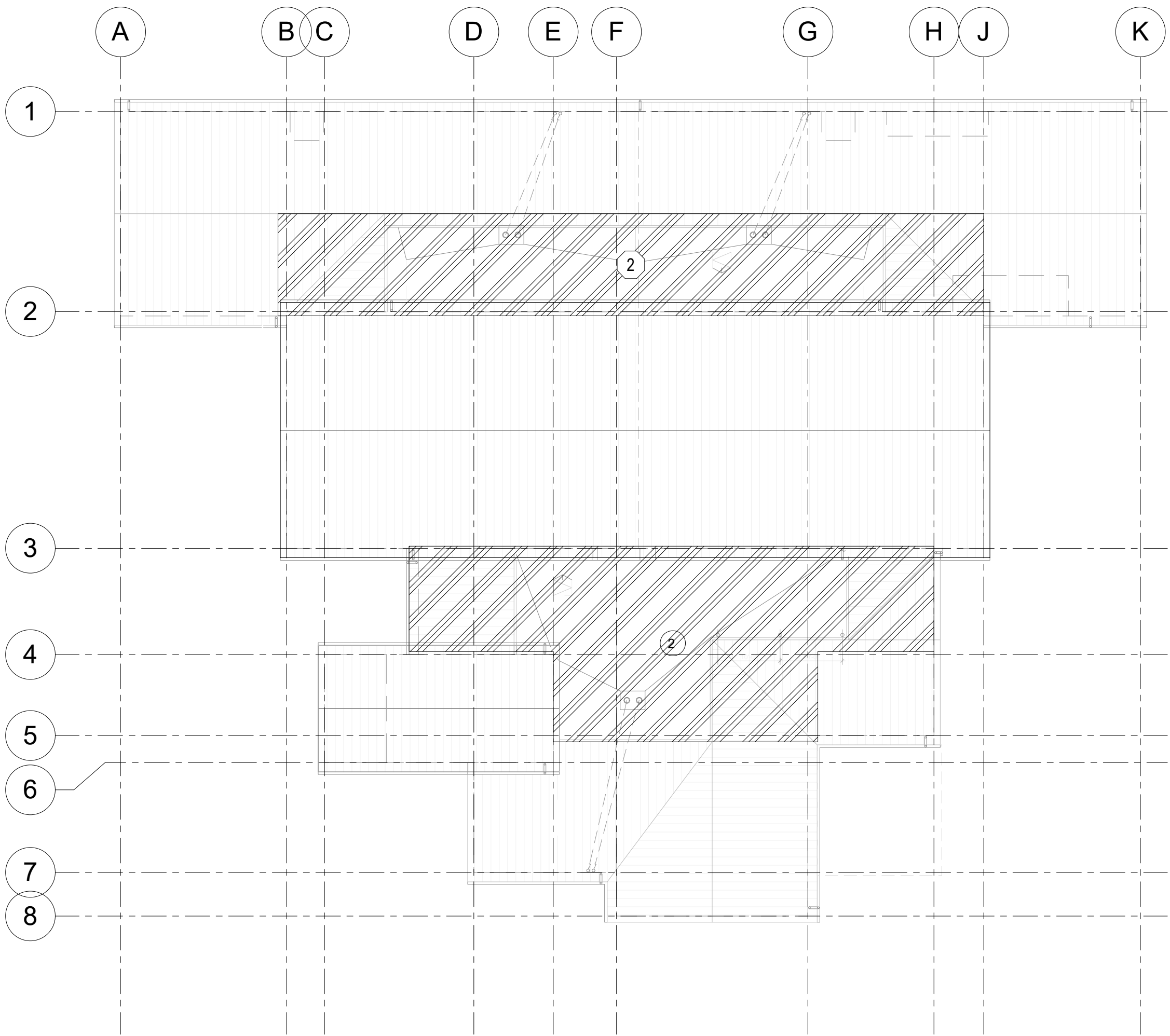
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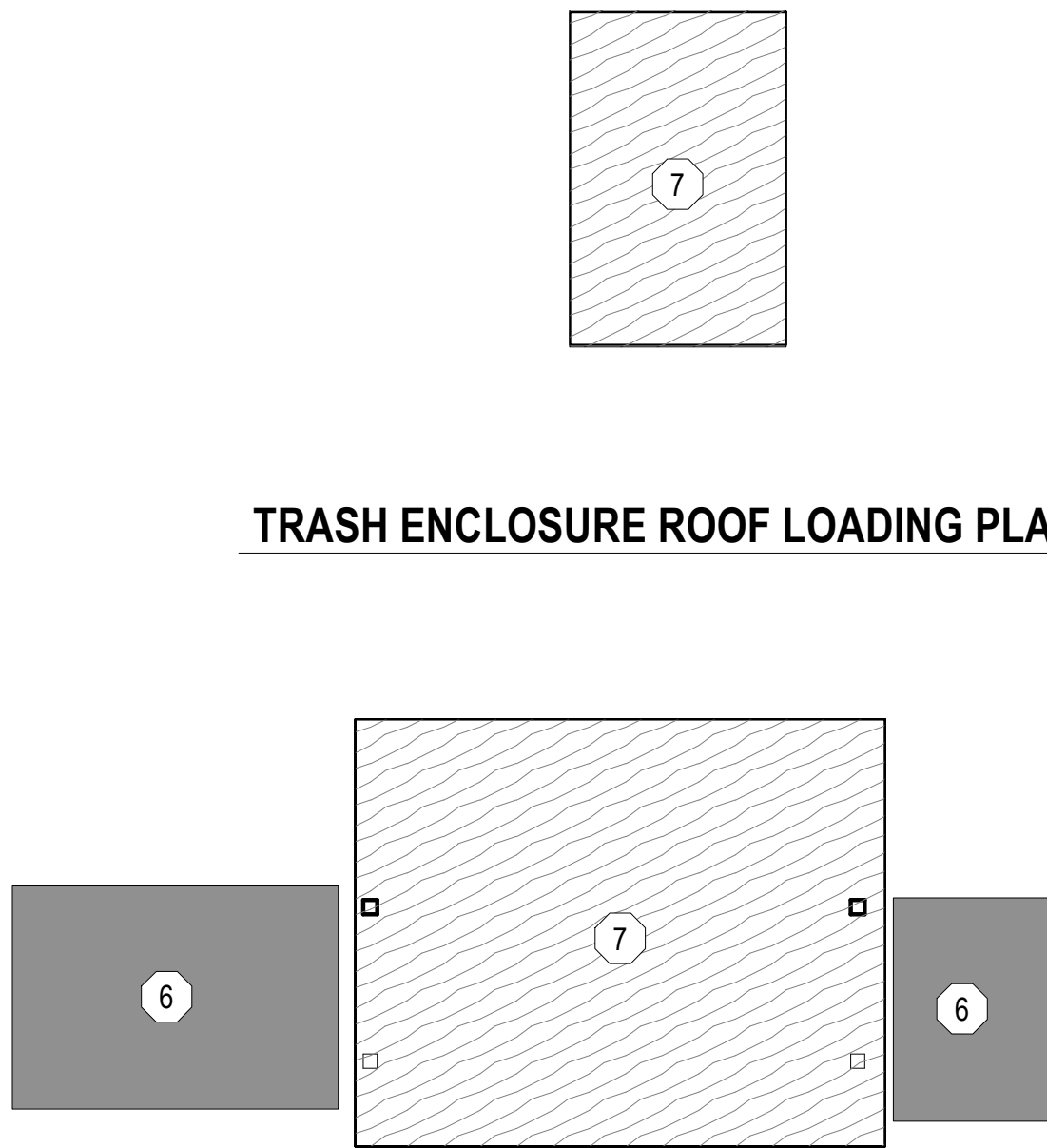
APPENDIX 5



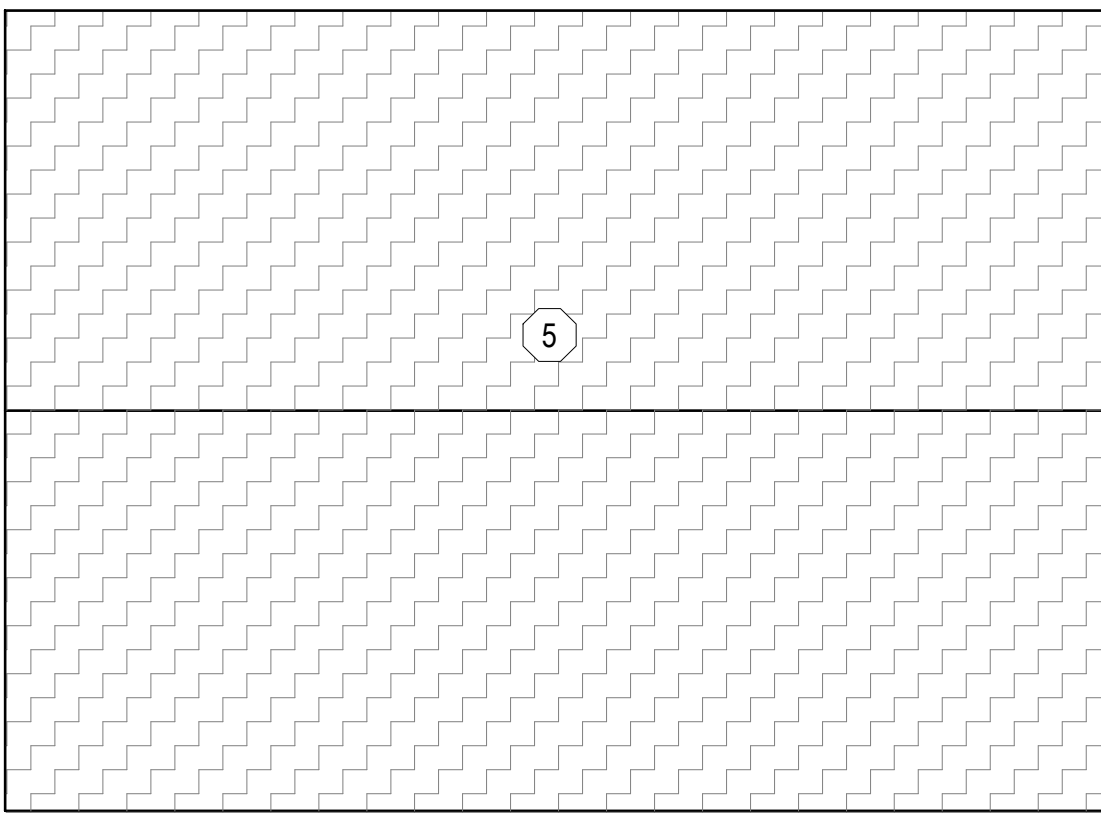
SLOPED ROOF AT MAIN BUILDING LOADING PLAN



FLAT ROOF AT MAIN BUILDING LOADING PLAN



TRASH ENCLOSURE ROOF LOADING PLAN



ROOF AT RESERVE APPARATUS BUILDING LOADING PLAN

LOADING DIAGRAM TABLE				
MARK	AREA DESCRIPTION	LIVE LOAD	SUPER IMPOSED DEAD LOAD ⁽¹⁾	LOCATION
1	TYPICAL SLOPED ROOF WITHOUT PV PANEL	20 PSF REDUCIBLE	18 PSF	MAIN BUILDING
2	TYPICAL FLAT ROOF AT MECH'L WELL	50 PSF NON-REDUCIBLE (AREA AROUND FOOT PRINT OF MECHANICAL UNITS)	16 PSF + WEIGHT OF MECHANICAL UNITS	MAIN BUILDING
3	TYPICAL SLOPED ROOF WITH PV PANEL	20 PSF REDUCIBLE	18 PSF + 6 PSF (MAX WEIGHT OF PV PANELS INCLUDING FRAMING)	MAIN BUILDING
4	APPARATUS ROOF WITHOUT PV PANEL	20 PSF REDUCIBLE	21 PSF	MAIN BUILDING
5	RESERVE APPARATUS ROOF	20 PSF REDUCIBLE	19 PSF	RESERVE APPARATUS
6	FLAT ROOF	20 PSF REDUCIBLE	17.5 PSF	FUEL AREA
7	FUEL CANOPY ROOF	20 PSF REDUCIBLE	16 PSF	FUEL AREA
8	APPARATUS ROOF WITH PV PANEL	20 PSF REDUCIBLE	21 PSF + 6PSF (MAX WEIGHT OF PV PANELS INCLUDING FRAMING)	MAIN BUILDING

NOTES:
1. SUPER IMPOSED DEAD LOAD IS IN ADDITION TO THE SELF WT OF ROOF DECK.

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90230

TEL 310 392 3995

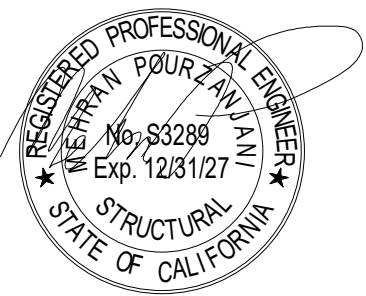
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structural engineers

725 S. Figueroa St.,
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LOADING PLANS

FIRE STATION 46

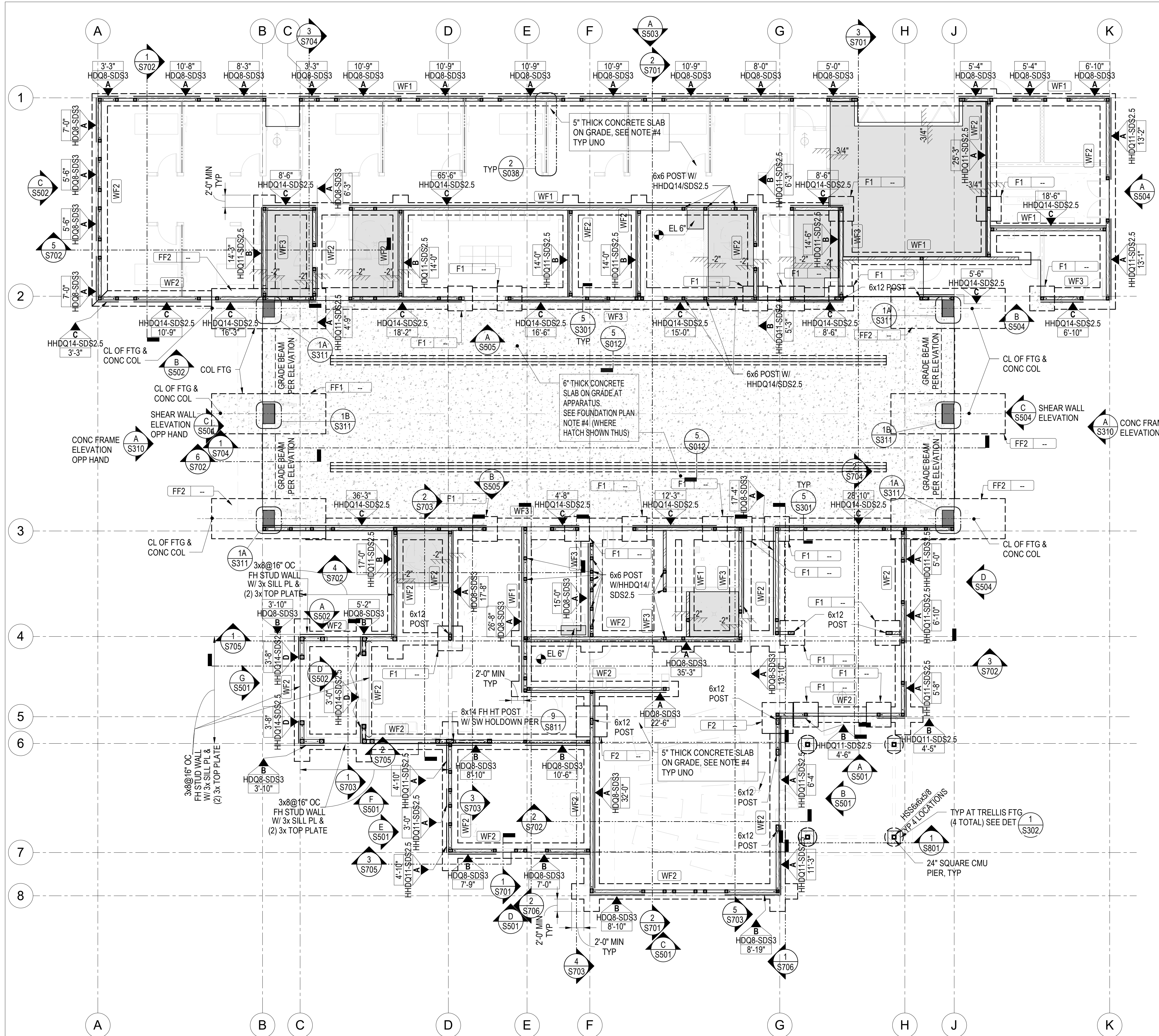
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
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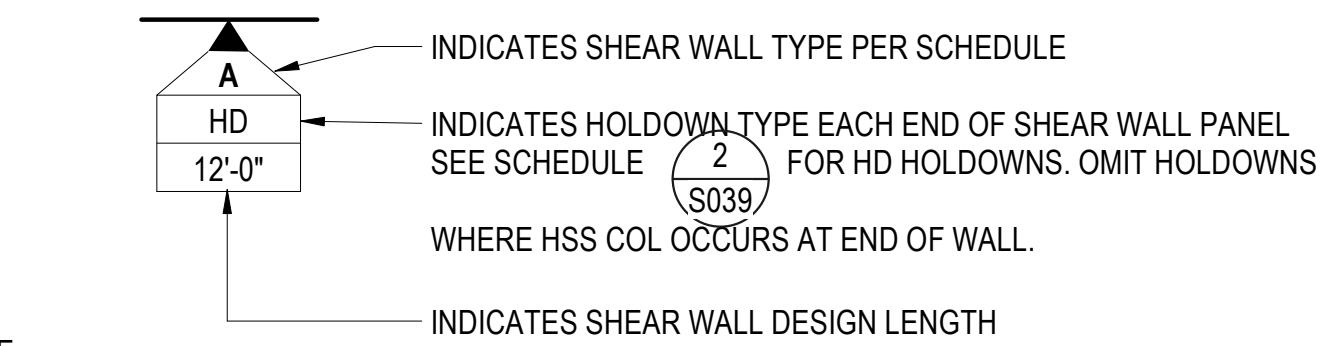
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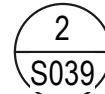
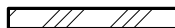


S110
APPENDIX 5



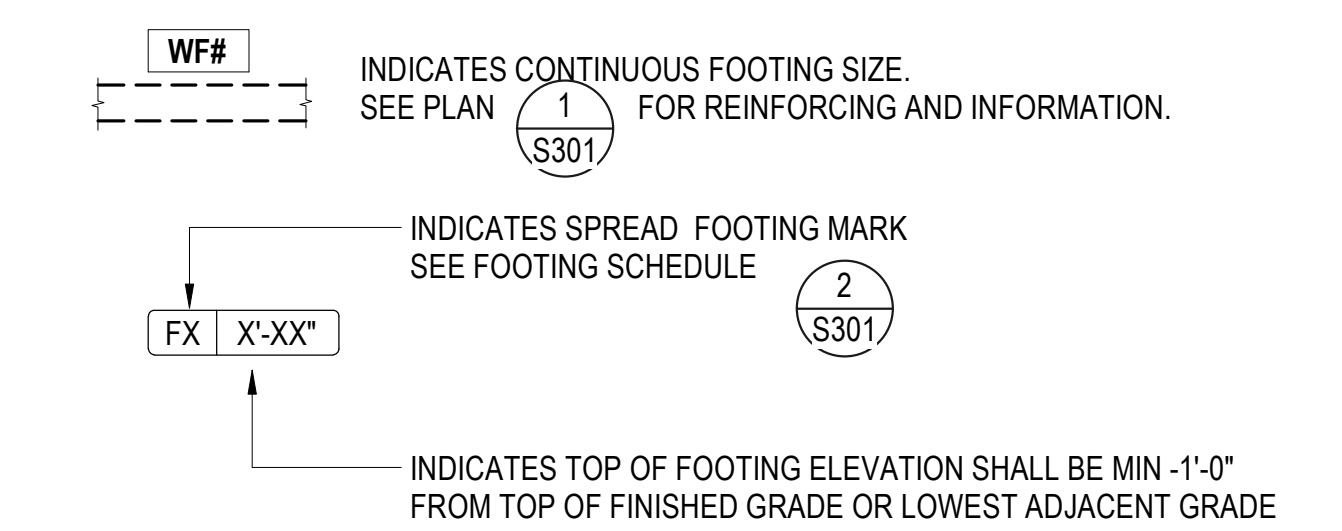
FOUNDATION PLAN NOTES

1. FOR GENERAL NOTES SEE S0.0 SERIES AND TYPICAL DETAILS SEE S0 SERIES SHEETS.
 2. VERIFY ALL DIMENSIONS PRIOR TO START OF WORK. SEE ARCHITECTURAL DRAWINGS FOR REMAINDER OF DIMENSIONS NOT SHOWN ON THIS PLAN.
 3. SEE ARCHITECTURAL DRAWINGS FOR FLOOR ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, SLAB EDGE LOCATIONS, ETC., AND FOR WALL OVERALL DIMENSIONS, LOCATIONS OF OPENINGS, ETC., NOT INDICATED ON STRUCTURAL DRAWINGS.
 4. TYPICAL SLAB ON GRADE SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:
 - A. 5" CONCRETE SLAB W/ #4@18"OC EACH WAY AT CENTER OF SLAB OVER
 - B. SLAB SHALL BE PLACED OVER 20 MIL VAPOR BARRIER UNDERLAID AND OVER 4 INCHES OF CLEAN COARSE SAND AND OVERLAID BY 2 INCHES OF CLEAN COARSE SAND, SEE SOIL REPORT.
-  INDICATES 6" THICK CONCRETE SLAB ON GRADE
- A. PROVIDE #4@12" BW AT MID-HEIGHT OF SLAB.
 - B. SLAB SHALL BE PLACED OVER 20 MIL VAPOR BARRIER UNDERLAID AND OVERLAID BY 4 INCHES OF CLEAN COARSE SAND AND OVERLAID BY 2 INCHES OF CLEAN COARSE SAND, SEE SOIL REPORT
5. PROVIDE CONSTRUCTION JOINTS AND WEAKENED PLANE JOINTS (INDICATED AS CJ AND PW, RESPECTIVELY) IN SLAB ON GRADE AS SHOWN ON DETAIL (LOCATE PER ARCHITECTURAL DRAWINGS).
- 
6. INDICATES SHEAR WALL MARK FOR PLYWOOD SHEAR WALLS:












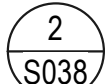


7.  INDICATES HOLDOWN LOCATION. PROVIDE POST FOR HOLDOWN PER 
8. TYPICAL WOOD STUDS SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE ON PLAN
- SHEAR WALL: 2x6@16" AT ALL EXTERIOR WALL, UNO
2x6 @16" AT INTERIOR WALL UNO
- EXTERIOR WALL
(EXCLUDING THE APPARATUS
RESERVE BUILDING): 2x6@16" UNO
- EXTERIOR WALL FOR
APPARATUS RESERVE
BUILDING: 2x8" @16" OC
- INTERIOR BEARING WALL: 2x6@16" AND LARGER AS INDICATED BY ARCH
- INTERIOR NON-BEARING WALL: 2x4 @16" FOR HEIGHTS UP TO 10' OR 2x6@16" AND
LARGER AS INDICATED BY ARCH
9.  INDICATES INTERIOR BEARING WALL. SEE DETAILS ON SHEETS **S021 & S022**
10. ALL NAILS ARE COMMON NAILS.
11. FOR CONTINUOUS FOOTING SEE PLAN AND DETAILS ON SHEET **S301**
12. WOOD WALL INTERSECTION CONNECTION SEE DETAILS  AT NON-SHEAR WALL &
 AT SHEAR WALL.

FOUNDATION PLAN LEGEND



ALL HARDWARE IS BY "SIMPSON" TYPICAL OR APPROVED EQUAL.

- ☒ OR **POST**
- INDICATES POST SHALL BE AS FOLLOWS:
 4x4 WOOD POST AT 4" INTERIOR STUD WALLS, UNO ON PLAN
 6x6 WOOD POST AT 6" EXTERIOR & INTERIOR STUD WALLS, UNO ON PLAN
 WHERE WOOD POST ARE ALSO USED AS SHEAR WALL HOLDOWN
 SEE DETAIL  &  AND USE THE LARGER POST
-  INDICATES SLOPED SLAB ON GRADE REFER TO ARCH DWGS
-  INDICATES STEP FOOTING, SEE DETAIL 
-  INDICATES CMU WALL, SEE DETAILS ON SHEETS **S021 & S021**
-  INDICATES RAISED OR DEPRESSED SLAB, SEE DETAILS COORDINATE W/ MECH/ELECT DWGS/ARCH A1.0 OR  
- GB** INDICATES GRADE BEAM, SEE DETAIL 
-  INDICATES PARTIAL HIGH DORM ROOM WALL PER SEE ARCH'L FOR LOCATION 

SHEAR WALL ELEVATION ON GRID 6

SCALE: 1/8" = 1'-0"

1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995



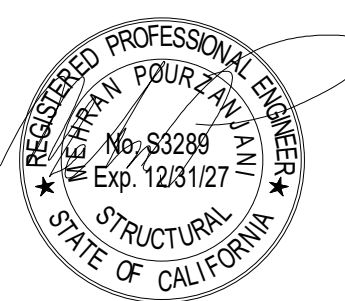
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FOUNDATION PLAN

FIRE STATION 46

MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
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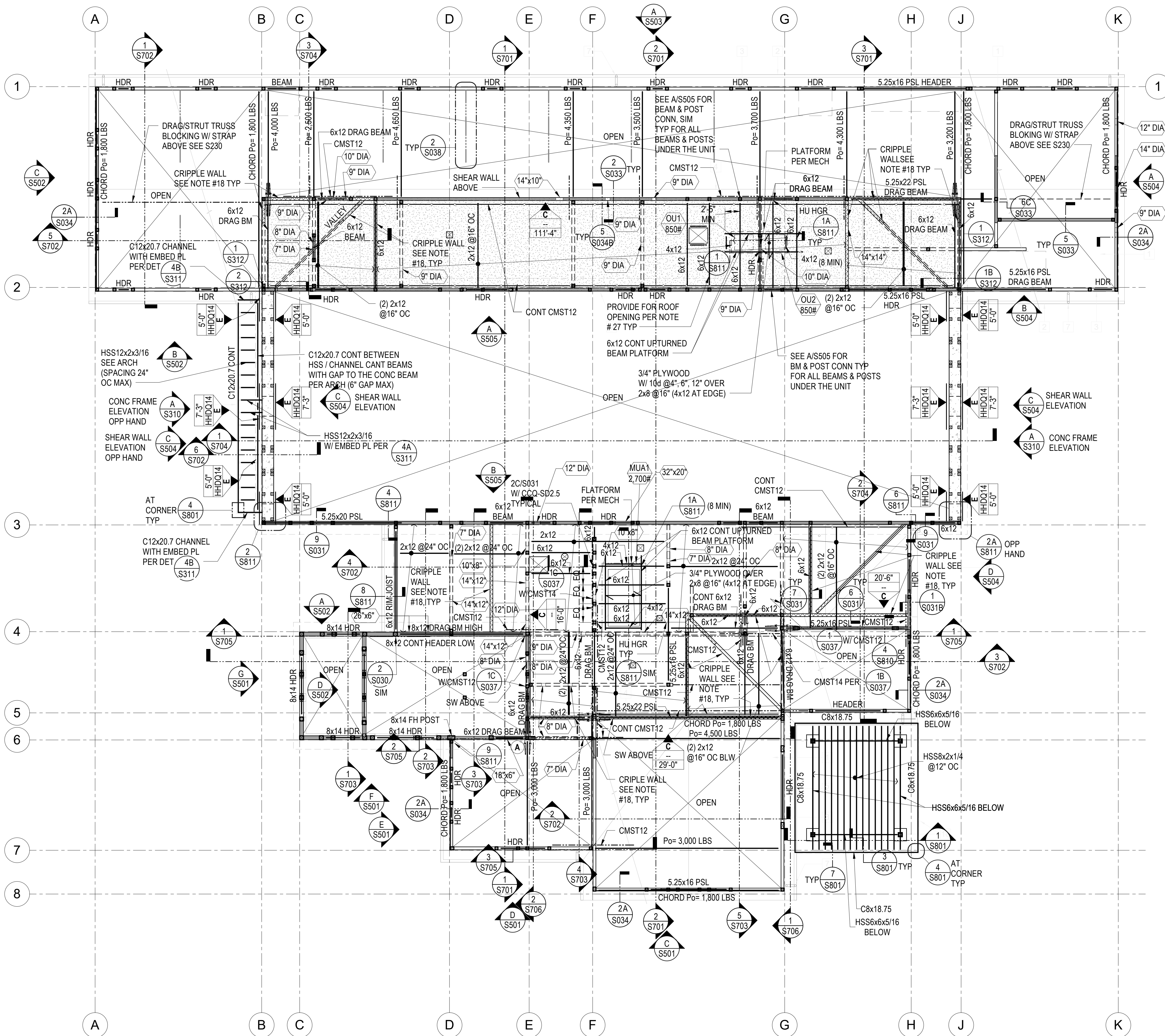
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WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALED DIMENSIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THIS OFFICE MUST BE ADVISED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS SHOWN BY THESE DRAWINGS. SHOP DETAILS SHALL BE SUBMITTED TO THIS OFFICE FOR APPROVAL, BEFORE PROCEEDING WITH FABRICATION.

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Job. No.	Project Number

S210



FLAT ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

1

FRAMING PLAN NOTES

- FOR GENERAL NOTES SEE S0.0 SERIES AND TYPICAL DETAILS SEE S0 SERIES SHEETS.
- VERIFY ALL DIMENSIONS PRIOR TO START OF WORK. SEE ARCHITECTURAL DRAWINGS FOR REMAINDER OF DIMENSIONS NOT SHOWN ON THIS PLAN.
- WHERE A SHEAR WALL IS SHOWN AS PORTION OF A WALL, THE REMAINDER OF THE WALL (INCLUDING ABOVE AND BELOW OPENINGS, PARAPETS, ETC) SHALL BE SHEATHED WITH THE SAME SHEATHING AND NAILING SCHEDULE ALONG THE ENTIRE WALL. IN ADDITION, CORNERS OF ALL OPENINGS IN SHEAR WALL SHALL BE STRAPPED.
- ALL SHEAR WALLS AND SHEAR TRANSFER NAILS SHALL BE COMMON WIRE NAILS. SINKER AND BOX NAILS ARE NOT PERMITTED.
- HOLDOWN CONNECTORS SHALL BE TIGHTENED JUST PRIOR TO COVERING THE WALL FRAMING.
- INDICATES BEARING AND/OR SHEAR WALL BELOW.
- ROOF DIAPHRAGM SHEATHING AND NAILING SHALL BE PER SHEET S240
- TYPICAL WOOD STUD WALL: SEE NOTE #8 ON SHEET S210
- ALL HARDWARE ARE BY "SIMPSON" TYPICAL OR APPROVED EQUAL.
- ALL SAWN WOOD FRAMING EXPOSED TO WEATHER SHALL BE P.T.D.F.
- HDR INDICATES HEADER, FOR HEADER BEAM SIZE PER SCHEDULE SEE DETAIL S030. UNO. IF THE OPENING WIDTH IS 14 FT OR GREATER SEE PLAN & ELEVATION. FOR BEAM SIZES, TREAT ALL HEADERS FOR BEARING WALL, SEE DETAIL S030.
- INDICATES ROOF JOIST
- ROOF DIAPHRAGM NAILING TO BE INSPECTED BEFORE COVERING, FACE GRAIN OF PLYWOOD SHALL BE PERPENDICULAR TO SUPPORT.
- ALL NAILS ARE COMMON NAILS, UNO.
- CMST INDICATES SIMPSON CMST14 STRAP, UNO

HD INDICATES SHEAR WALL TYPE PER SCHEDULE

HD INDICATES HOLDOWN TYPE EACH END OF SHEAR WALL PANEL SEE SCHEDULE S030 FOR HD HOLDOWNS S030

INDICATES SHEAR WALL DESIGN LENGTH
- SEE MECH DWGS FOR EQUIPMENT LOCATIONS
A. MOUNTED ON FLAT ROOFS, SEE DETAILS 1 & 5 ON S036 FOR TYPICAL MECHANICAL UNIT PLATFORM.
B. HANG MEP EQUIPMENTS SEE DETAIL S036 FOR STRUCTURAL SUPPORTING MEMBERS.
- INDICATES 2x6 STUD WALL @16"OC (CRIPPLE WALL) WITH PLYWOOD TYPE C PER SCHEDULE S031 MIN SEE DETAILS S031 & S037 FOR S030

BOTTOM CONNECTION. SEE DETAIL S031 FOR TOP CONNECTION
- INDICATES AXIAL SEISMIC DRAG STRUT LOAD (SERVICE) IN LBS. CONTRACTOR SHALL DESIGN TRUSSES TO RESIST THESE LOADS IN COMBINATION WITH GRAVITY LOADS IN ACCORDANCE WITH APPLICABLE BUILDING CODE AND ALL LISTED CRITERIA. WHERE REQUIRED BY ANALYSIS, PROVIDE DOUBLE OR MORE TRUSSES (PROVIDE MIN DOUBLE TRUSSES), SEE S034 UNO
- AC # INDICATES MECHANICAL UNIT TYPE

INDICATES MECHANICAL UNIT OPERATING WT IN LBS
- T1 INDICATES ROOF WOOD TRUSSES AER DESIGN-BUILT. REFER TO S0.38 SERIES FOR TRUSS PROFILE AND GENERAL NOTES FOR DESIGN CRITERIA, TYP.
- SOLAR PANEL, FRAMING & THEIR CONNECTIONS TO STRUCTURAL MEMBERS BY OTHERS
- INDICATES DRAG/STRUT STRAP AT DRAG BEAM / TRUSS OR BLOCKS AND EXTENT PER PLAN, S037 AT BEAMS & S034 AT TRUSSES
- INDICATES CHORD STRAP. PROVIDE MIN 4x4 BLOCKING UNDER STRAP FULL LENGTH OF THE STRAP CMST14 UNO
- STRAP NAILING SCHEDULE

CMST14 - USE (2) ROWS OF 10d NAILS @ 3 1/2" OC UNO

CMST12 - USE (2) ROWS OF 10d NAILS @ 3 1/2" OC UNO

CMST16 - USE (2) ROWS OF 16d SINKERS @ 3" OC UNO

SEE S037 FOR SPLICE DETAIL WHERE LENGTH SPECIFIED ON PLAN EXCEEDS AVAILABLE STRAP LENGTH PER MANUFACTURER
- TPS-X INDICATES TOPPLATE SPLICE @ TOP OF WALL ABOVE PER USE TPS-3 AT ALL SHEAR AND BEARING WALLS UNLESS NOTED OTHERWISE

INDICATES DRAG BEAM TO SHEAR WALL PER S037 WITH CMST14 UNO
- DRAG BEAM

INDICATES DRAG BEAM TO DRAG BEAM PER WITH (2) HDU11-SDS2.5 EA SIDE (TOTAL 4) UNO

INDICATES DRAG BEAM TO DRAG TRUSS PER WITH (2) HD7B EA SIDE (TOTAL 4) UNO
- INDICATES OPENING, IN PLAN SEE DETAILS S036, S036 & S036
- INDICATES OPENING IN SHEAR WALLS. VERIFY SIZE AND LOCATION WITH MEP/ARCH. WHERE ANY DISCREPANCIES OCCUR, PROMPTLY NOTIFY WITH AOR & SEOR, PRIOR TO PROCEEDING WITH WORK
- INDICATES PARTIAL HIGH DORM ROOM WALL PER S038

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

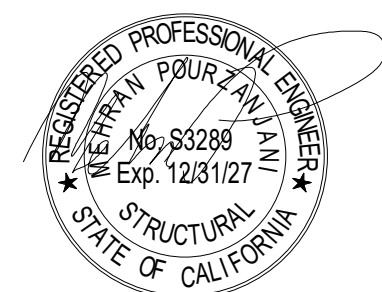
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725 S. Figueroa St.,
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LOW ROOF FRAMING PLAN

FIRE STATION 46

MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

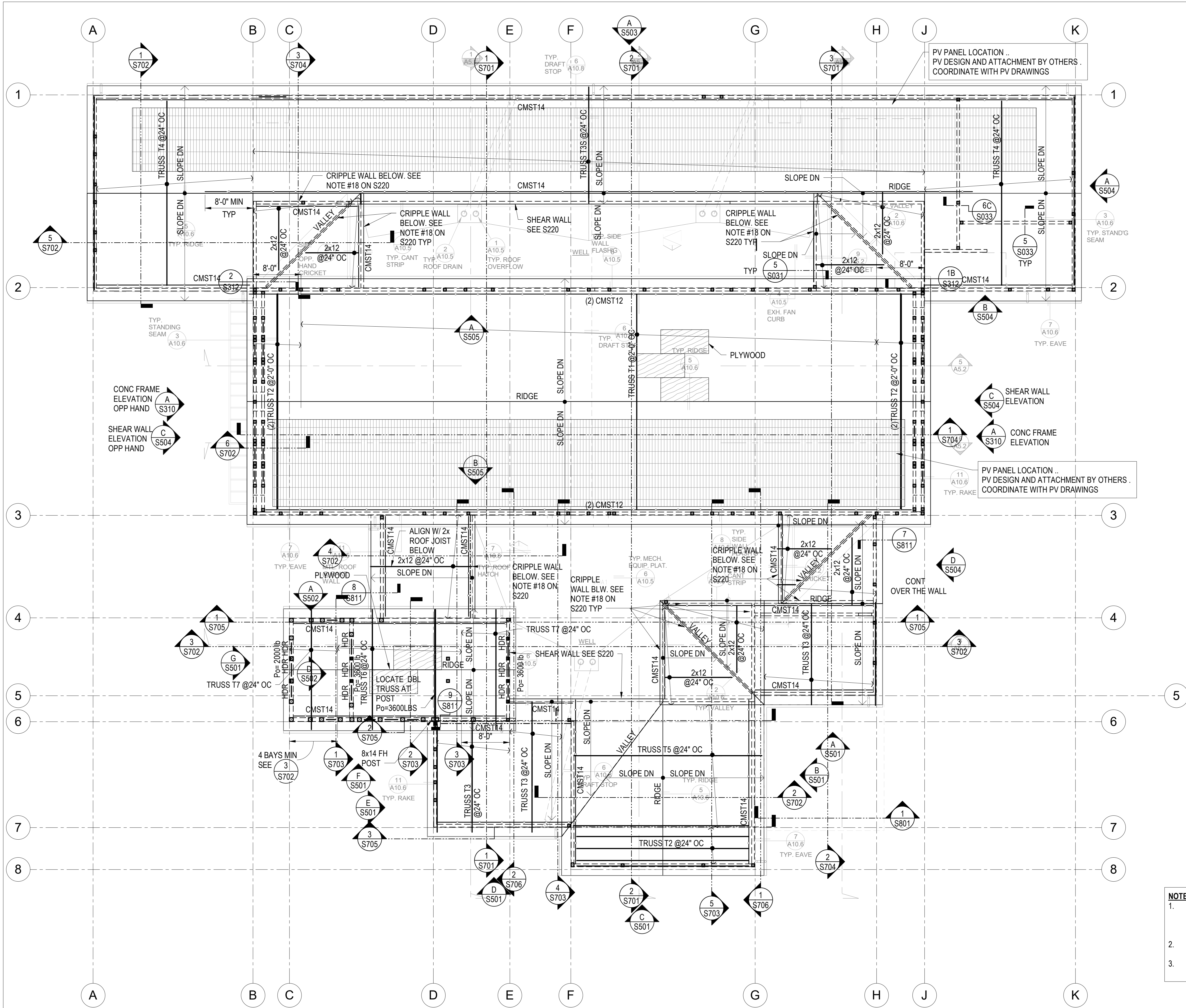


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Job No.	Project Number

S220

ADDENDUM 5



- NOTES:**
- SEE MECH'L DWGS FOR LOCATION OF HANGING EQUIPMENT FROM TRUSSES, AND DETAIL FOR STRUCTURAL SUPPORTING MEMBERS AT TRUSSES.
 - SEE SHEET S220 FOR FRAMING NOTES AND LEGENDS.
 - ROOF DIAPHRAGM NAILING TO BE INSPECTED BEFORE COVERING.

HIGH ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"

1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

so
saiful-bouquet
structural engineers

725 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
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HIGH ROOF FRAMING
PLAN
FIRE STATION 46
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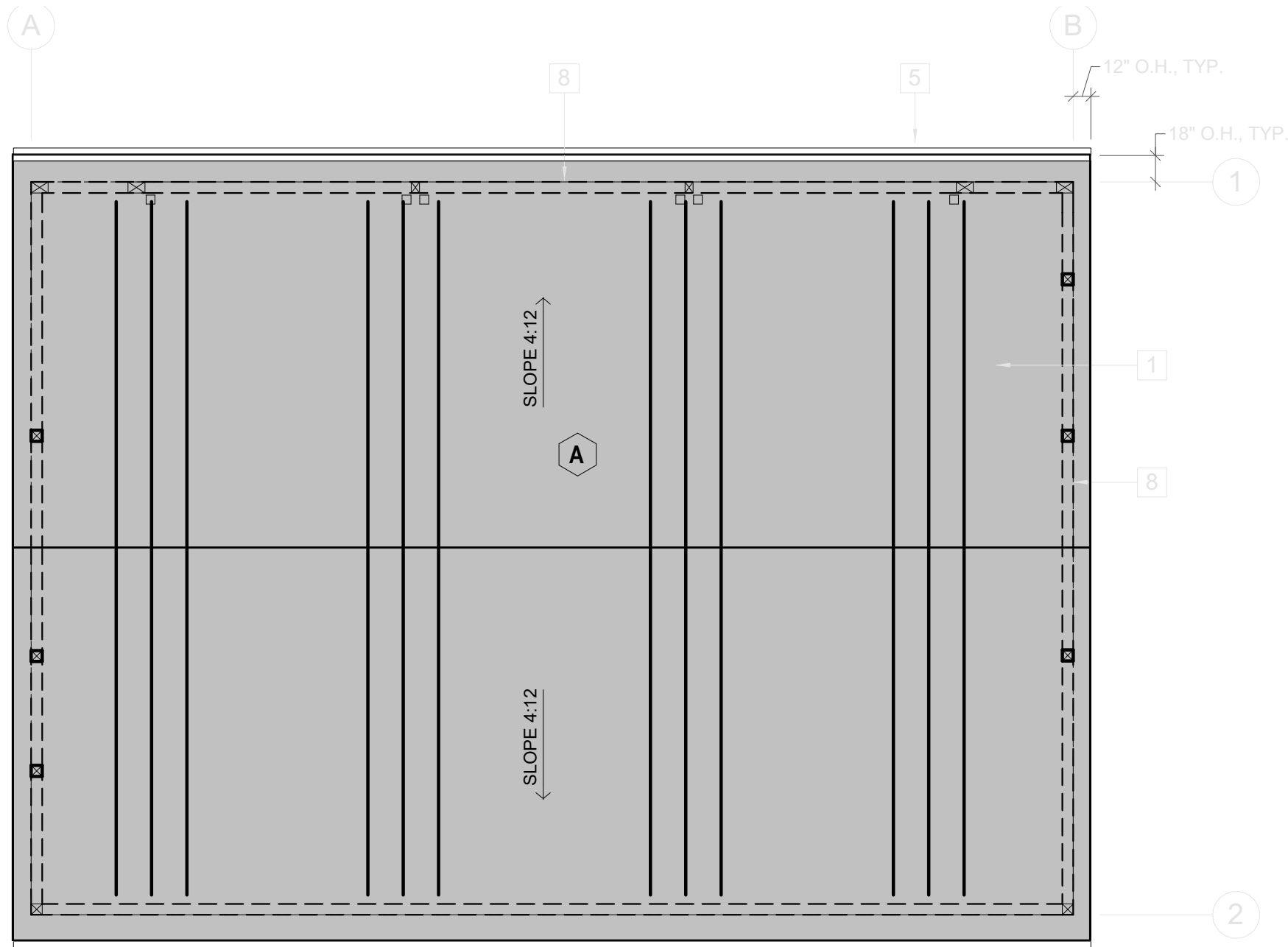


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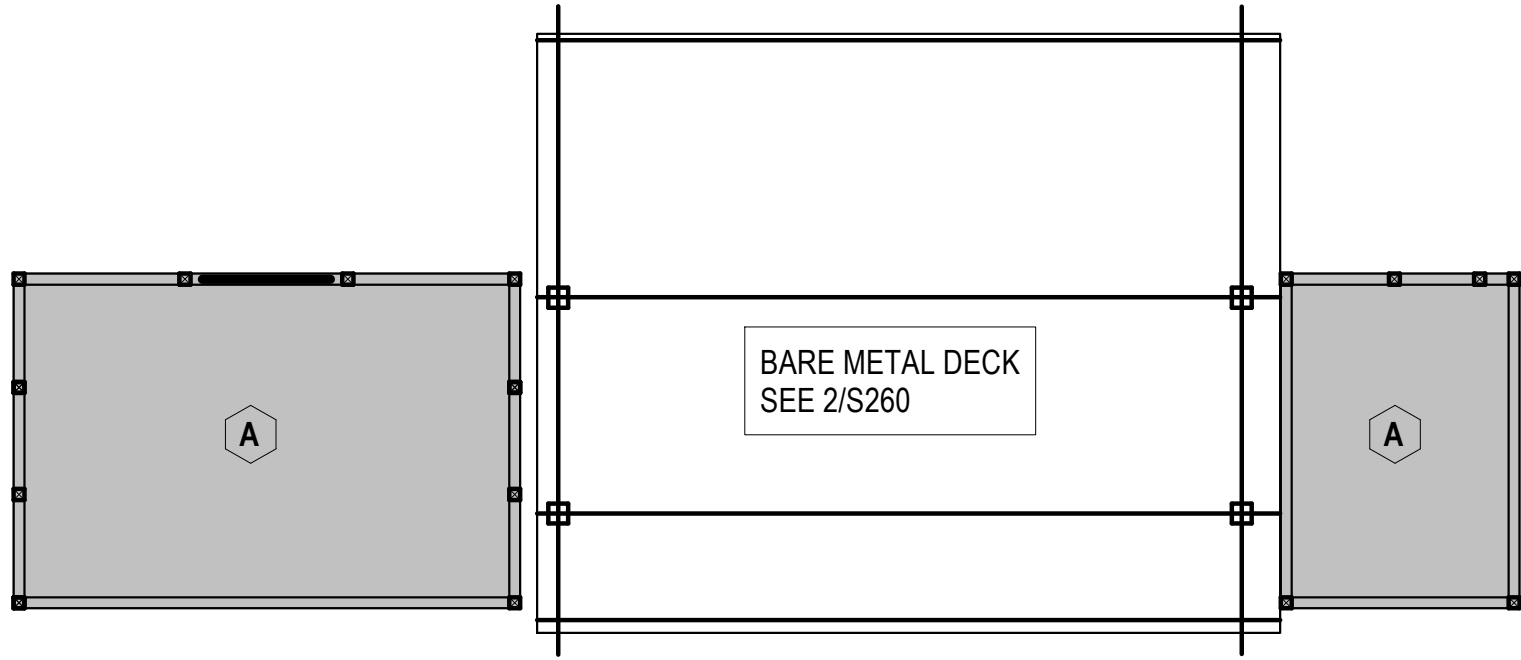
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S230

ADDENDUM 5



ROOF AT RESERVE APPARATUS BUILDING DIAPHRAGM PLAN



ROOF AT FUEL DISPENSING AREA DIAPHRAGM PLAN

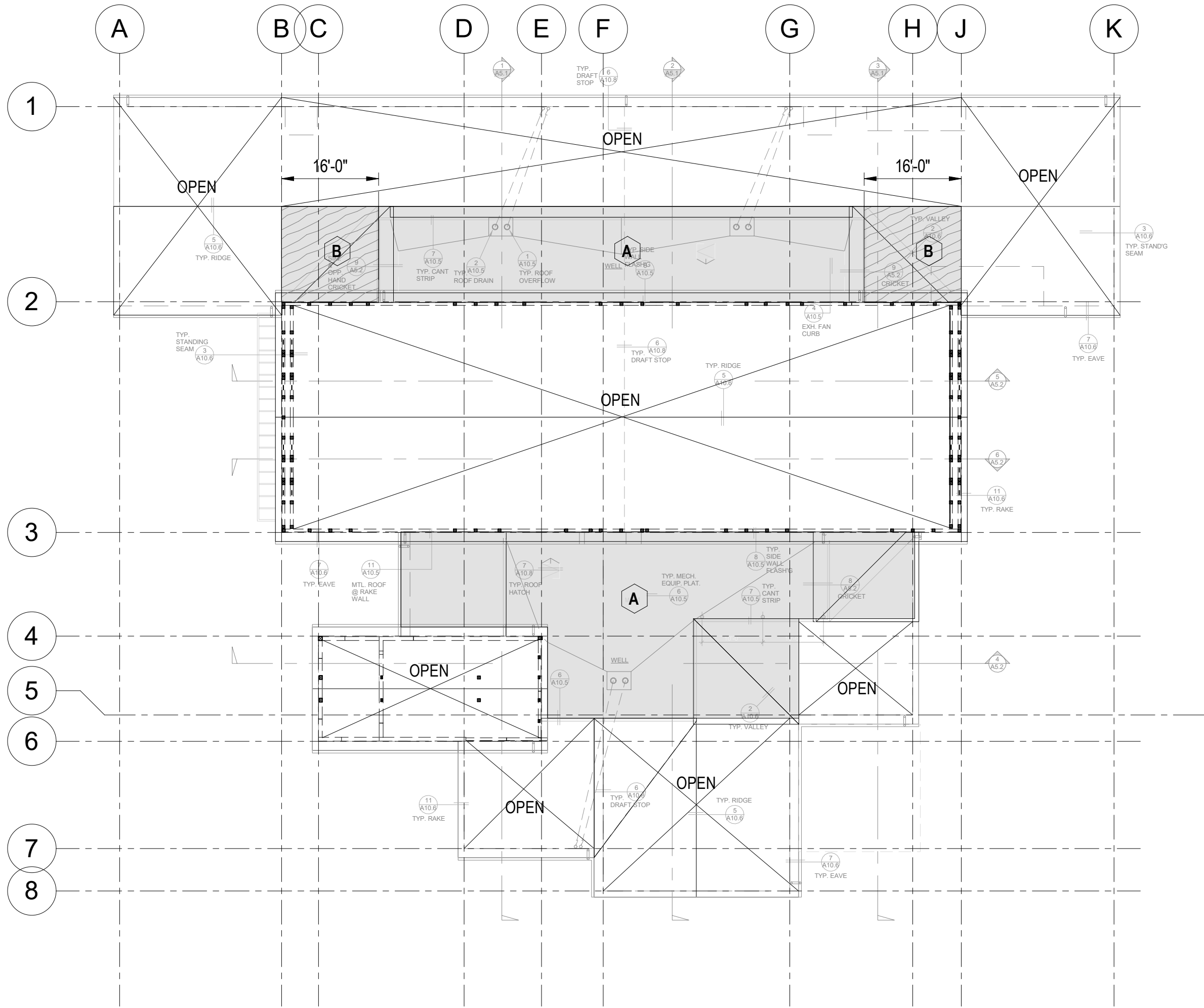
FRAMING PLAN NOTES

1. ROOF DIAPHRAGM SHEATHING AND NAILING SHALL BE AS FOLLOWS:

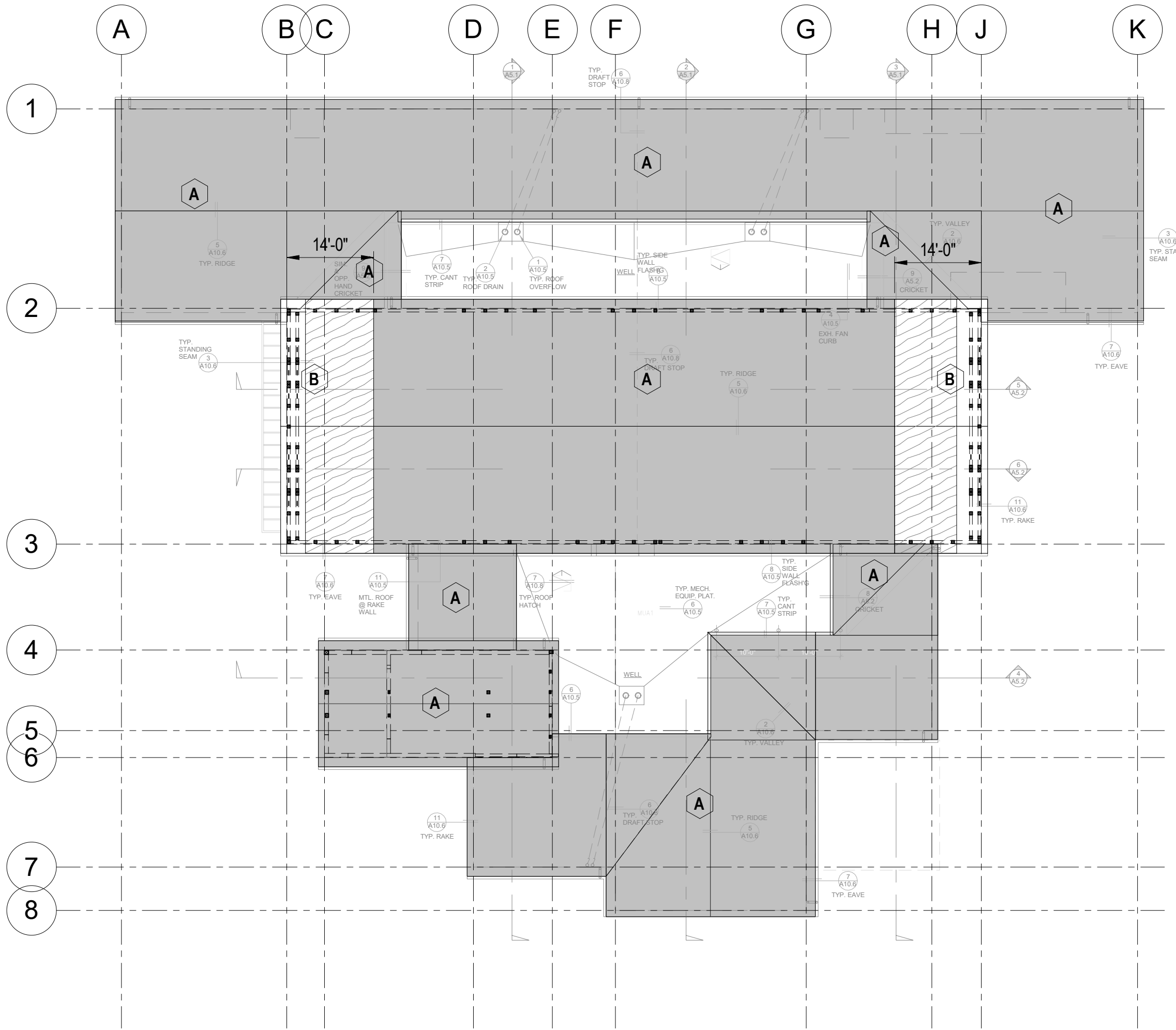
- TYPE **A** : 19/32" THICK CDX PLYWOOD (BLOCKED DIAPHRAGM)
10d@4" BN, 10d@6" EN, 10d@12" FN
- TYPE **B** : 19/32" THICK CDX PLYWOOD (BLOCKED DIAPHRAGM)
10d@2 1/2" BN, 10d@4" EN, 10d@12" FN
(USE DOUBLE TRUSSES AT ADJOINING PANELS AND BOUNDARY NAILING)

2. SEE DETAIL **3** FOR REMAINDER OF INFO.

3. ROOF DIAPHRAGM NAILING TO BE INSPECTED BEFORE COVERING



FLAT ROOF AT MAIN BUILDING DIAPHRAGM PLAN



SLOPED ROOF AT MAIN BUILDING DIAPHRAGM PLAN

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

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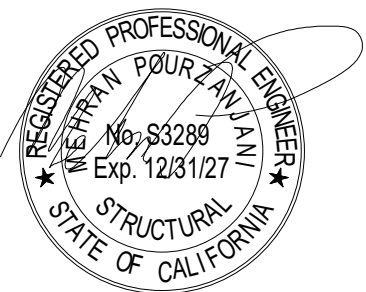
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3rd Floor
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ROOF WOOD DIAPHRAGM
PLANS

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AND DEVELOPED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT. VERBAL
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SCALED DIMENSIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS
FOR ALL DIMENSIONS AND CONDITIONS FOR ALL DIMENSIONS AND CONDITIONS
SHOWN IN THESE DRAWINGS. SHOP DRAWINGS SHALL BE SUBMITTED TO THIS
OFFICE FOR APPROVAL BEFORE PROCEEDING WITH FABRICATION.
DATE

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APPENDIX 5
S240



3



2



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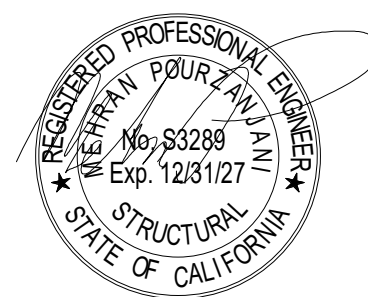
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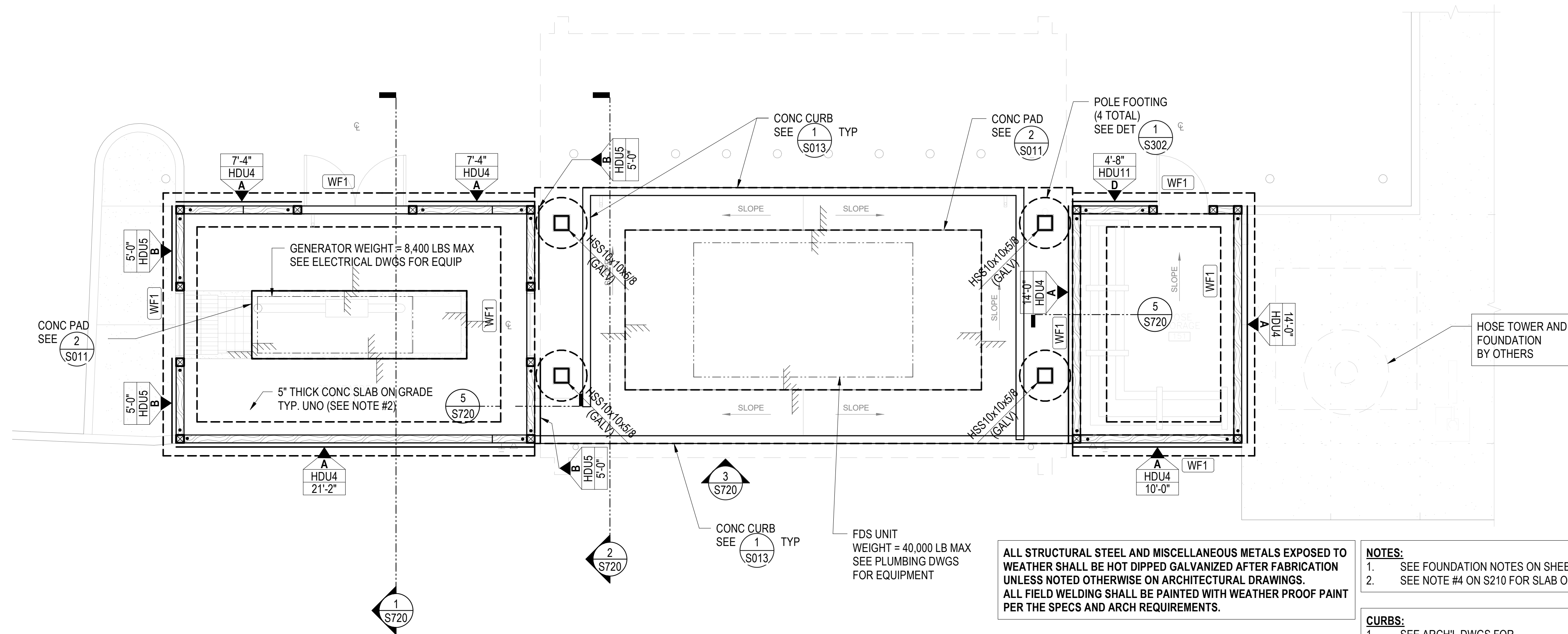
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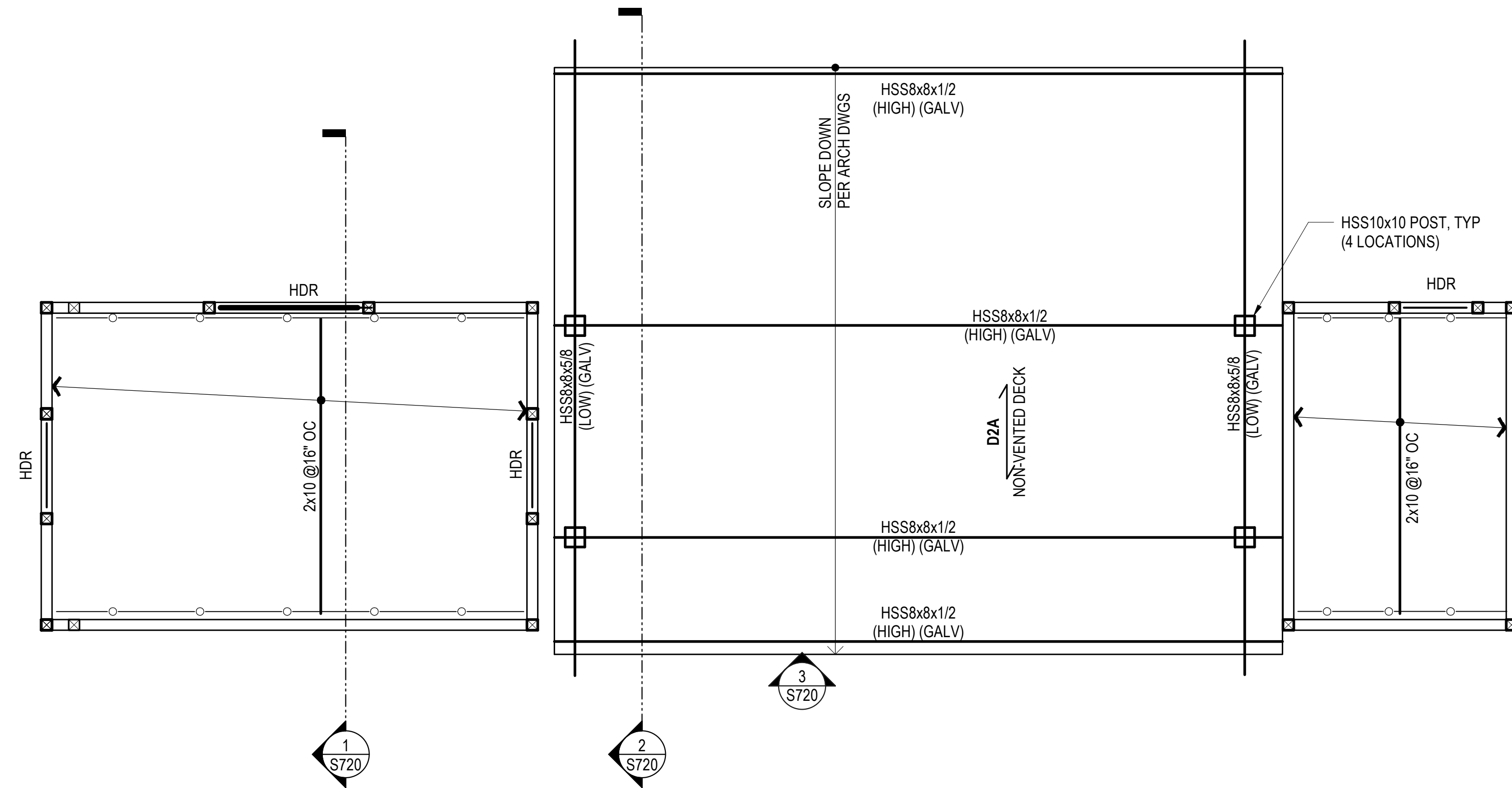
S250

PLAN CHECK SUBMITTAL - October 31 2025
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SCALE: 1/4" = 1'-0"

1

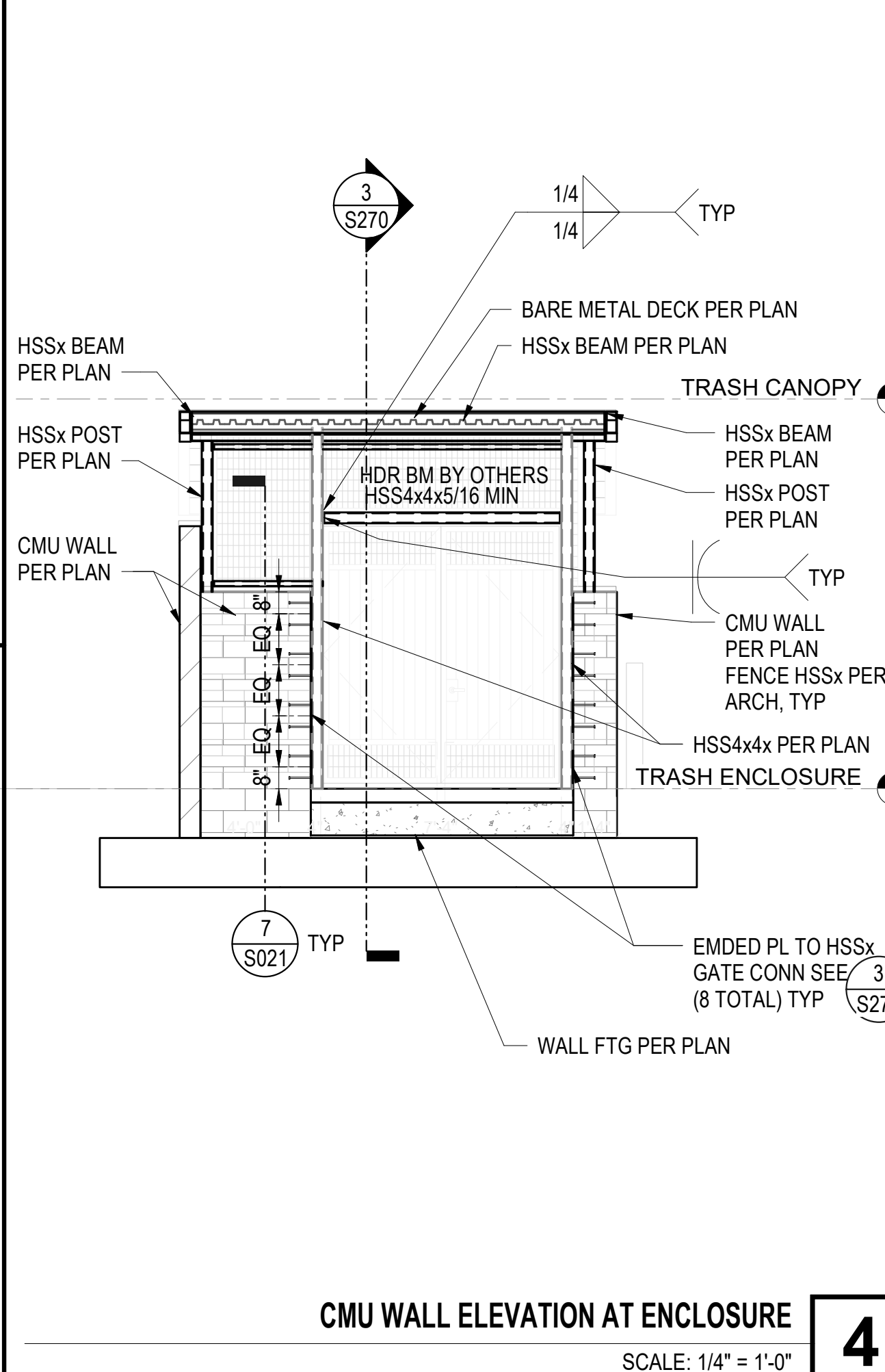
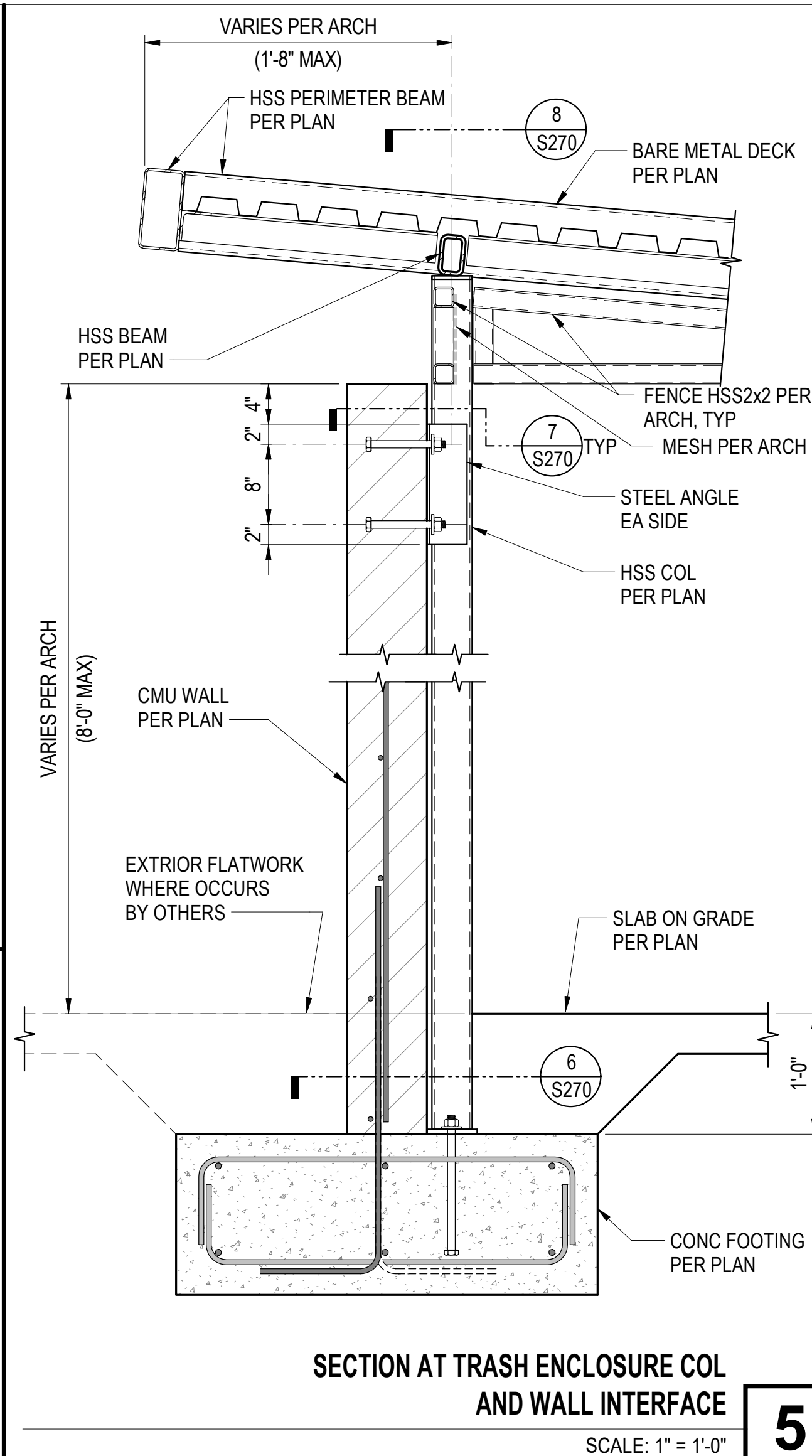
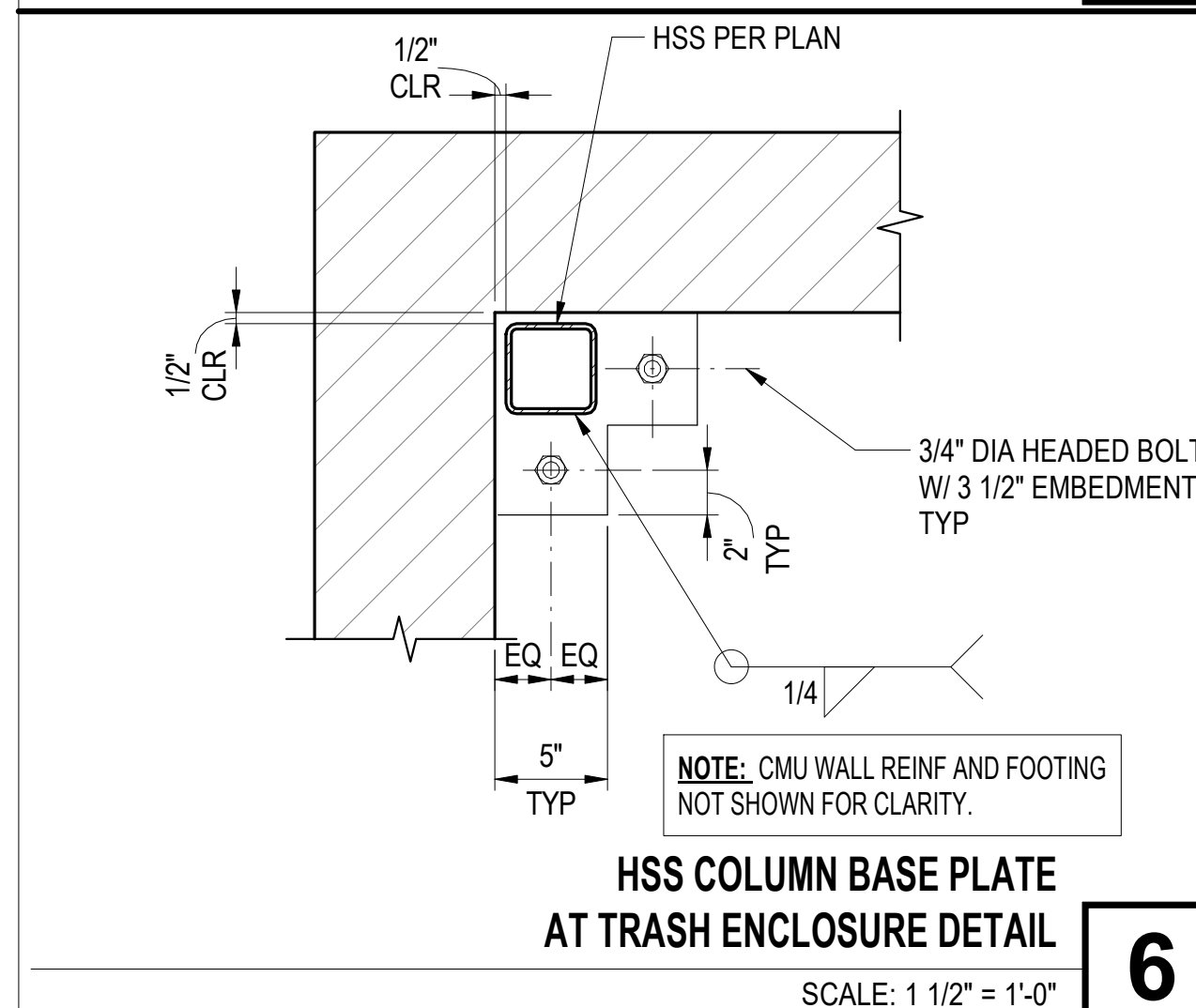
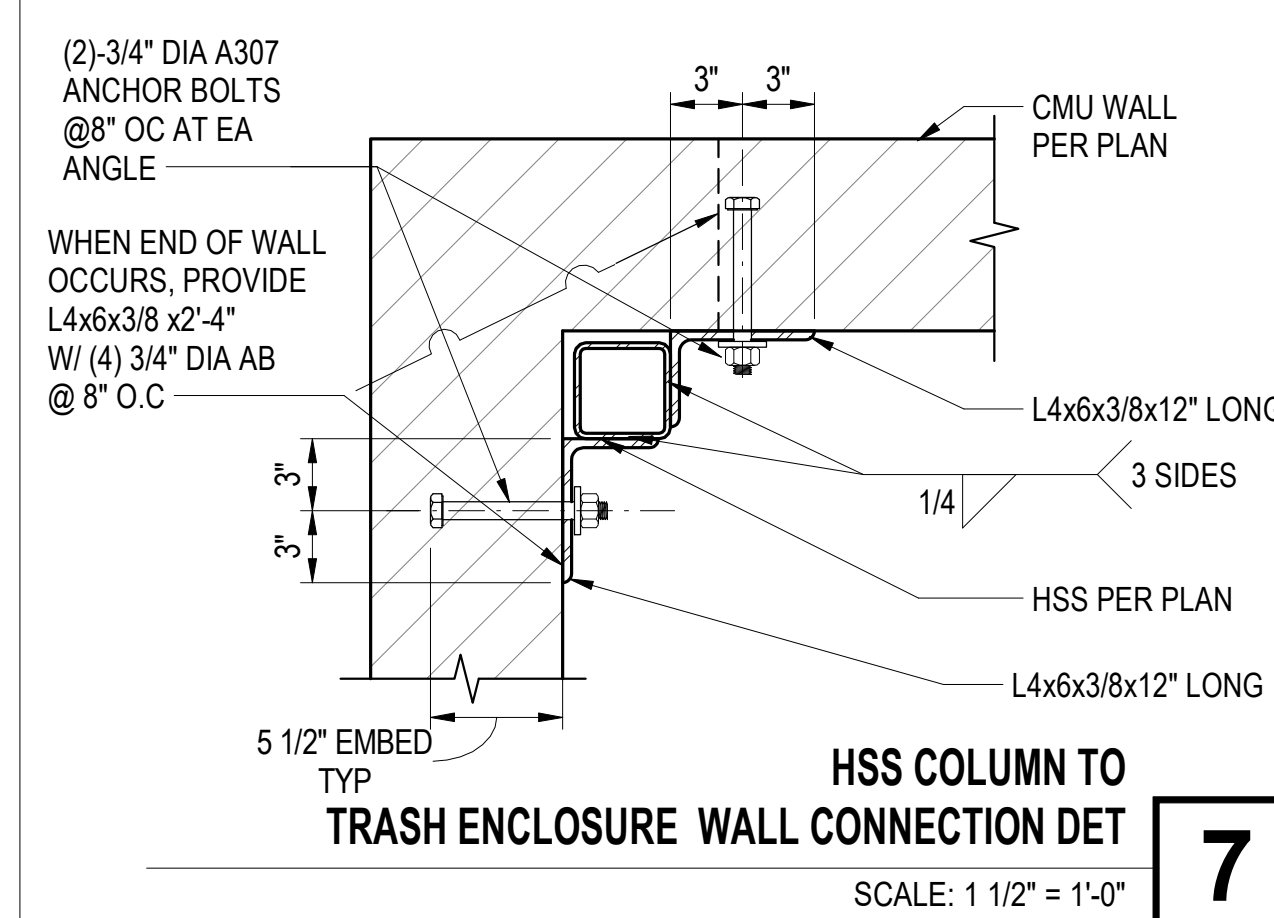
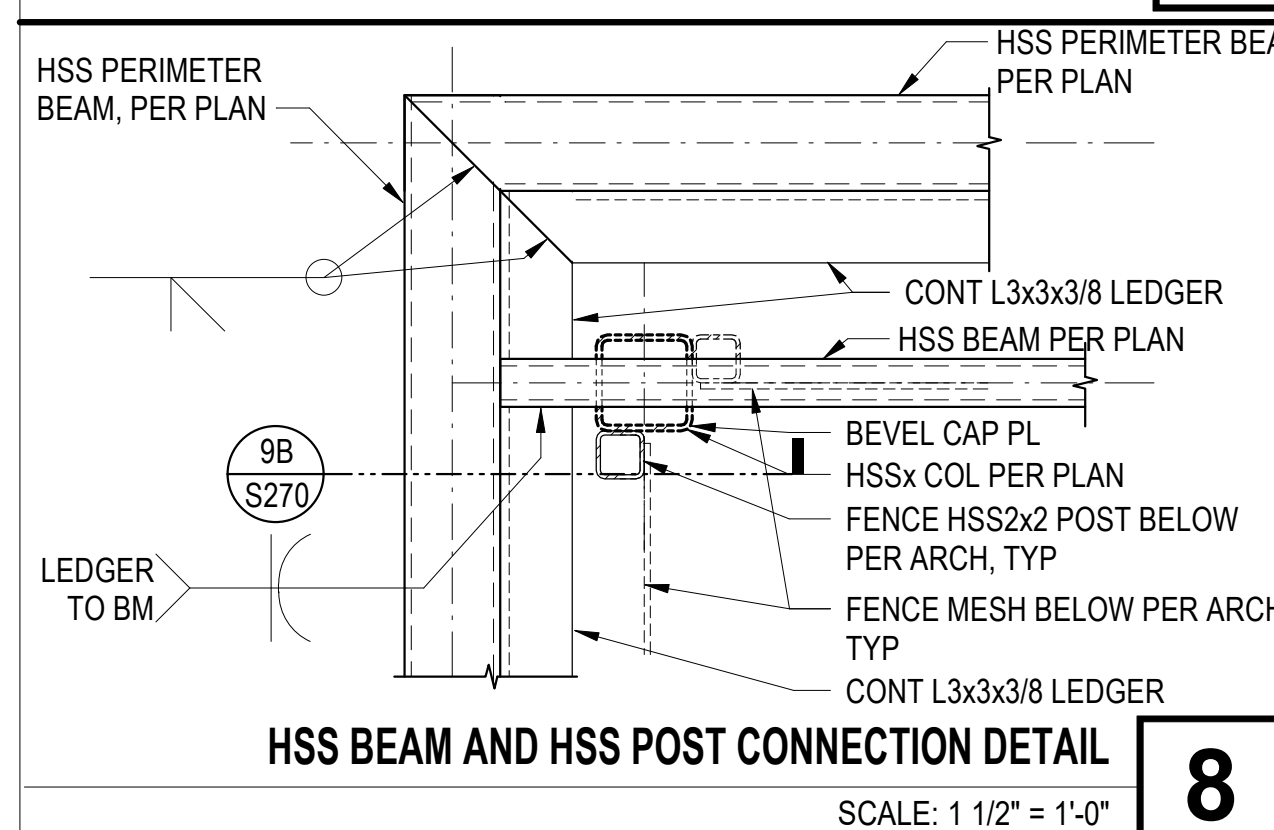
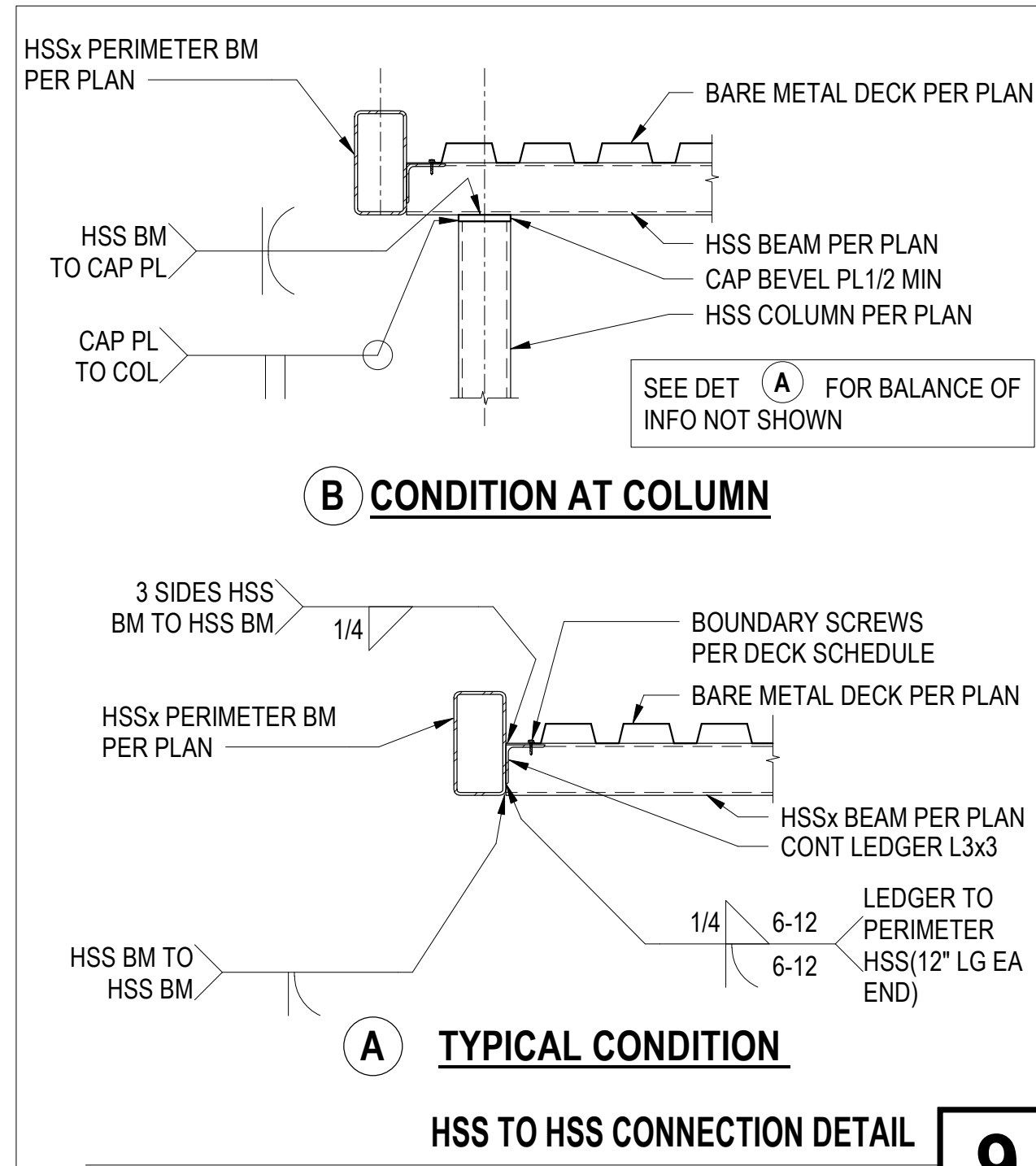


ALL STRUCTURAL STEEL AND MISCELLANEOUS METALS EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION UNLESS NOTED OTHERWISE ON ARCHITECTURAL DRAWINGS. ALL FIELD WELDING SHALL BE PAINTED WITH WEATHER PROOF PAINT PER THE SPECS AND ARCH REQUIREMENTS.





- NOTES:
1. SEE ROOF NOTES ON SHEET S220
 2.  INDICATES DIRECTION OF STEEL DECKING AND SLAB CONSTRUCTION. SEE SCHEDULE ON
(1)
S015
 3. ROOF DIAPHRAGM SHEATHING AND NAILING SHALL BE AS FOLLOWS:
SEE SHEET S240
 4. SEE PLAN NOTES AND LEGEND ON S220 & S270 FOR BALANCE OF INFO.

SCALE: 1/4" = 1'-0"












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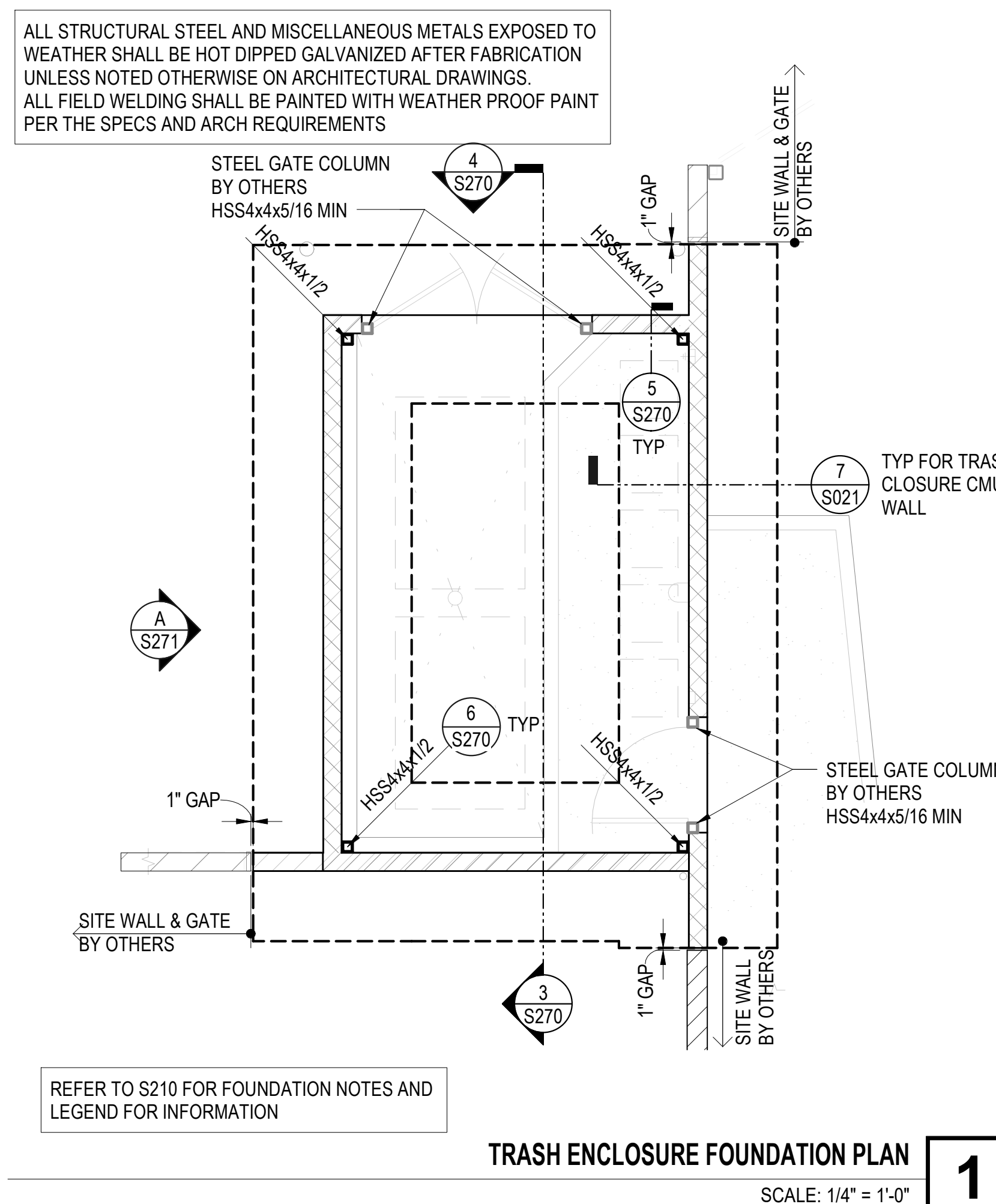
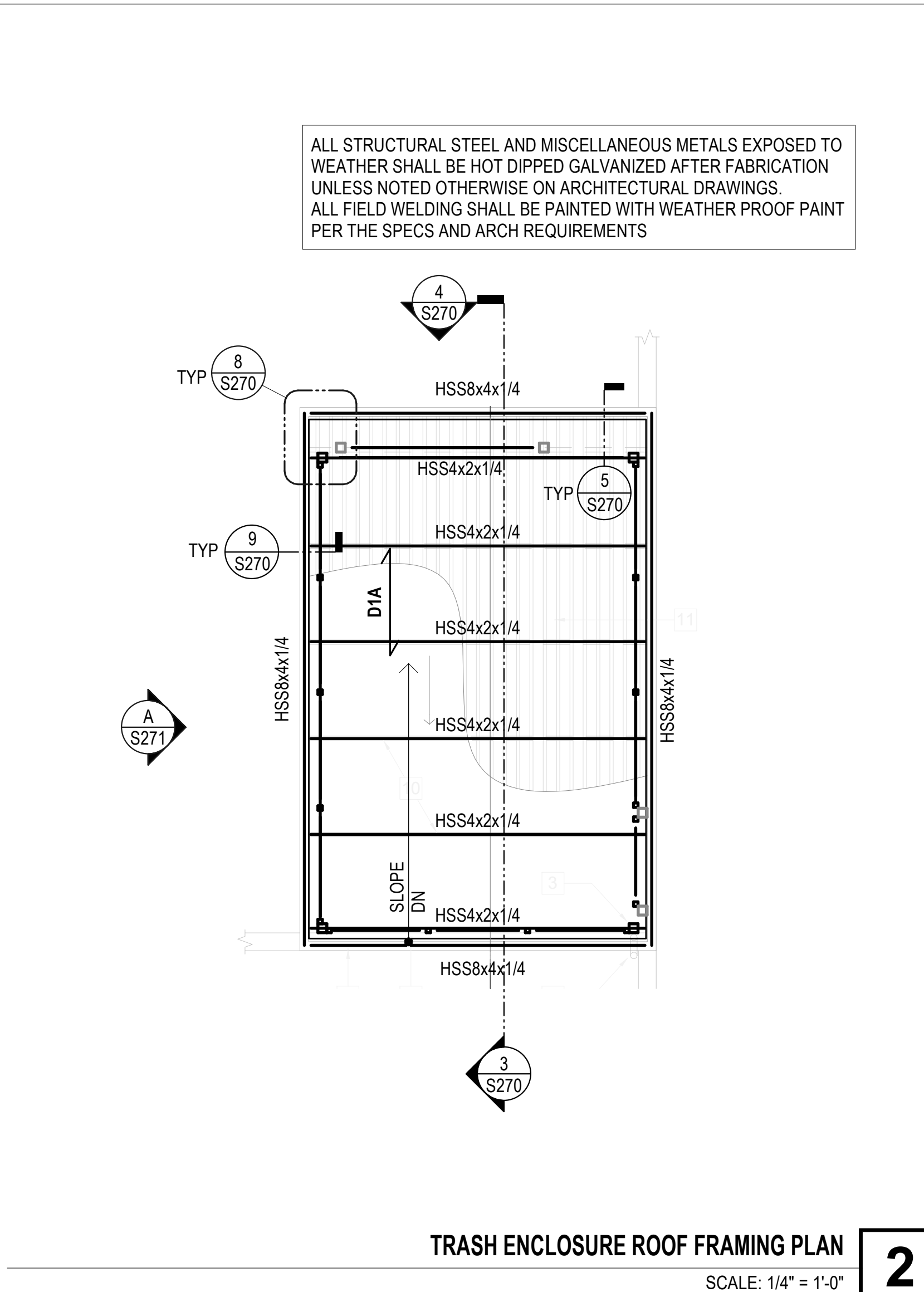
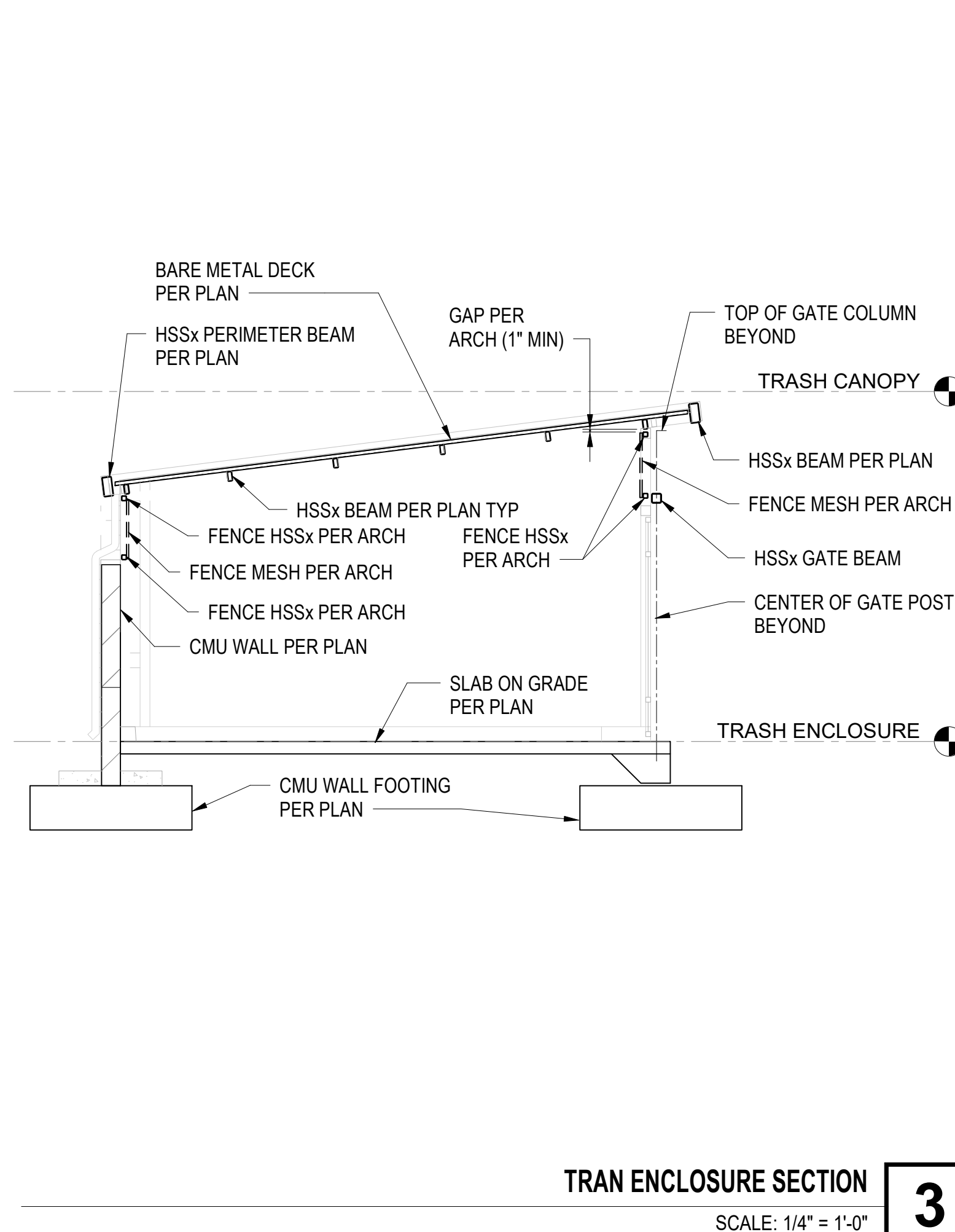


- ### ROOF FRAMING PLAN NOTES

1. FOR GENERAL NOTES SEE **S0.0X** SERIES SHEETS. TYPICAL DETAILS OCCUR THROUGHOUT THESE STRUCT DWGS IN ADDITION TO THOSE ON **S0.XX** SERIES SHEETS.
2. VERIFY SLAB ELEVATIONS INCLUDING SLAB DEPRESSIONS, SLOPES, OPNGS, CURBS, DRAINS, TRENCHES, & SLAB EDGE LOCATIONS; & WALL OVERALL DIMENSIONS INCLUDING LOCATIONS OF OPNGS WITH ARCHITECTURAL DWGS.
3. SEE ARCHITECTURAL DWGS FOR REMAINDER OF DIMENSIONS & ELEVATIONS NOT SHOWN ON STRUCT DWGS. VERIFY ALL DIMENSIONS & ELEVATIONS W/ ARCHITECTURAL DWGS PRIOR TO START OF WORK.
4. CENTER COLUMNS ON GRIDLINES UNO.
5. SPACE BEAMS EQUALLY BETWEEN COLUMNS & GIRDERS UNO
6. TOP OF THE STRUCTURAL STEEL (TOS EL) SHALL BE TOP OF SLAB ELEVATION MINUS THE THICKNESS OF THE STRUCTURAL AS SHOWN ON DECK SCHEDULE ON 
7. SLOPE THE STRUCTURAL STEEL FRAMING TO ACCOMMODATE THE ROOF SLOPES. TYPICAL SLOPING PATTERNS FOR THE STRUCTURAL STEEL FRAMING MEMBERS ARE INDICATED ON THE PLANS. SEE ARCHITECTURAL DRAWINGS FOR A COMPREHENSIVE PLAN OF ROOF DRAINAGE PATTERN AND COORDINATE ACCORDINGLY.
8. THERE SHALL BE NO STEPS OR ELEVATION OFFSETS AT THE BOTTOM OF STRUCTURAL DECK UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION. THE STRUCTURAL STEEL FRAMING ELEVATION SHALL PROVIDE FOR CONTINUITY OF THE ROOF DECK AT ALL CONDITIONS.
9. ALL EXPOSE STEEL AND CONNECTIONS SHALL BE HOT-DIP GALVANIZED, TYP 
10. ATTACH DECK TO ALL BEAM PARALLEL TO FLUTE, SEE DETAIL 
11. PROVIDE CAP PLATE FOR HSS BEAM OR HSS POST WHERE EXPOSED TO WEATHER SEE TYP UNO 

- ## ROOF FRAMING LEGEND

- | | | |
|---|-------------|---|
|  | EL XXX'-XX" | DATUM TOP OF STRUCT SLAB ELEVATION - VERIFY W/ ARCHITECTURAL DWGS |
|  | (-X") | TOP OF STRUCT SLAB ELEVATION OTHER THAN DATUM - VERIFY W/ ARCHITECTURAL DWGS |
|  | | NON-FRAME STEEL COLUMN MARK FOR COLUMN OCCURRING ABOVE - "CB" DENOTES COLUMN BELOW - COLUMN W/ NO MARK DENOTES CONTINUING COLUMN FROM BELOW TO POINTS ABOVE - SEE  UNO AND  |
|  | | DECK CONSTRUCTION MARK - ARROWS DENOTE DECK SPAN DIRECTION - SEE  |
|  | | INDICATES BEAM TO BEAM, TYPICAL CONNECTION SEE DETAIL  |
|  | | INDICATES BEAM TO COLUMN CONNECTION PER  |



WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995



Sbaifal-bouquet
structural engineers

725 S. Figueroa St.,
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Project #25534

TRASH ENCLOSURE

FIRE STATION 46

MISSION VILLAGE

MISSION VILLAGE
COUNTY OF LOS ANGELES
VALENCIA, CALIFORNIA



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Scale	AS NOTED
Job. No.	Project Number

S270



SCALE: 1 1/2" = 1'-0"

2



SCALE: 1" = 1'-0"

1



TYPICAL GATE POST TO CMU WALL CONNECTION DETAIL

SCALE: 1" = 1'-0"

3



SCALE: 1/4" = 1'-0"

A

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

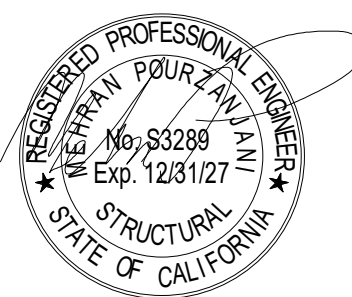


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structural engineers

725 S. Figueroa St.,
37th Floor
Los Angeles, CA 90017
www.saifubouquet.com
(213) 315-2277
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TRASH ENCLOSURE ELEVATION AND DETAILS

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VALENCIA, CALIFORNIA



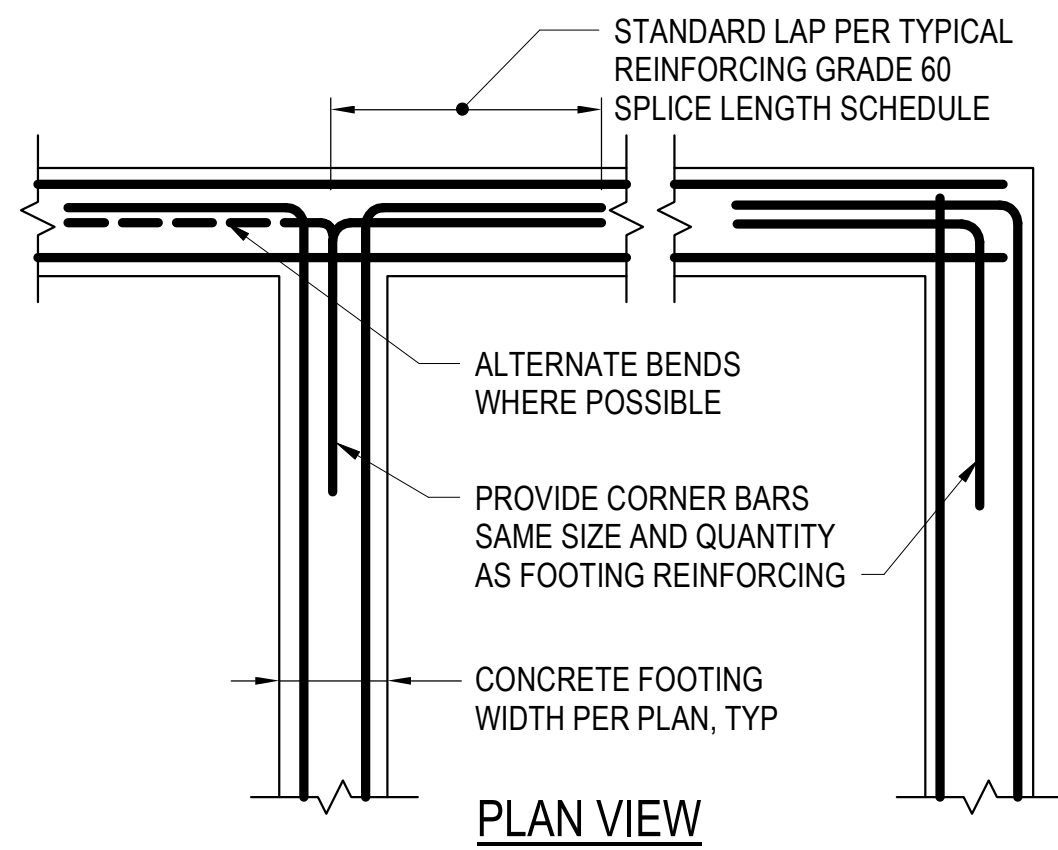
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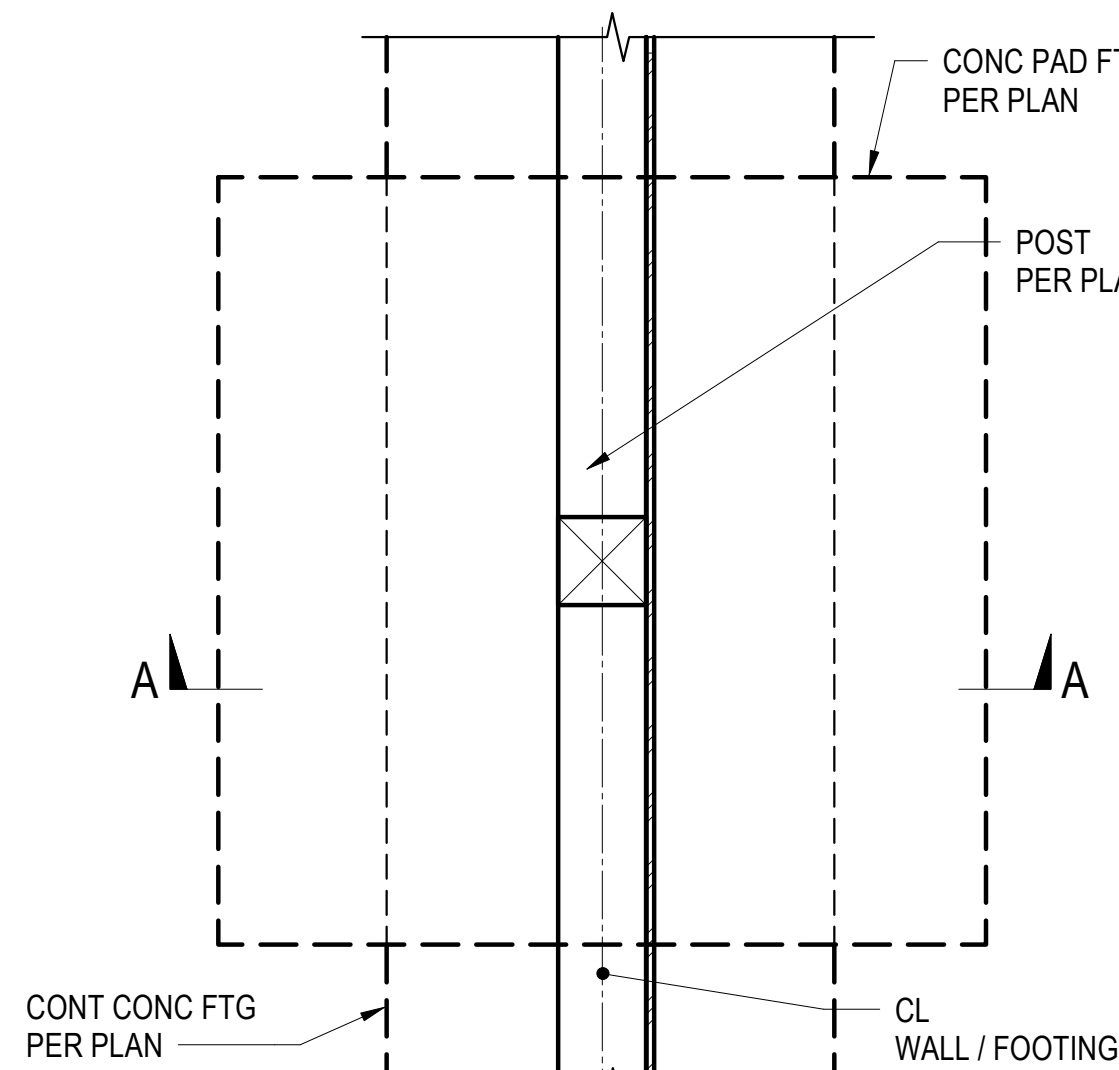
Drawings and specifications shall be prepared and sealed by a professional engineer who is duly licensed in the State of California and who is a member in good standing of the Architects and Engineers Council for Professional Development. The drawings and specifications shall be prepared by the architect and shall be subject to the review and approval of the engineer. The drawings and specifications shall be prepared by the architect and shall be subject to the review and approval of the engineer. The drawings and specifications shall be prepared by the architect and shall be subject to the review and approval of the engineer.



TYPICAL FOOTING REINFORCING AT CORNERS AND INTERSECTIONS DET

SCALE: NTS

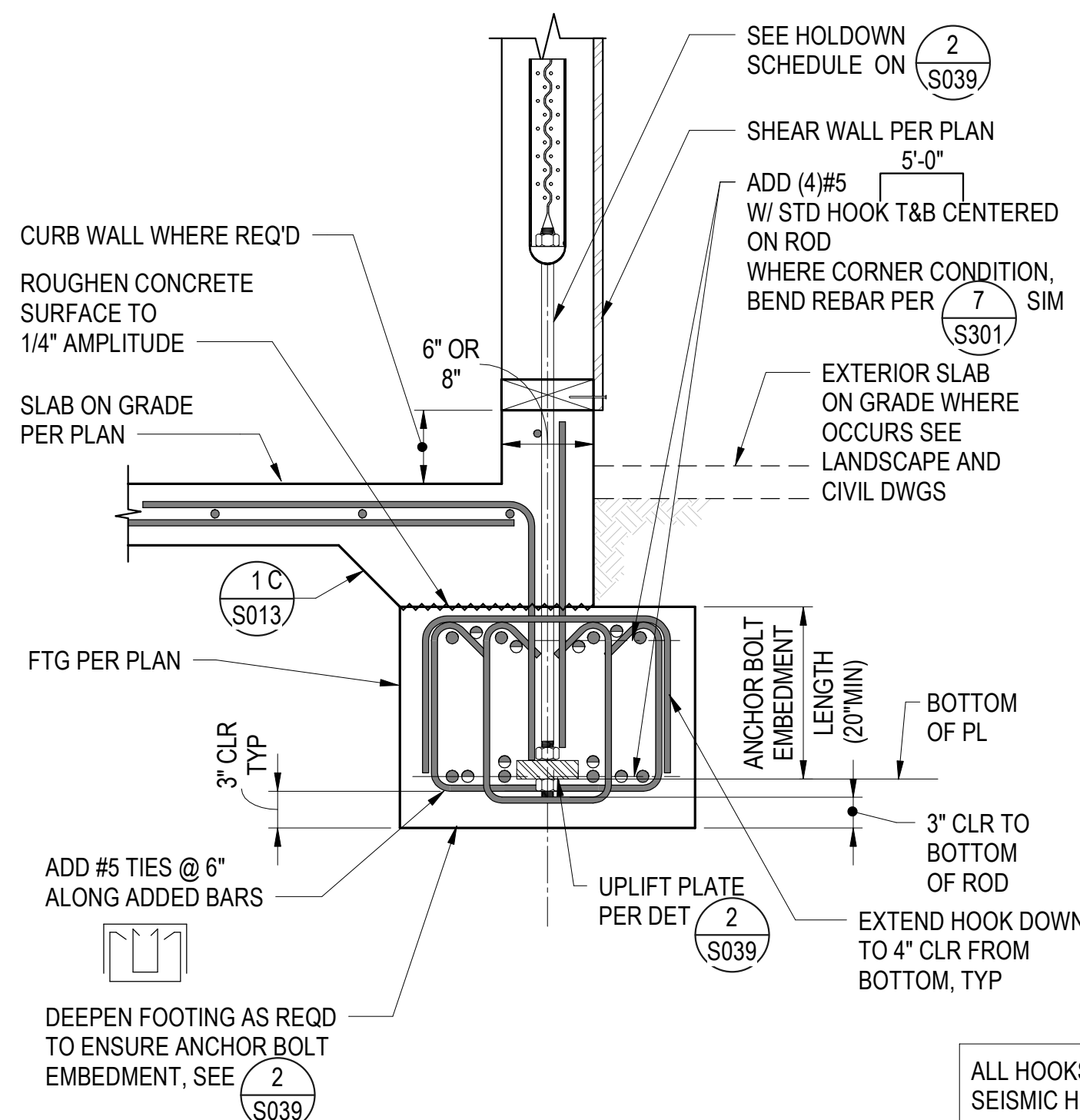
7



TYPICAL PAD FOOTING AT CONTINUOUS WALL FOOTING

SCALE: NTS

6



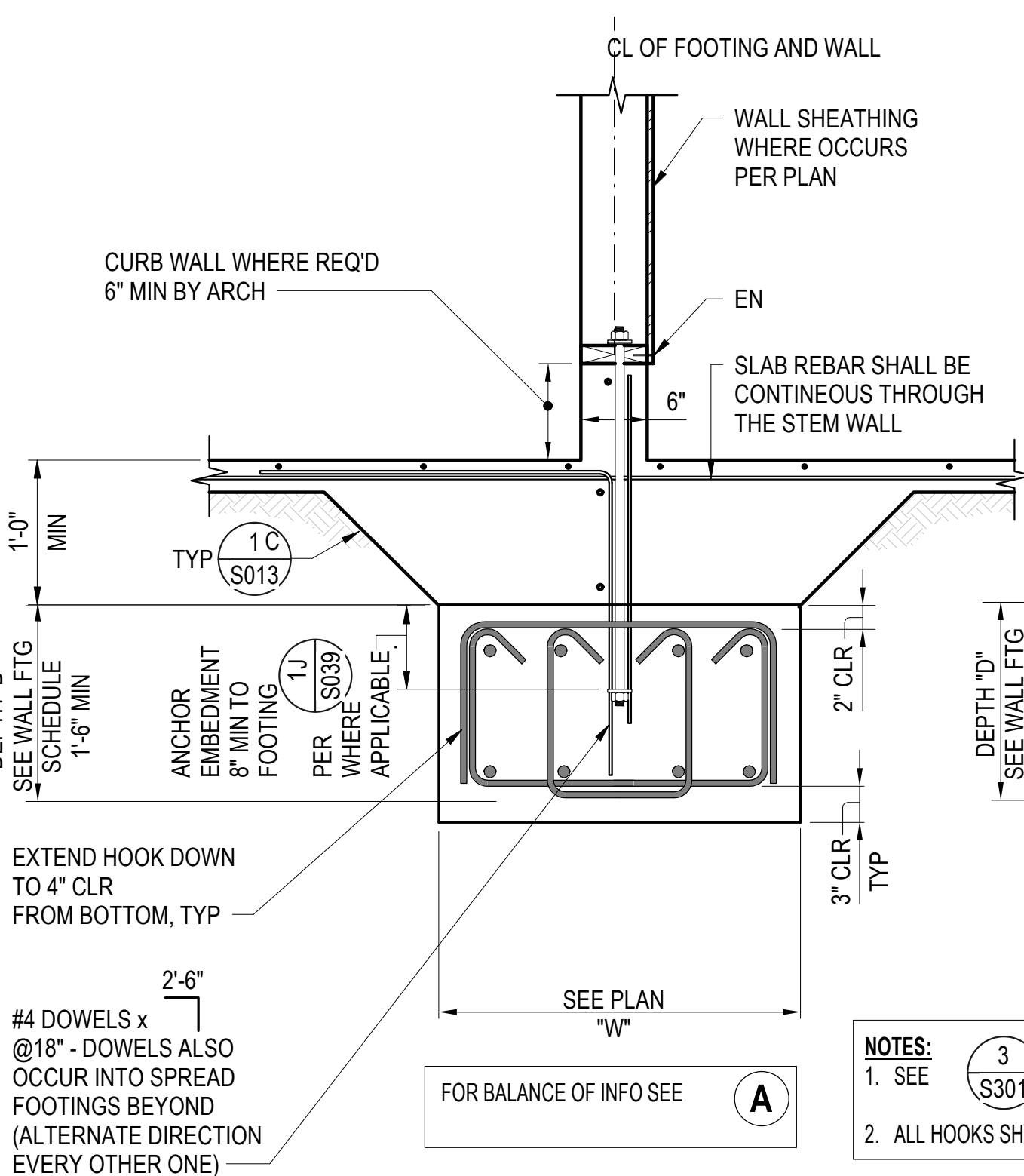
EXTERIOR WALL CONDITION

- ALL HOOKS SHALL BE SEISMIC HOOKS
- INDICATES MAIN FTG REINFORCING PER SCHEDULE.
- INDICATES ADDED REINFORCING.

TYPICAL SHEAR WALL HOLD DOWN ANCHORAGE WALL FTG DETAIL

SCALE: NTS

3



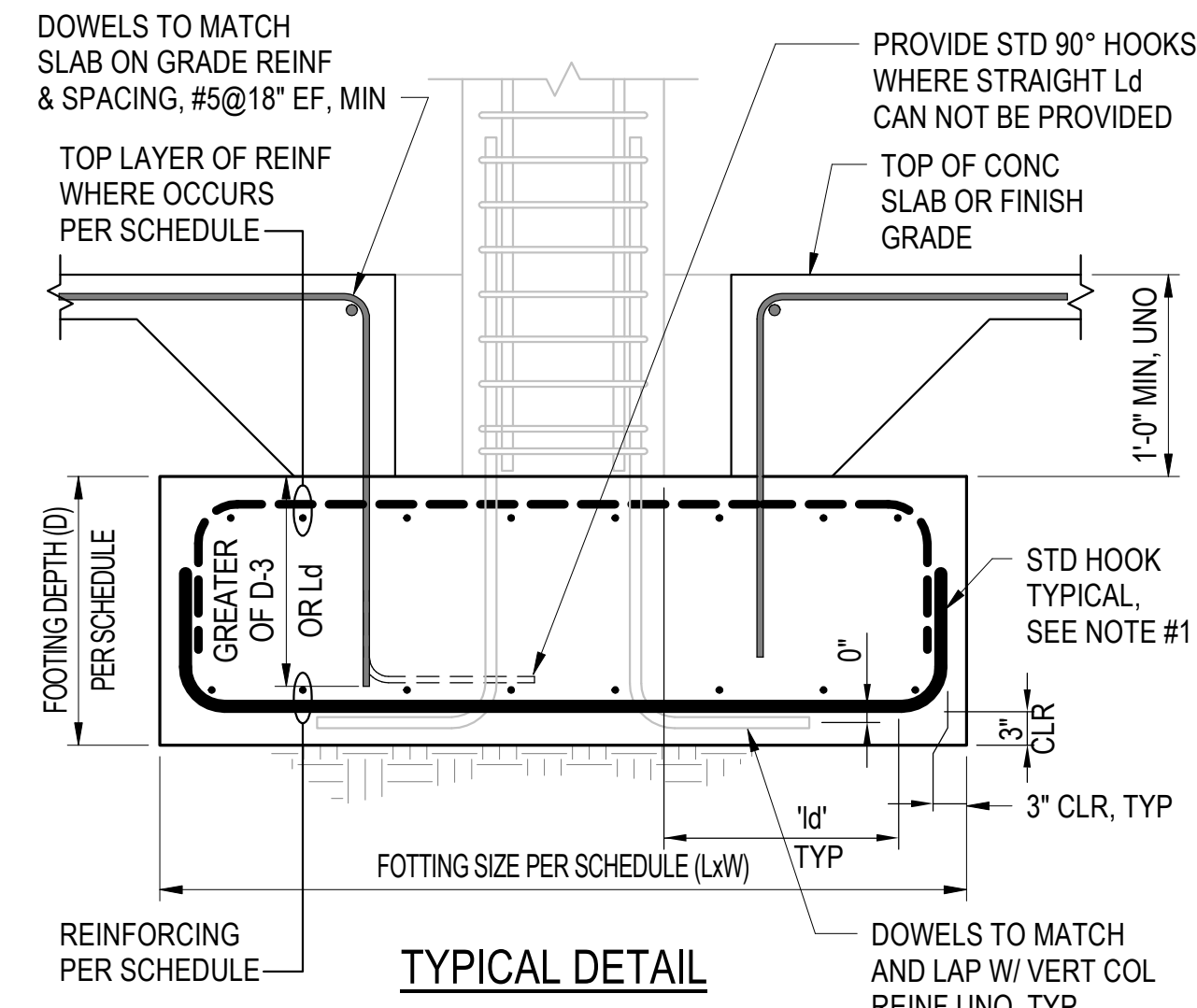
INTERIOR WALL CONDITION

WALL FOOTING SCHEDULE			
MARK	DEPTH (D)	WIDTH (W)	REMARKS
WF1	18"	2'-0"	SEE DET
WF2	18"	3'-0"	SEE DET
WF3	18"	3'-6"	SEE DET

TYPICAL PERIMETER FOOTING AT ENTRY

SCALE: NTS

4



TYPICAL DETAIL

SPREAD FOOTING SCHEDULE

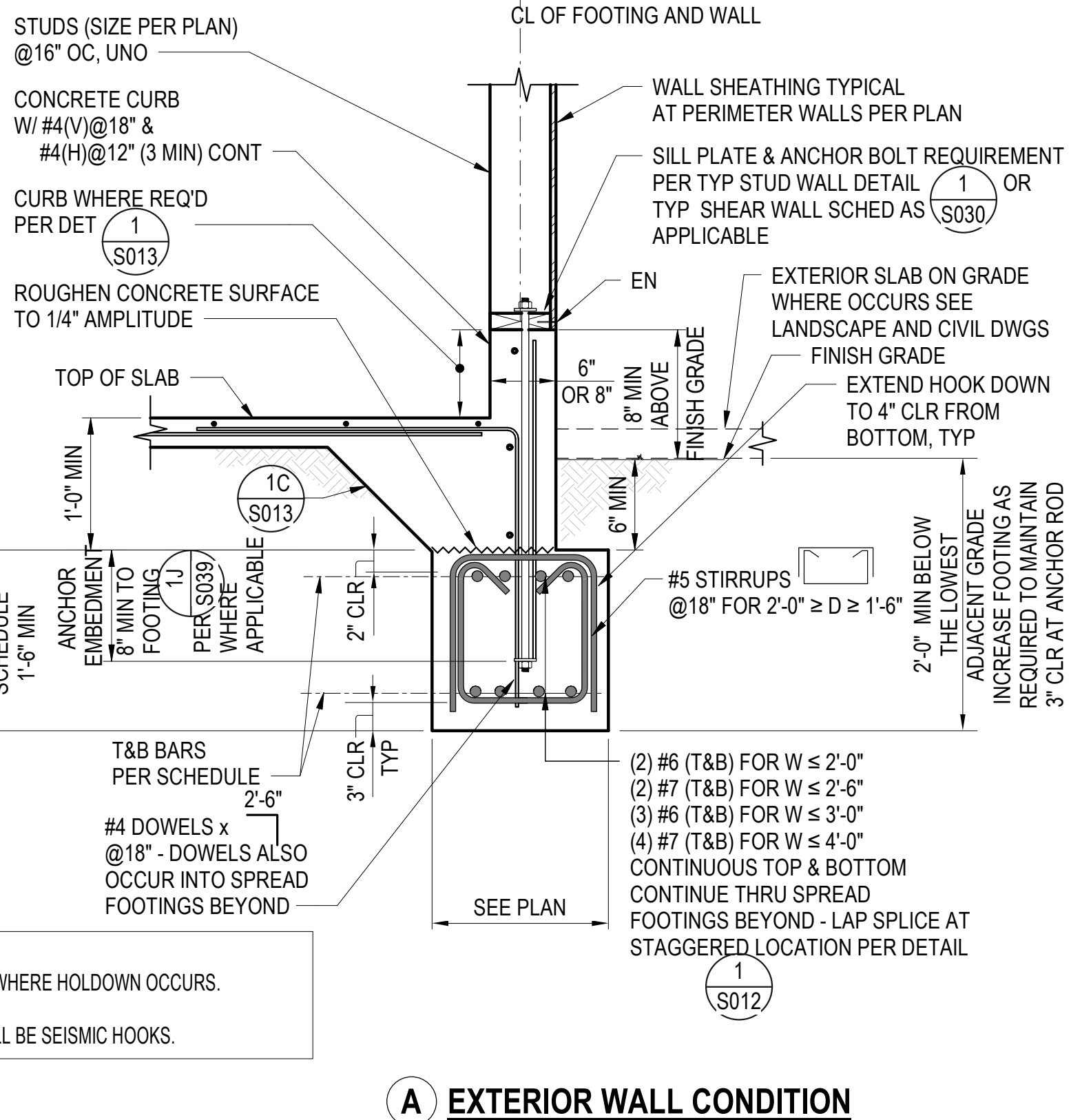
MARK	DEPTH (D)	SIZE (LxW)	REINFORCING				REMARKS
			BOTTOM		TOP		
			SHORT	LONG	SHORT	LONG	
F1	18"	4'-0"x4'-0"	(4)#6(B) EW	(4)#6(B) EW	-	-	
F2	18"	5'-0"x5'-0"	(6)#6(B) EW	(6)#6(B) EW	-	-	
FF1	36"	18'-0"x6'-6"	#8@12"	#8@12"	#8@12"	#8@12"	
FF2	36"	12'-0"x6'-6"	#8@12"	#8@12"	#8@12"	#8@12"	

- NOTES:
- "Ld"=STRAIGHT DEVELOPMENT LENGTH PER "TYPICAL STRAIGHT AND HOOKED DEVELOPMENT LENGTH SCHEDULE".
 - PROVIDE STANDARD HOOK WHERE A STRAIGHT BAR WITH "Ld" CANNOT BE PROVIDED.

TYPICAL SPREAD FOOTING SCHEDULE AND DETAIL

SCALE: NTS

2



EXTERIOR WALL CONDITION

TYPICAL CONTINUOUS BEARING OR SHEAR WALL FOOTING DETAIL

SCALE: NTS

1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

SO
saiful-bouquet
structural engineers

726 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
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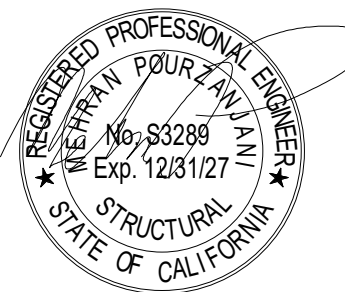
CONCRETE FOUNDATION
DETAILS

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S301

APPENDIX 5



SCALE: 1/2" = 1'-0"

A



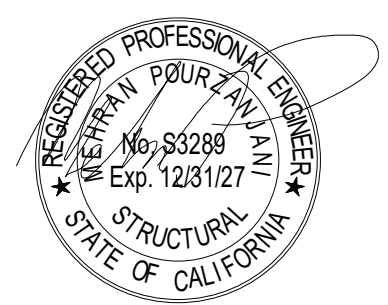
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725 S. Figueroa St.,
37th Floor
Los Angeles, CA 90017
www.saifullbouquet.com
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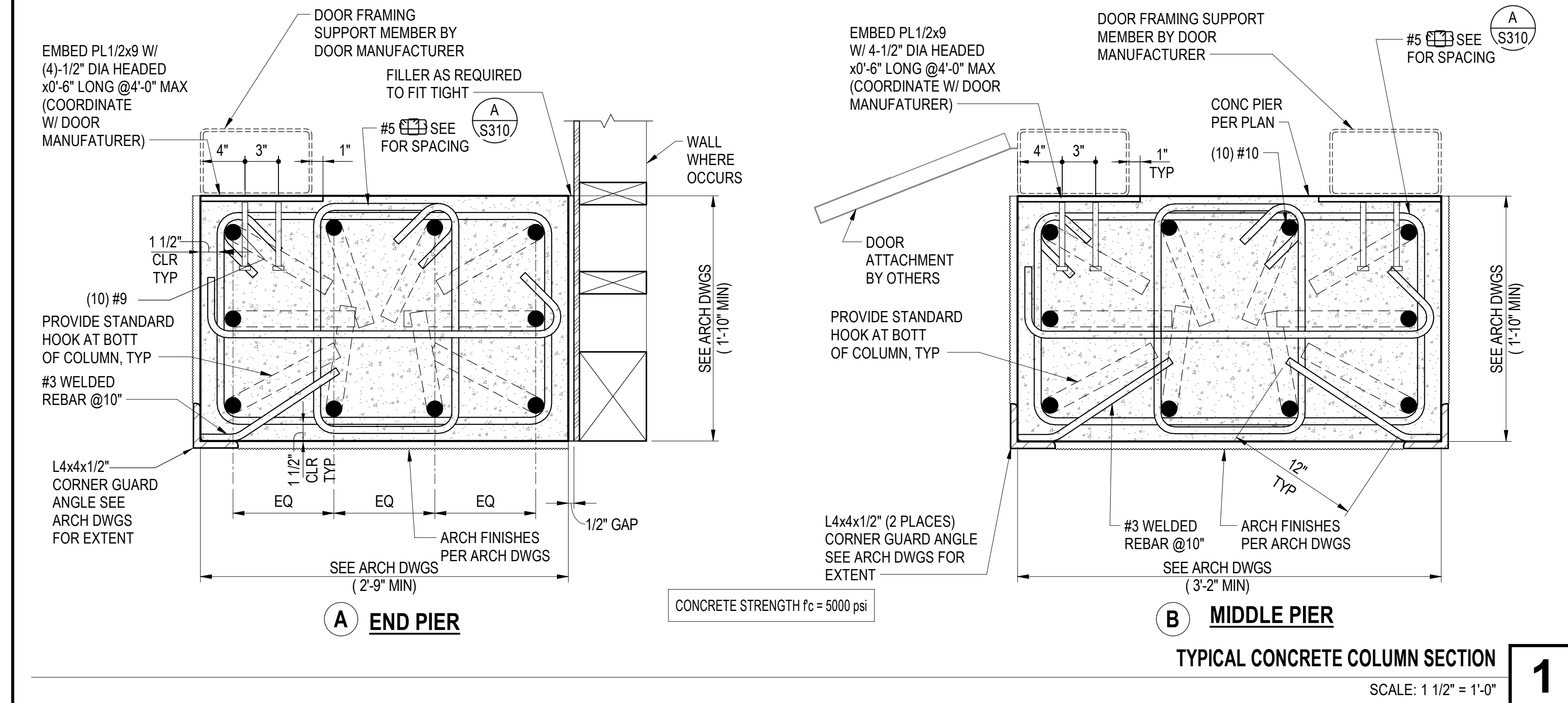
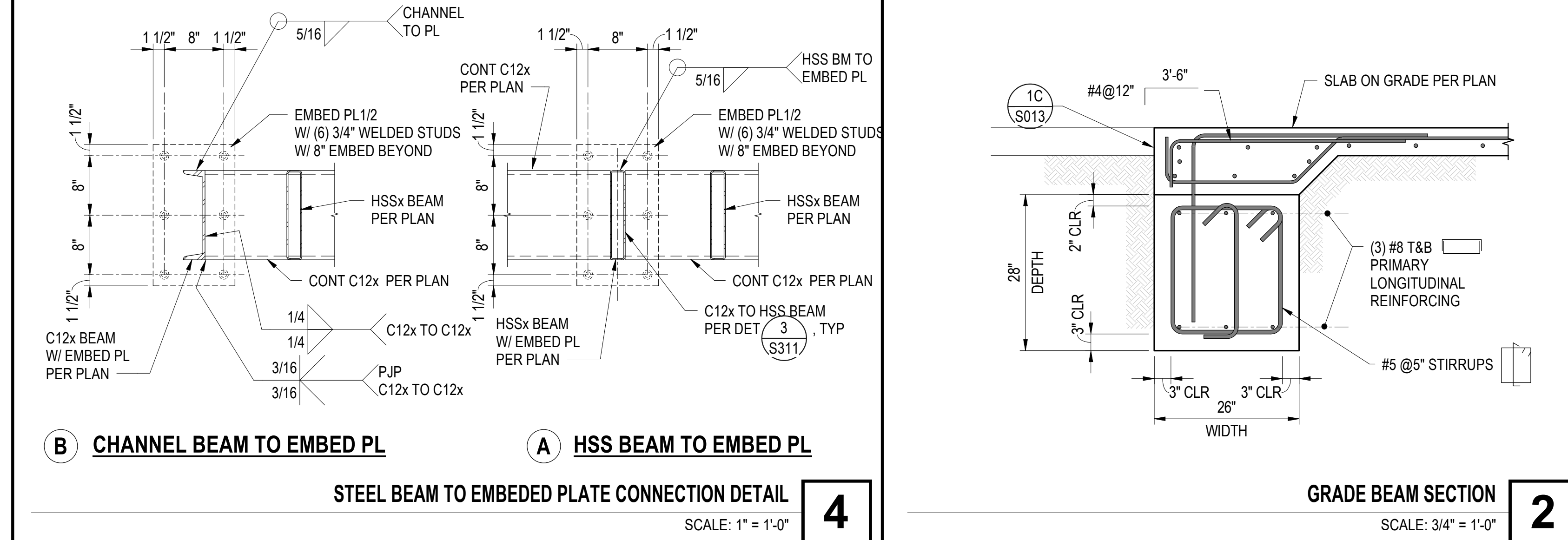
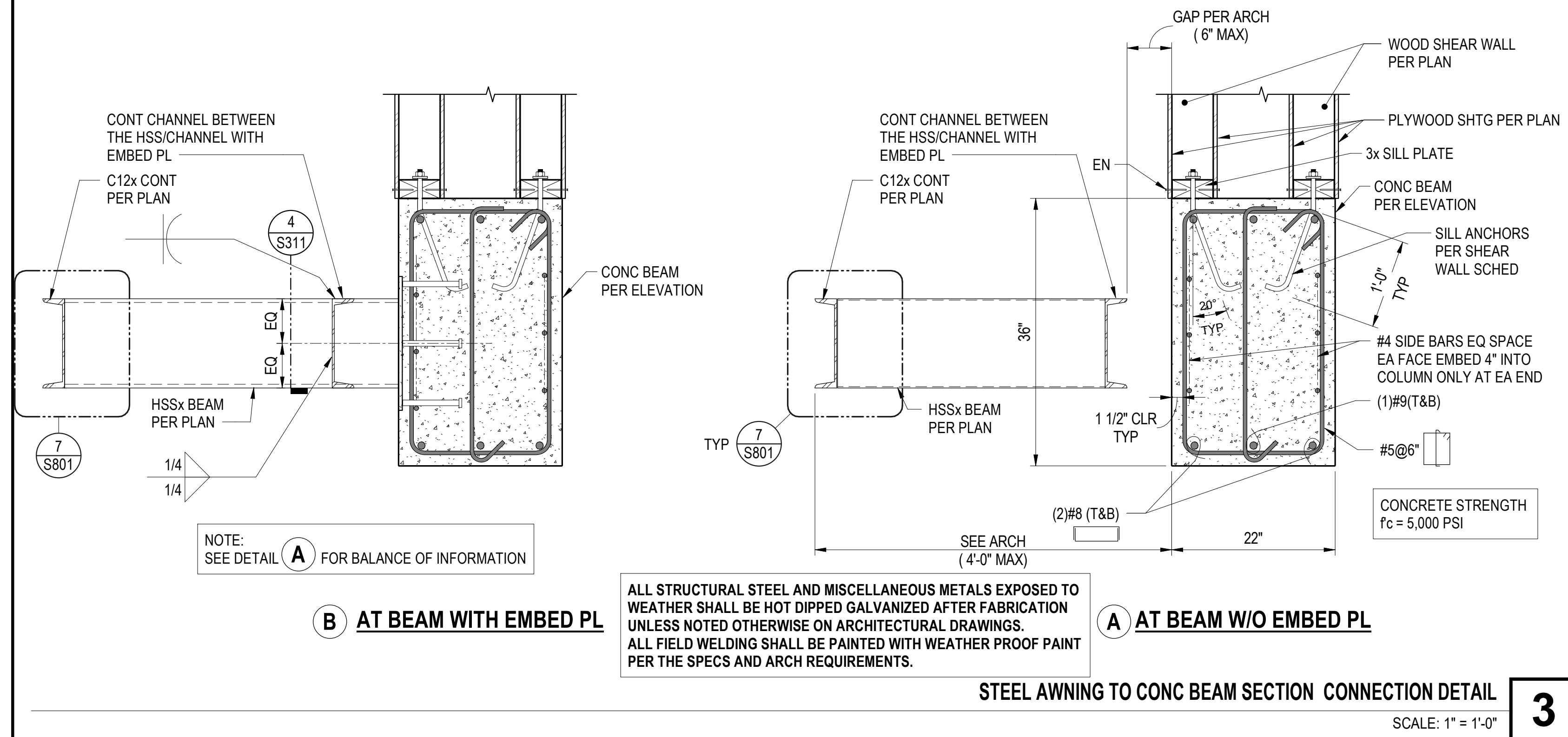


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37th Floor
Los Angeles, CA 90017
www.safilbouquet.com
(213) 315-2277
Project #25534

TEL 310 392 3995



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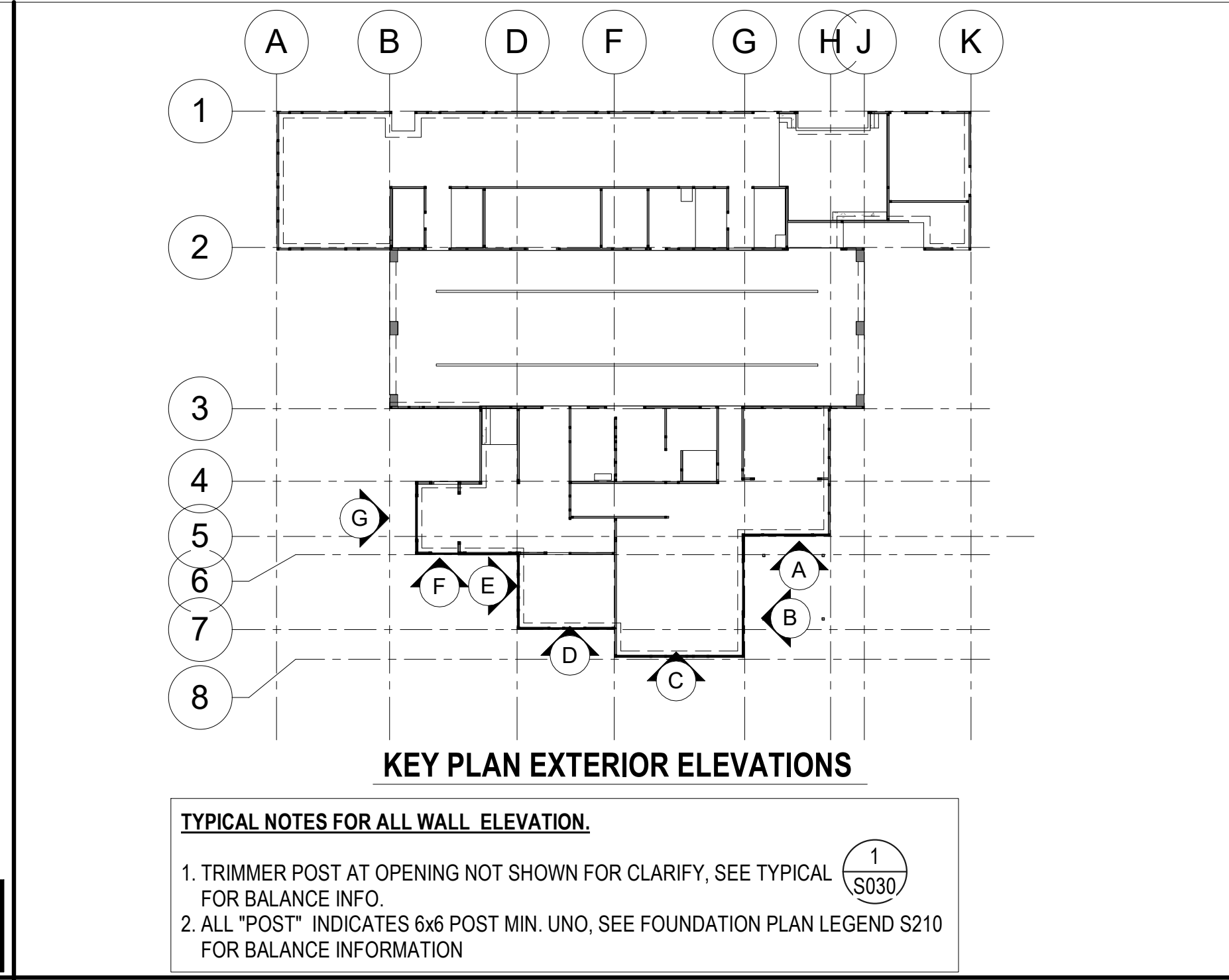
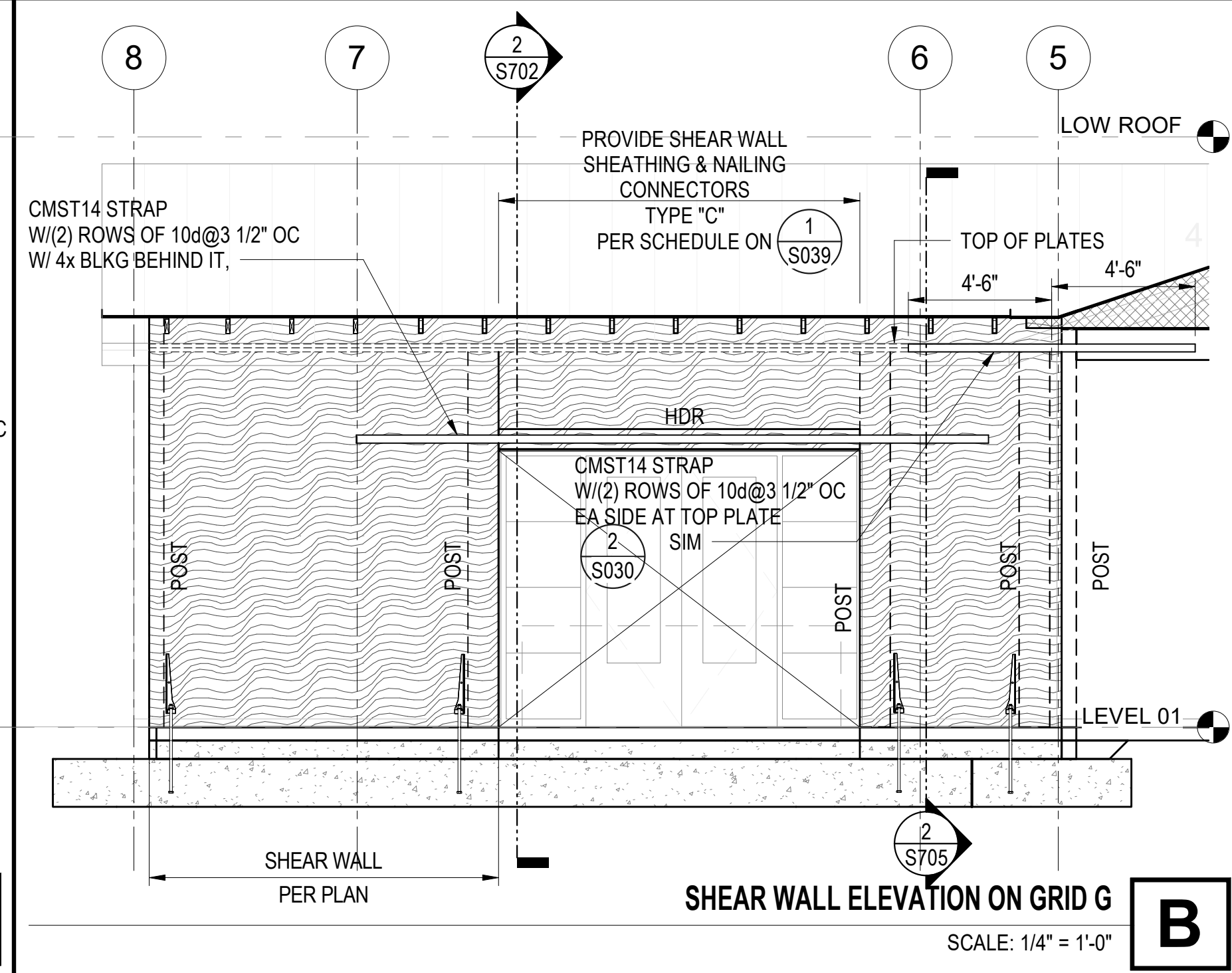
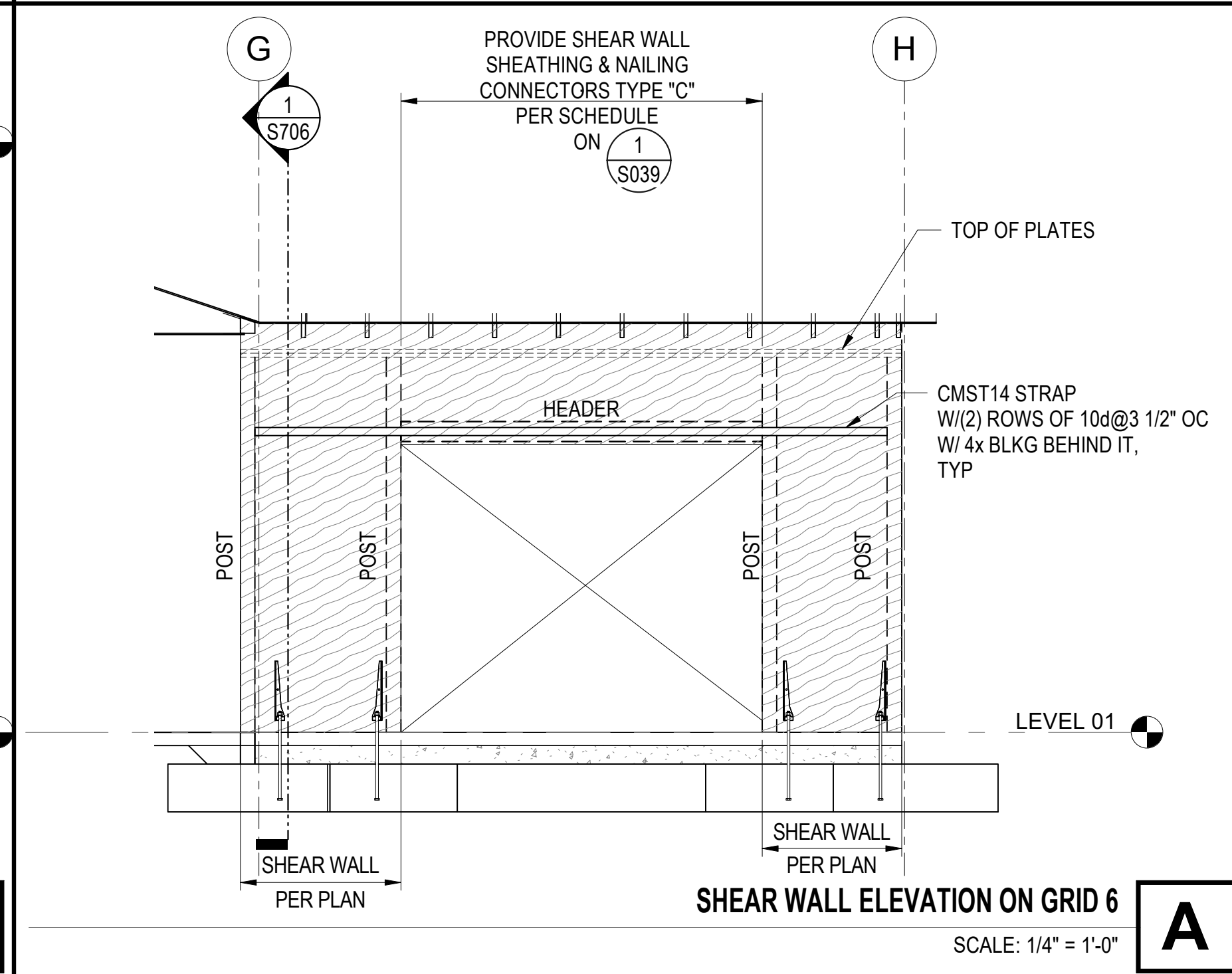
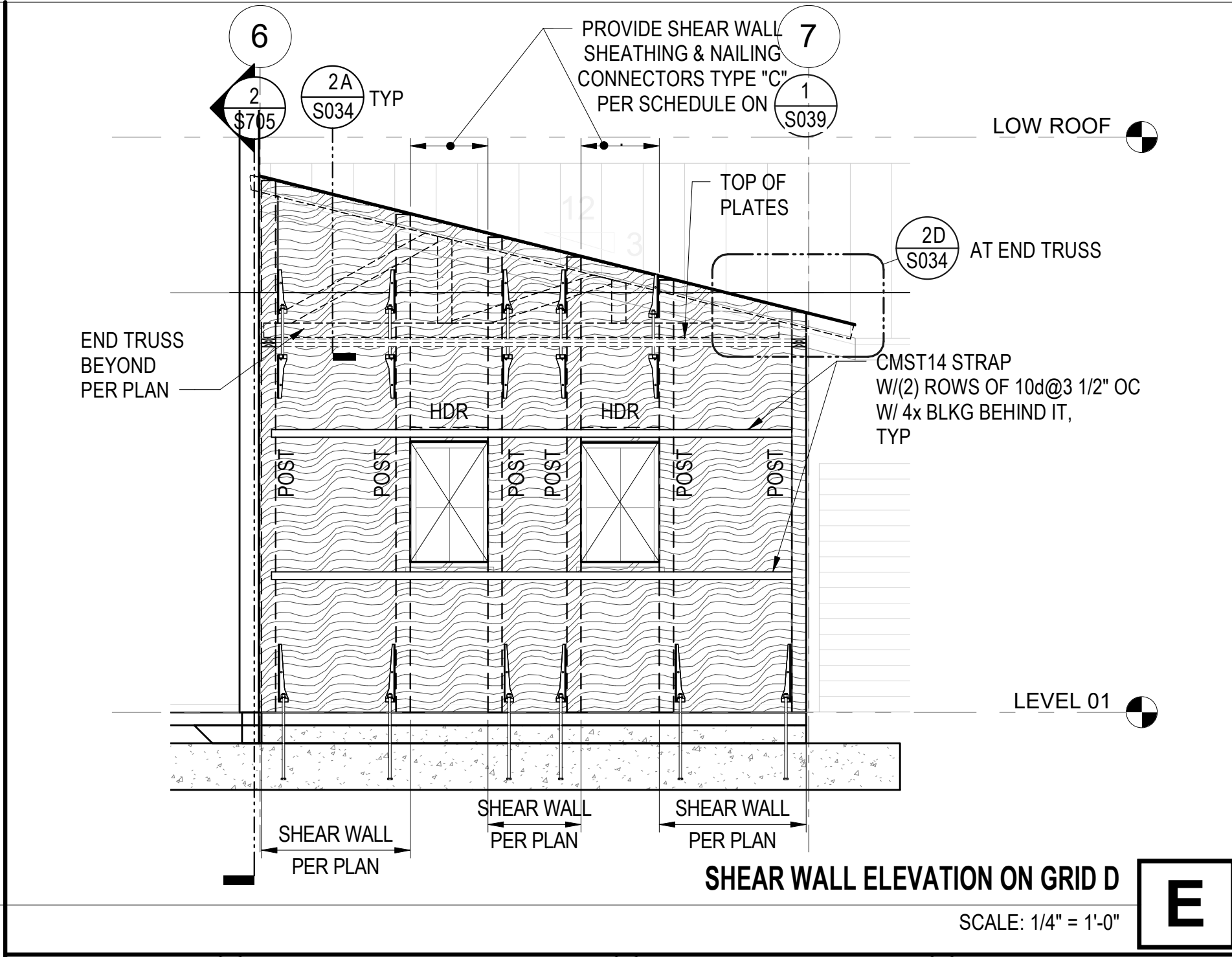
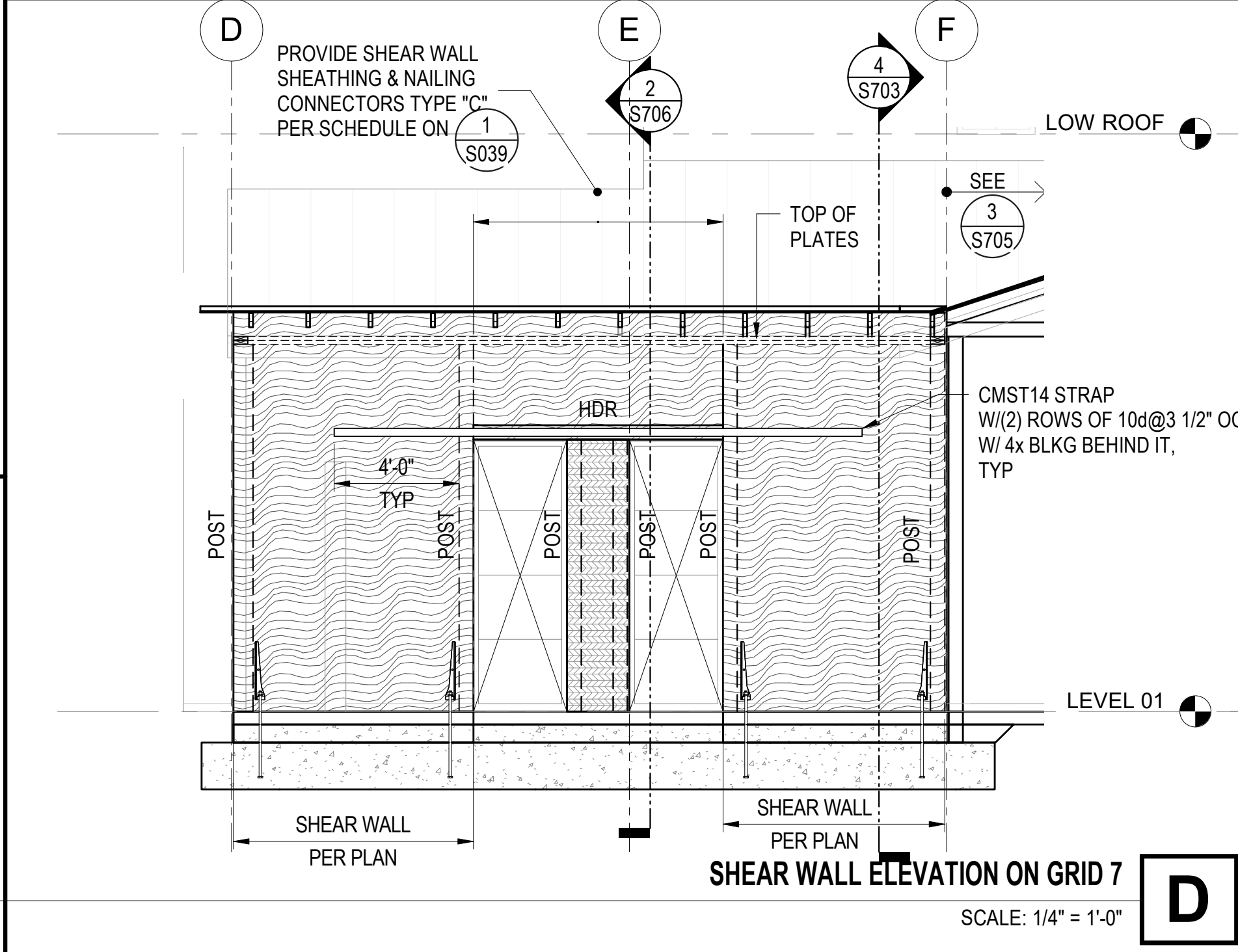
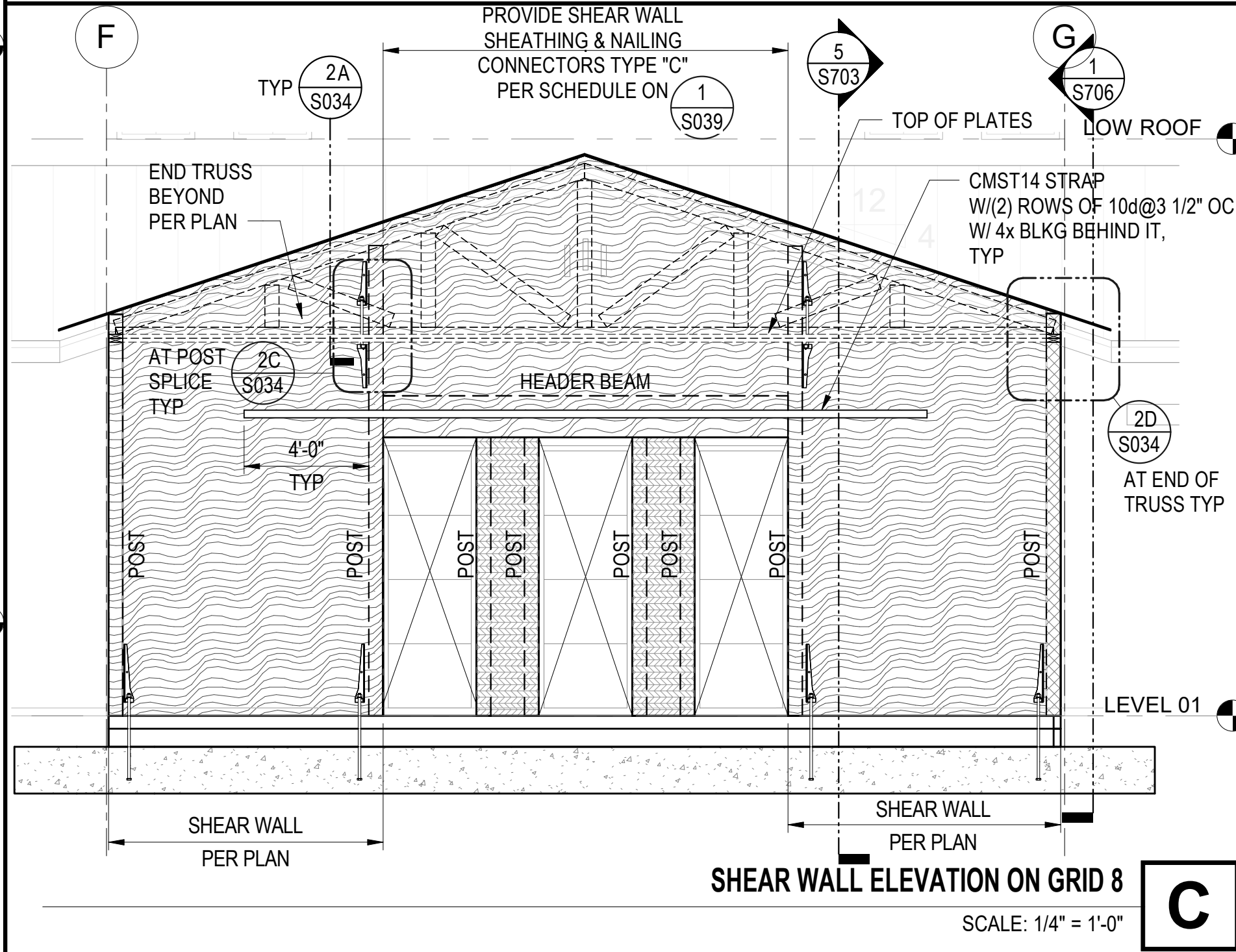
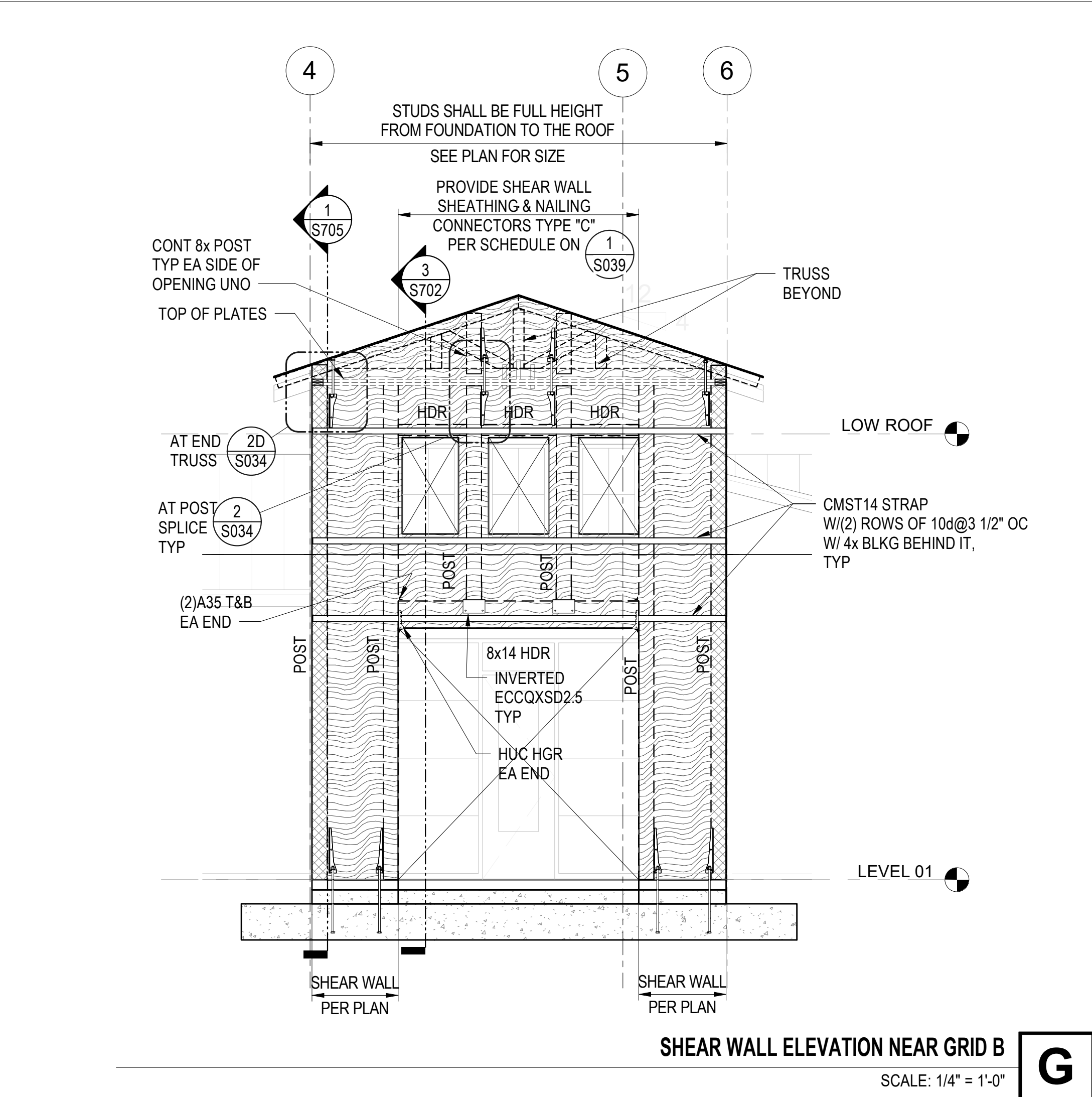
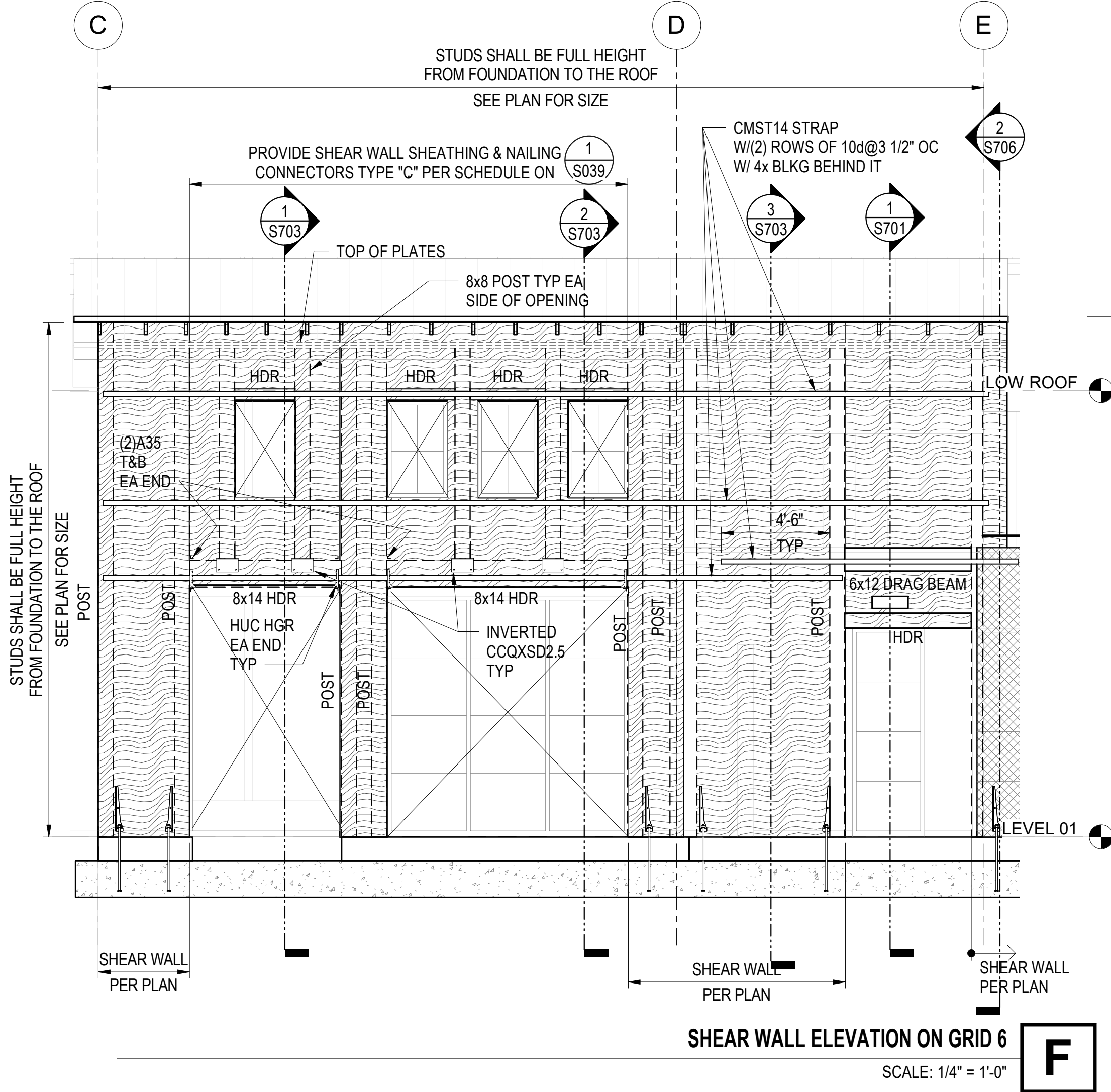
S311



DRAG BEAM TO CONCRETE FRAME CONNECTION DETAIL

SCALE: 1" = 1'-0"

↑



WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232
www.willlloydjones.com
TEL 310 392 3995

SO
structural engineers

726 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.sostructural.com
Project #25534

EXTERIOR WOOD SHEAR
WALL ELEVATIONS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S3289
Exp. 12/31/27
STRUCTURAL
STATE OF CALIFORNIA

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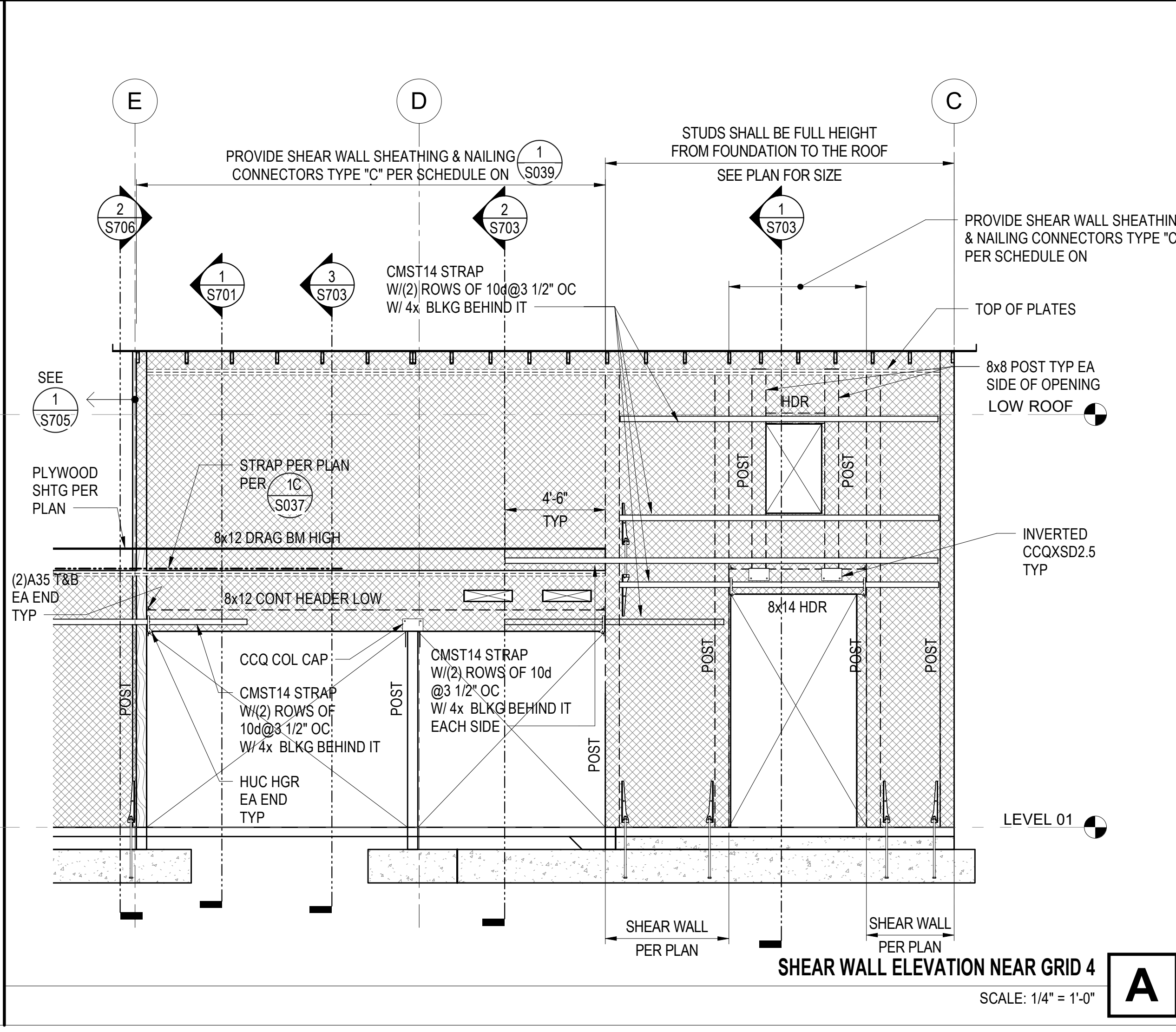
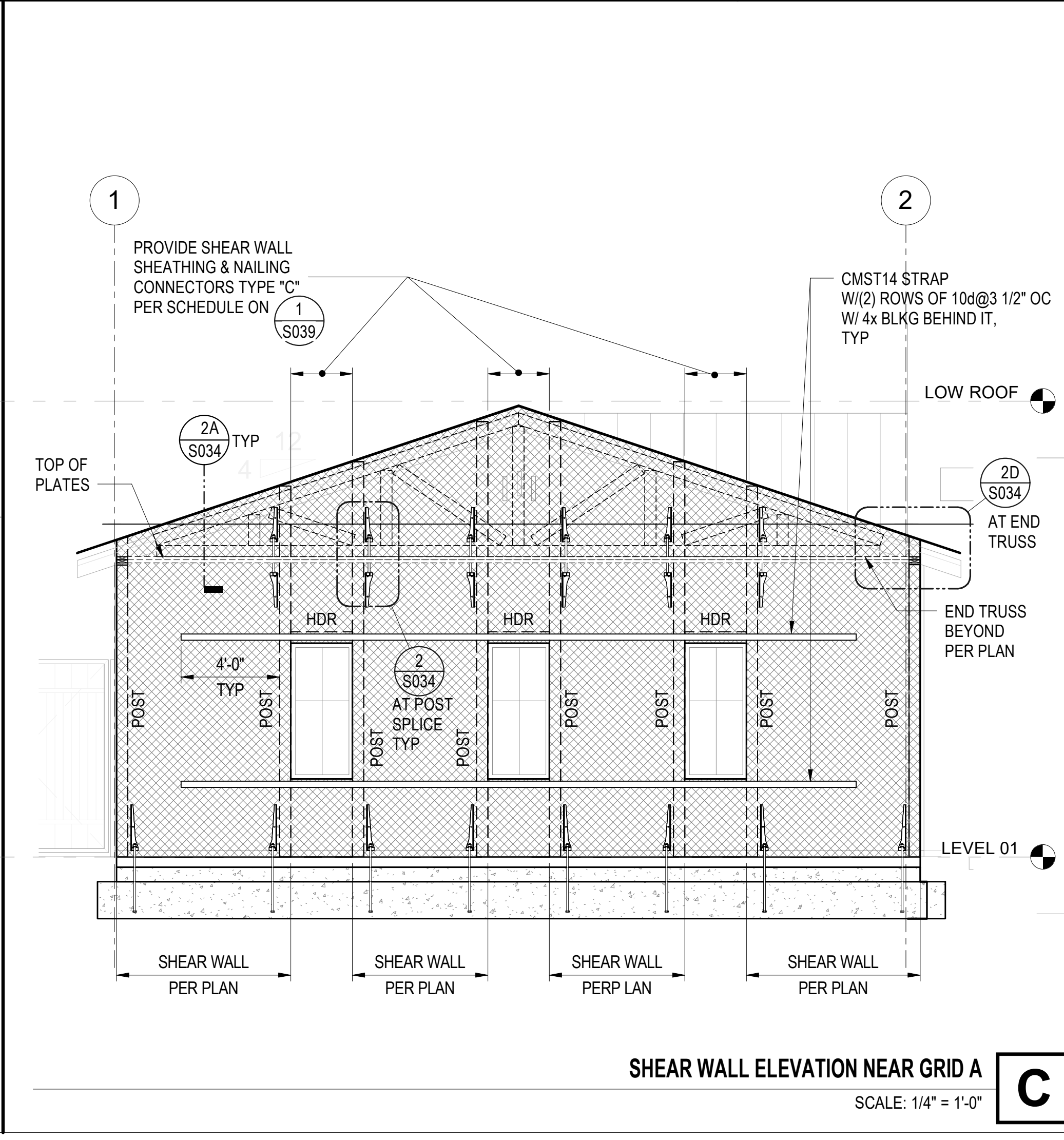
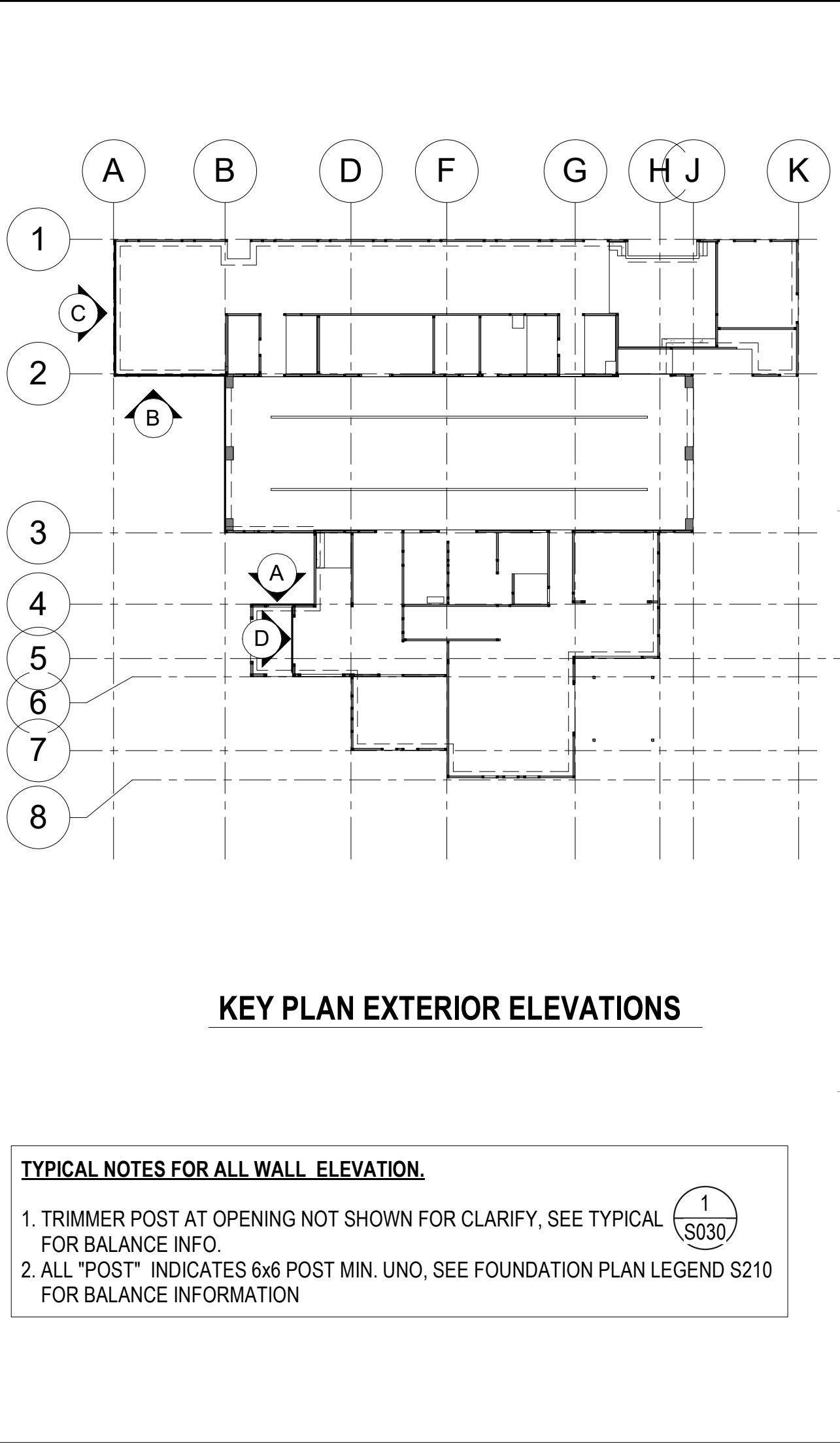
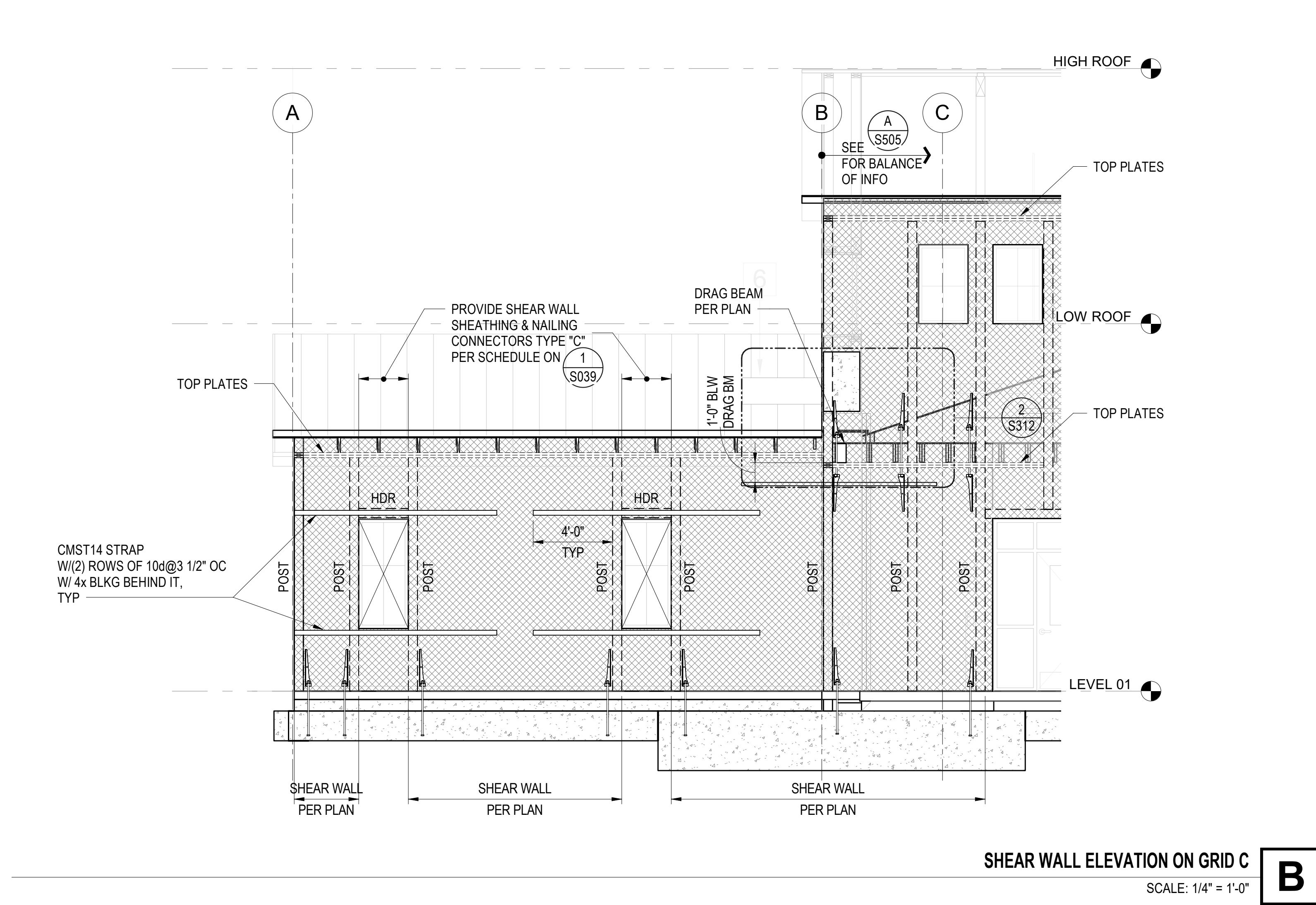
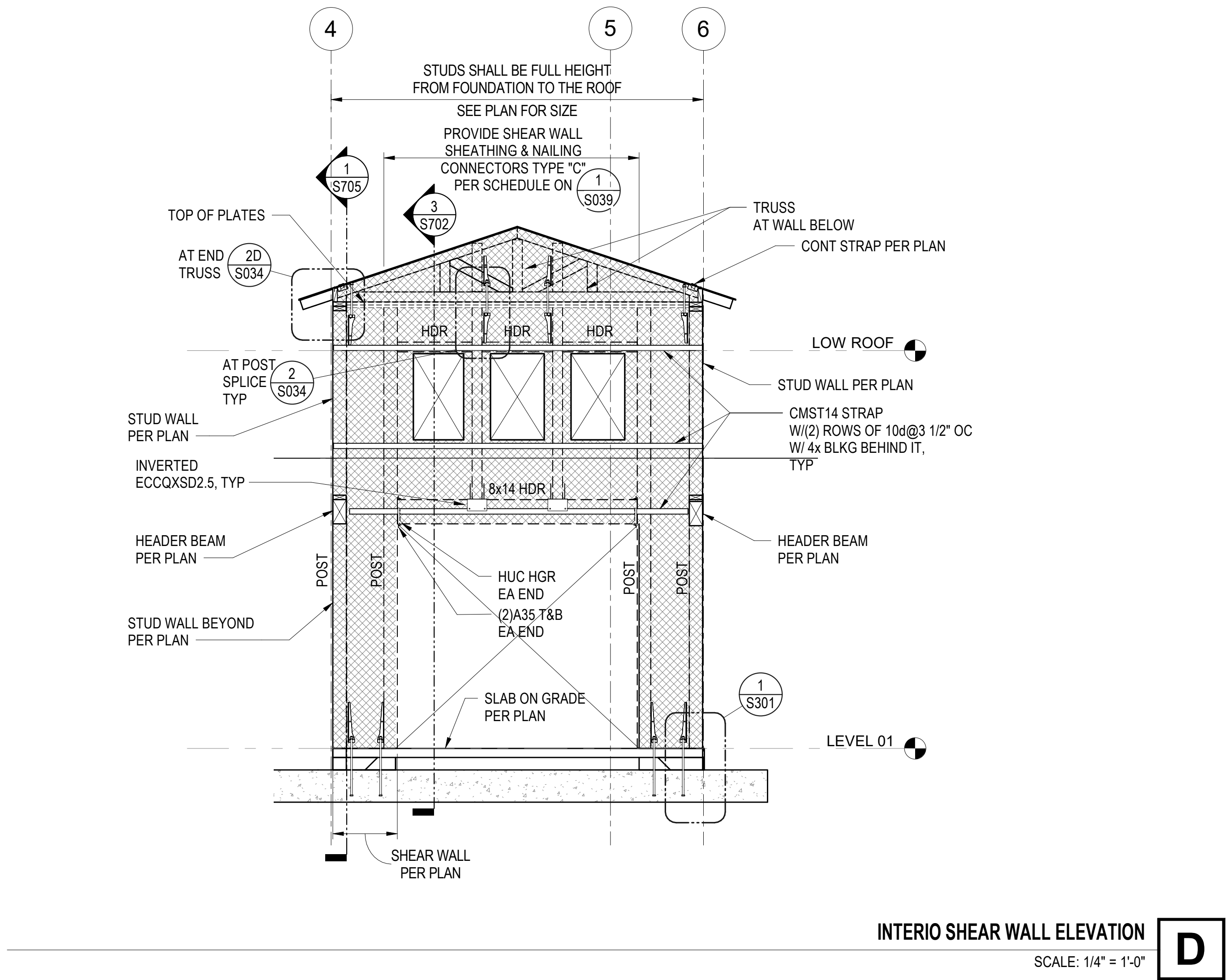
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APPENDIX 5

PLAN CHECK SUBMITTAL - October 31 2025

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WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2
www.willboul.com
TEL 310 392 3995

so
saiful-bouquet
structural engineers

726 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
(213) 315-2277
Project #25534

EXTERIOR WOOD SHEAR
WALL ELEVATIONS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S3289
Exp. 12/31/27
STRUCTURAL
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SCALE
JOB NO.

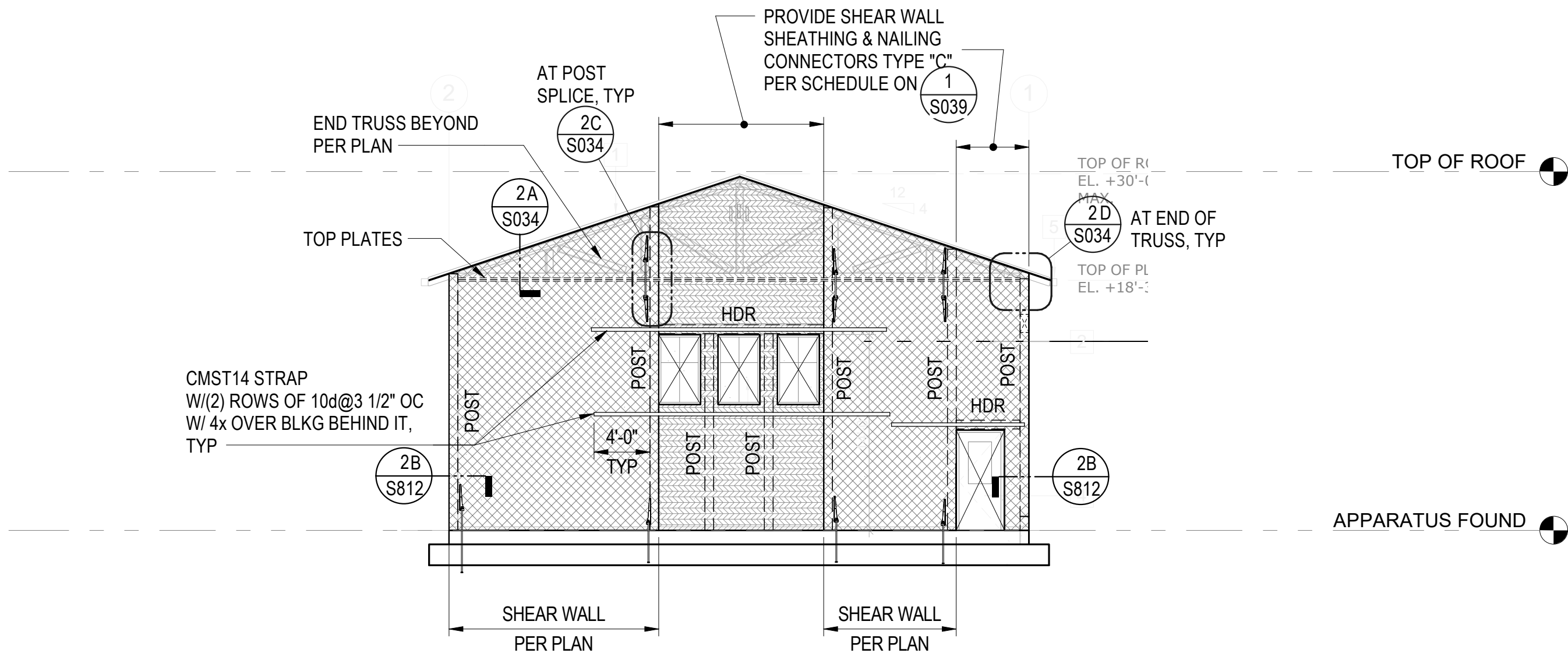
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PROJECT NUMBER

S502

APPENDIX M 5

PLAN CHECK SUBMITTAL - October 31 2025

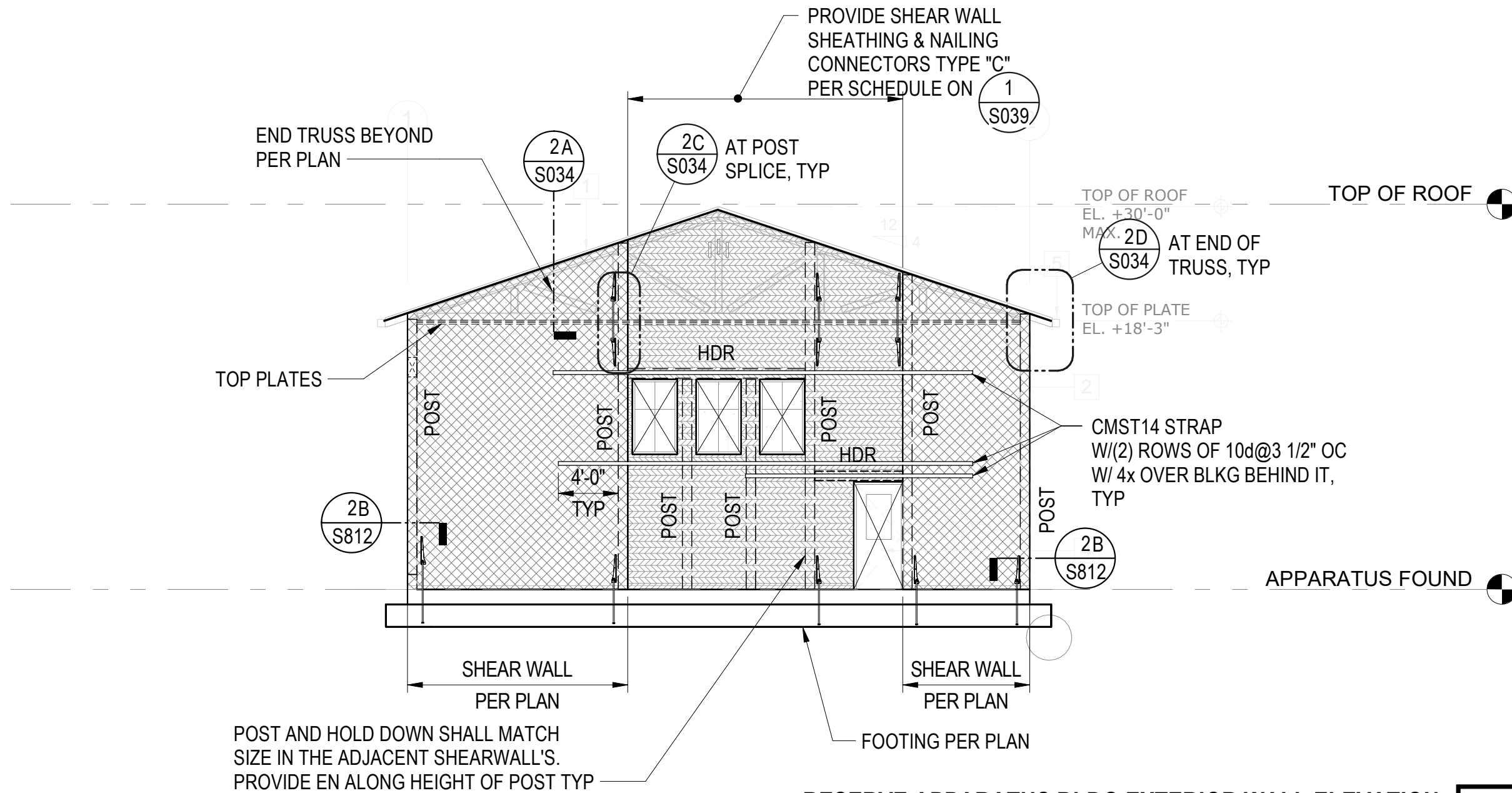
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RESERVE APPARATUS BLDG EXTERIOR WALL ELEVATION

SCALE: 1/8" = 1'-0"

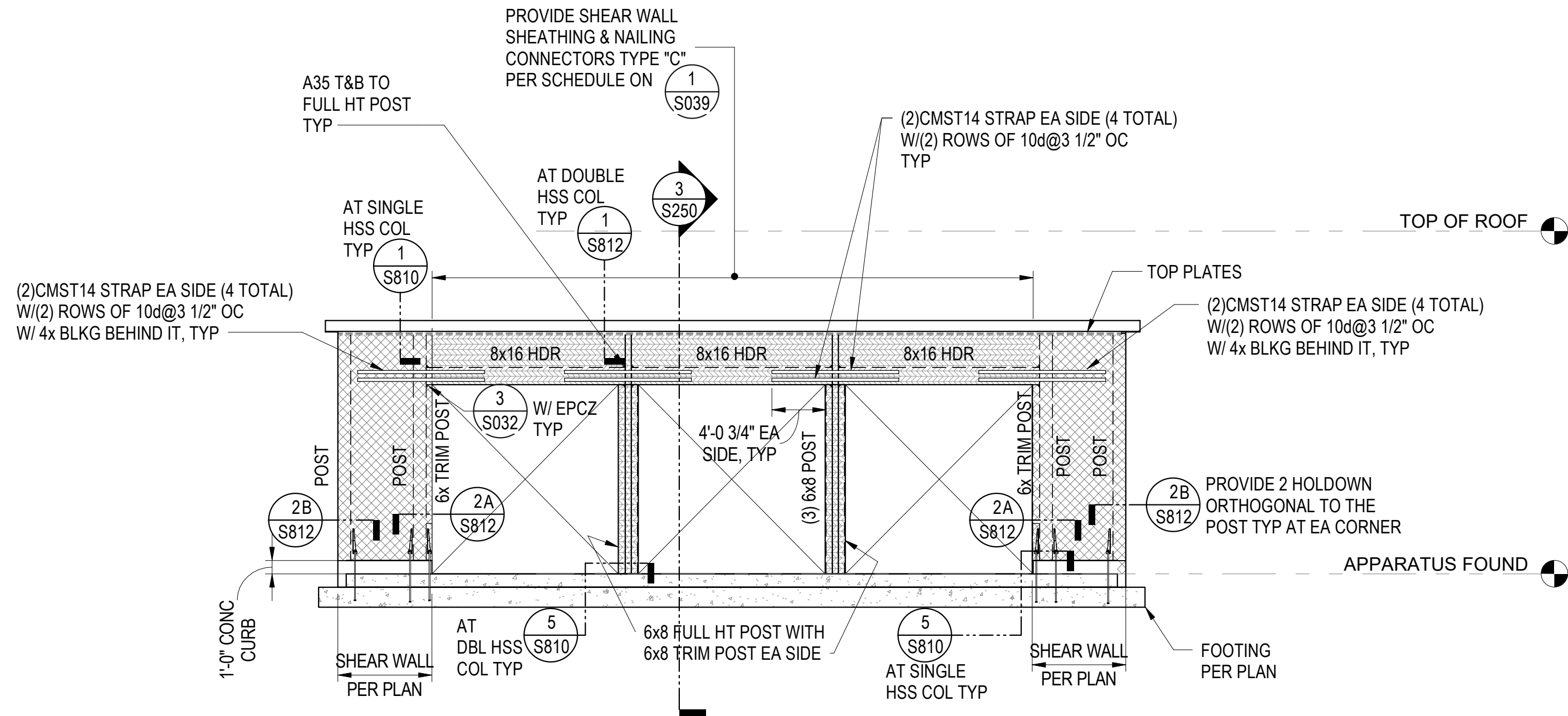
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RESERVE APPARATUS BLDG EXTERIOR WALL ELEVATION

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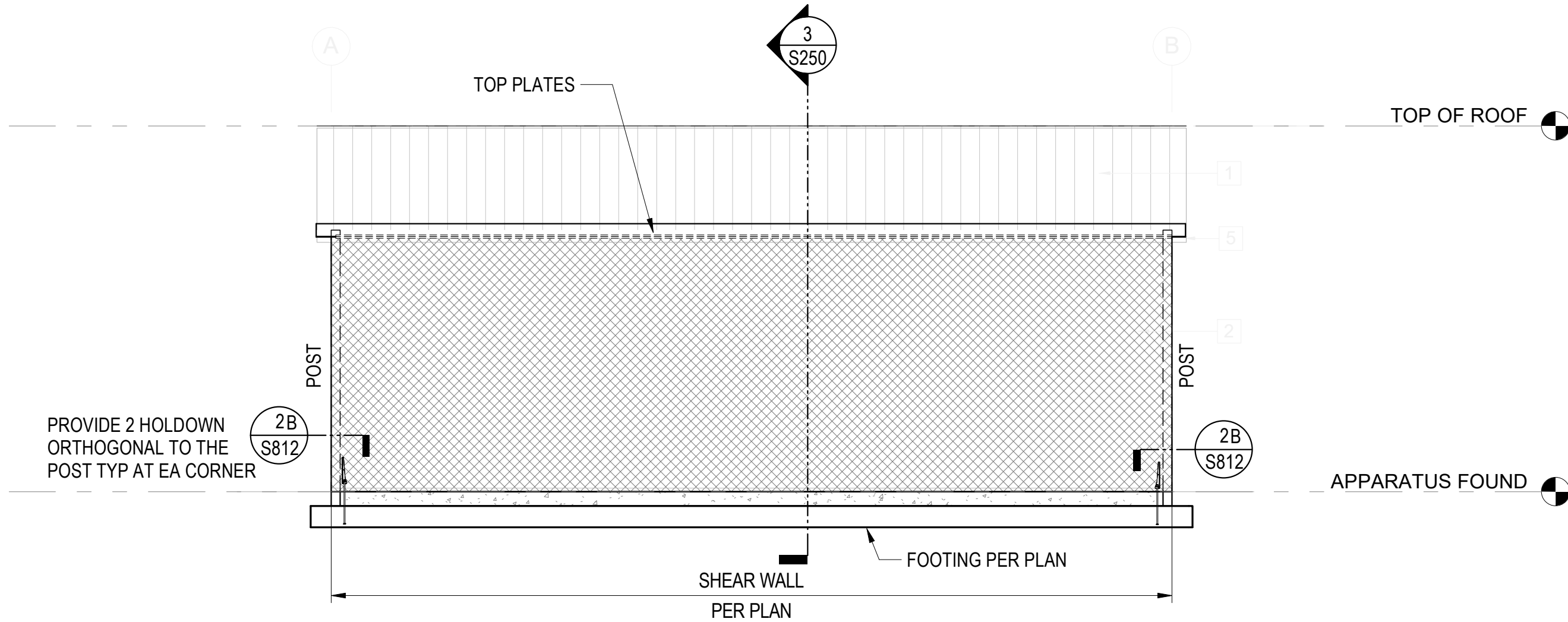
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RESERVE APPARATUS BLDG EXTERIOR WALL ELEVATION

SCALE: 1/8" = 1'-0"

C



RESERVE APPARATUS BLDG EXTERIOR WALL ELEVATION

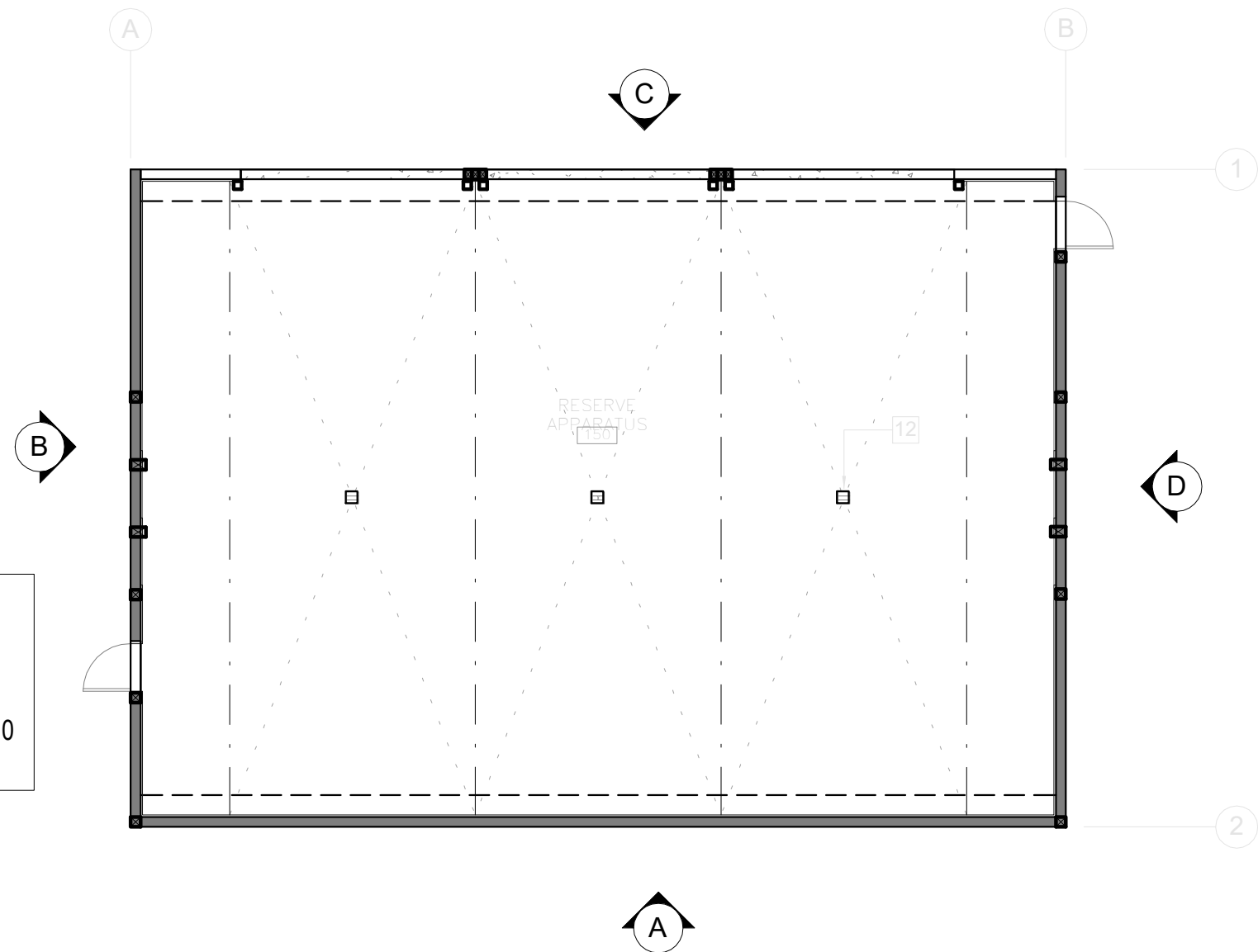
SCALE: 1/8" = 1'-0"

A

TYPICAL NOTES FOR ALL WALL ELEVATION.

1. TRIMMER POST AT OPENING NOT SHOWN FOR CLARIFY, SEE TYPICAL FOR BALANCE INFO.
2. ALL "POST" INDICATES 6x6 POST MIN. UNO, SEE FOUNDATION PLAN LEGEND S210 FOR BALANCE INFORMATION

1 S030



KEY PLAN EXTERIOR ELEVATIONS

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

SO
saiful-bouquet
structural engineers

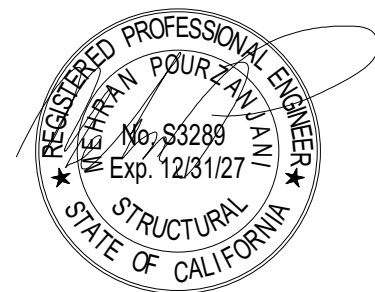
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RESERVE APPARATUS
BLDG ELEVATIONS

FIRE STATION 46

MISSION VILLAGE

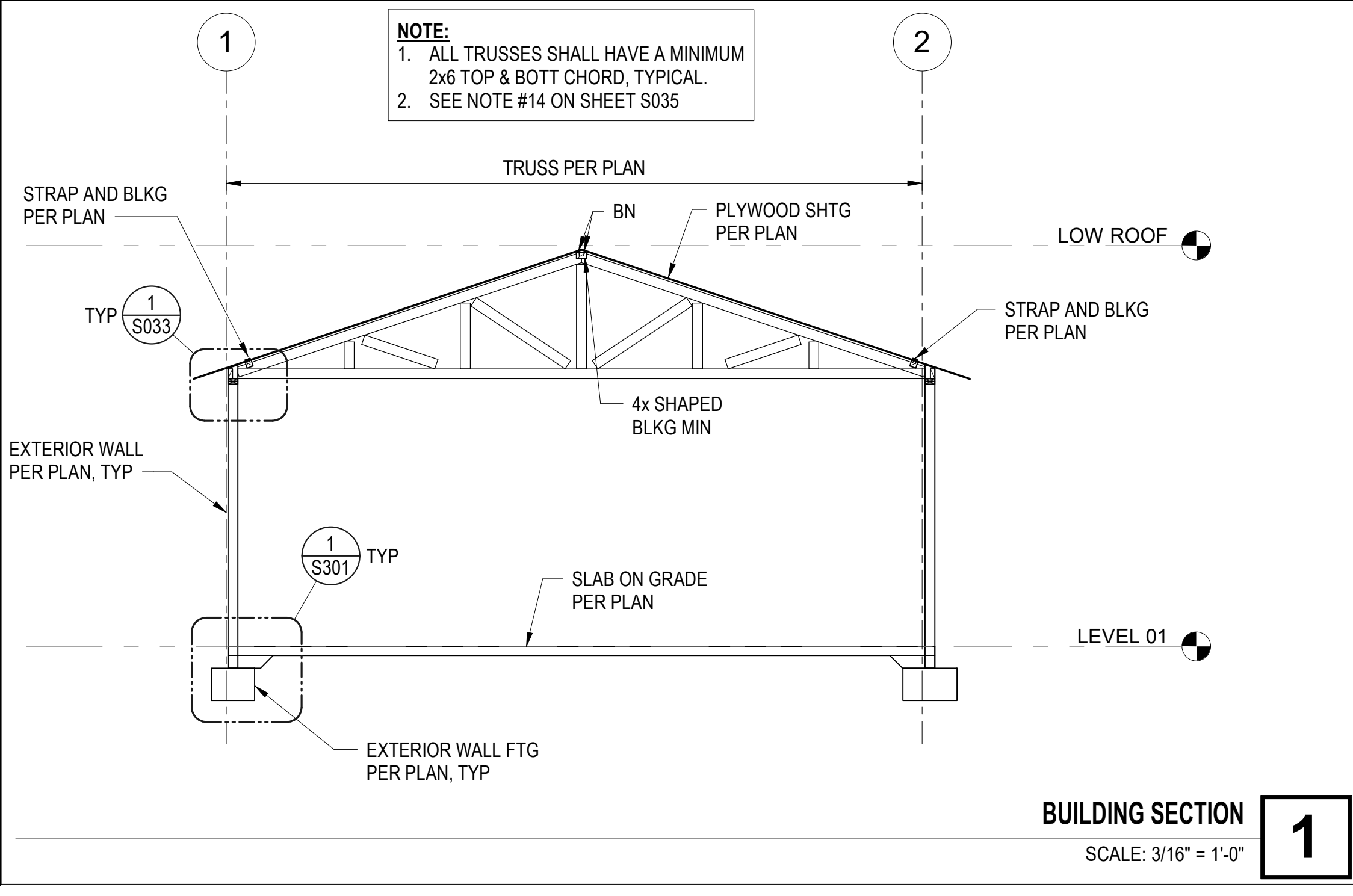
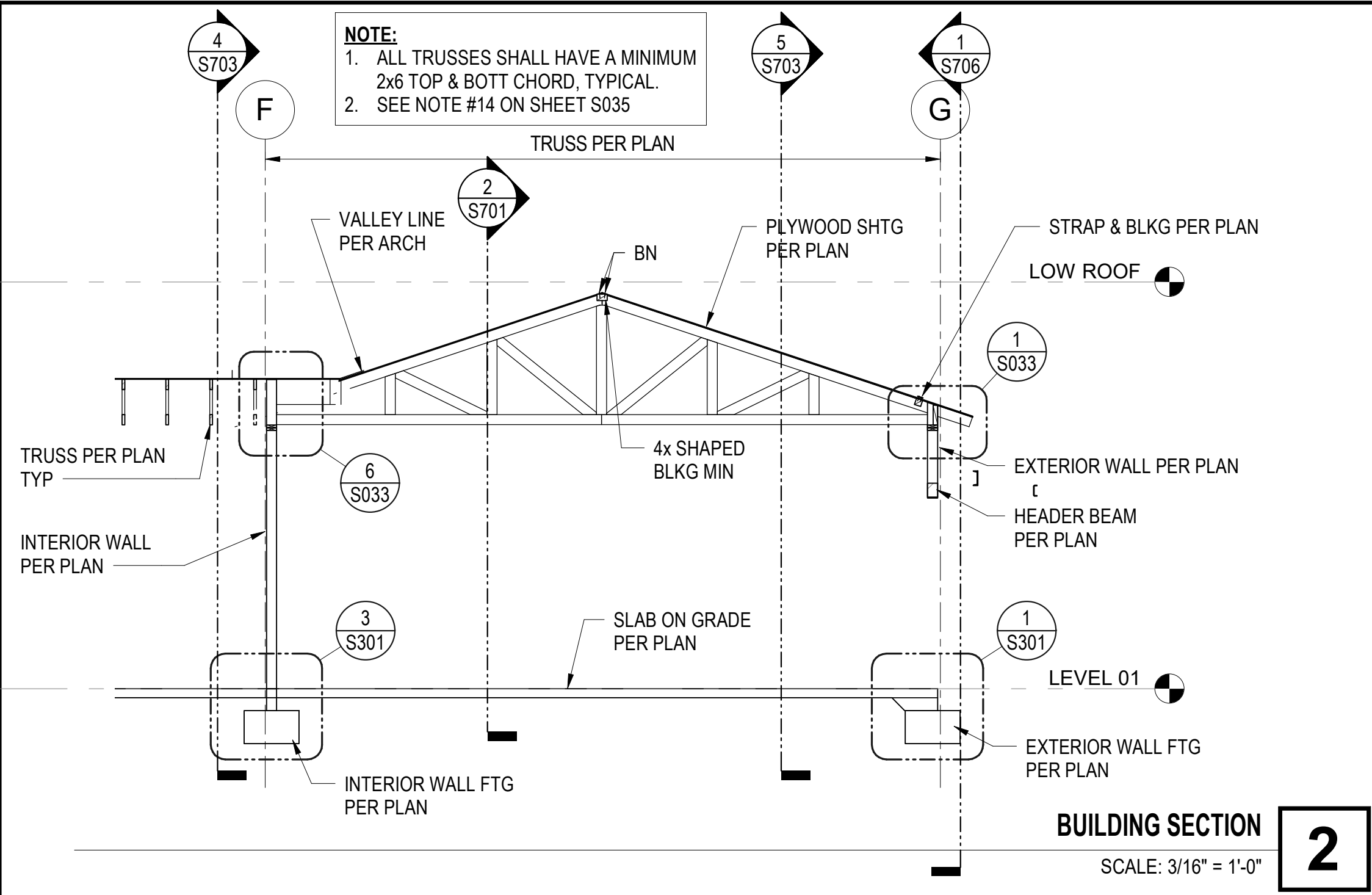
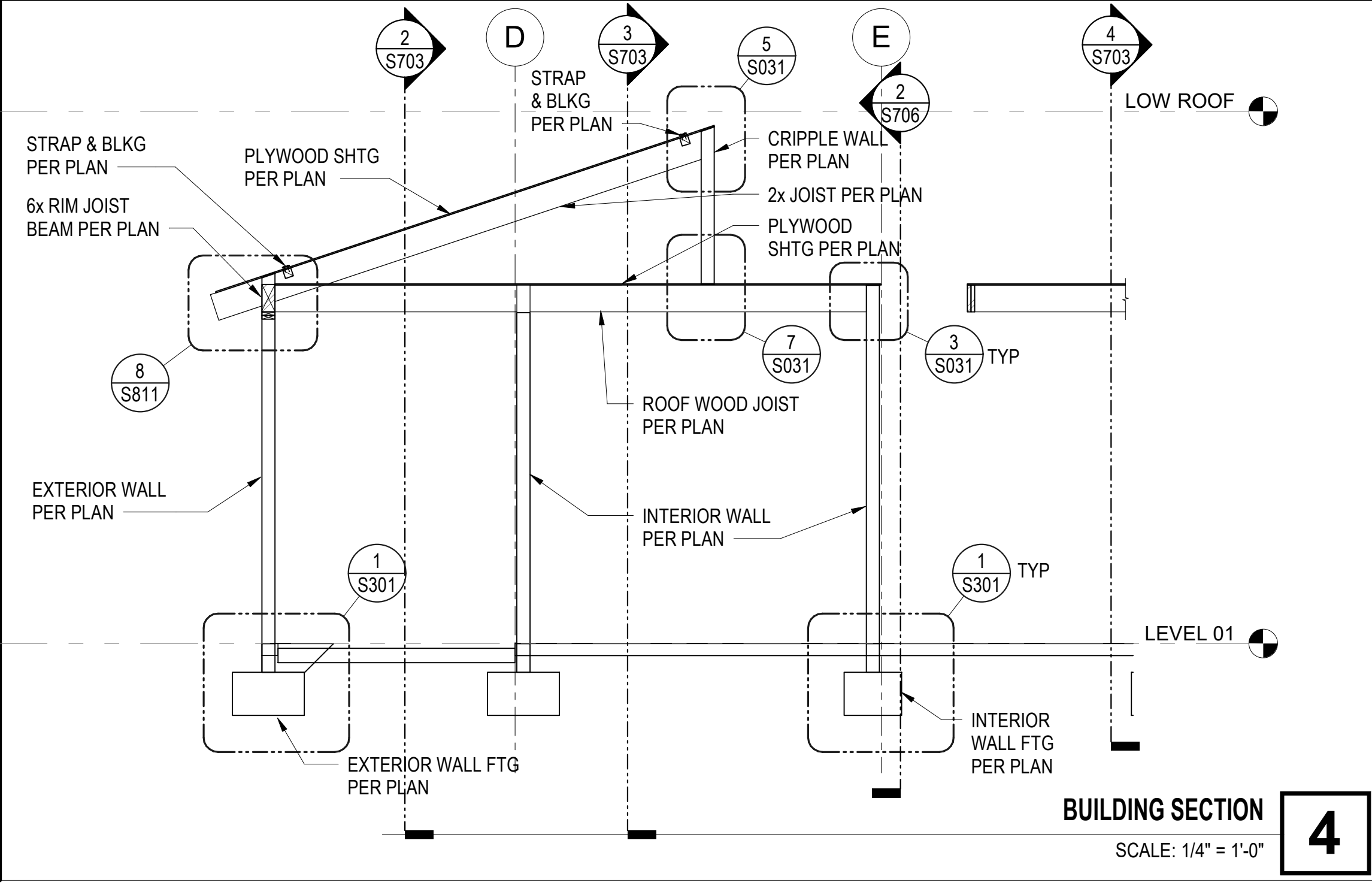
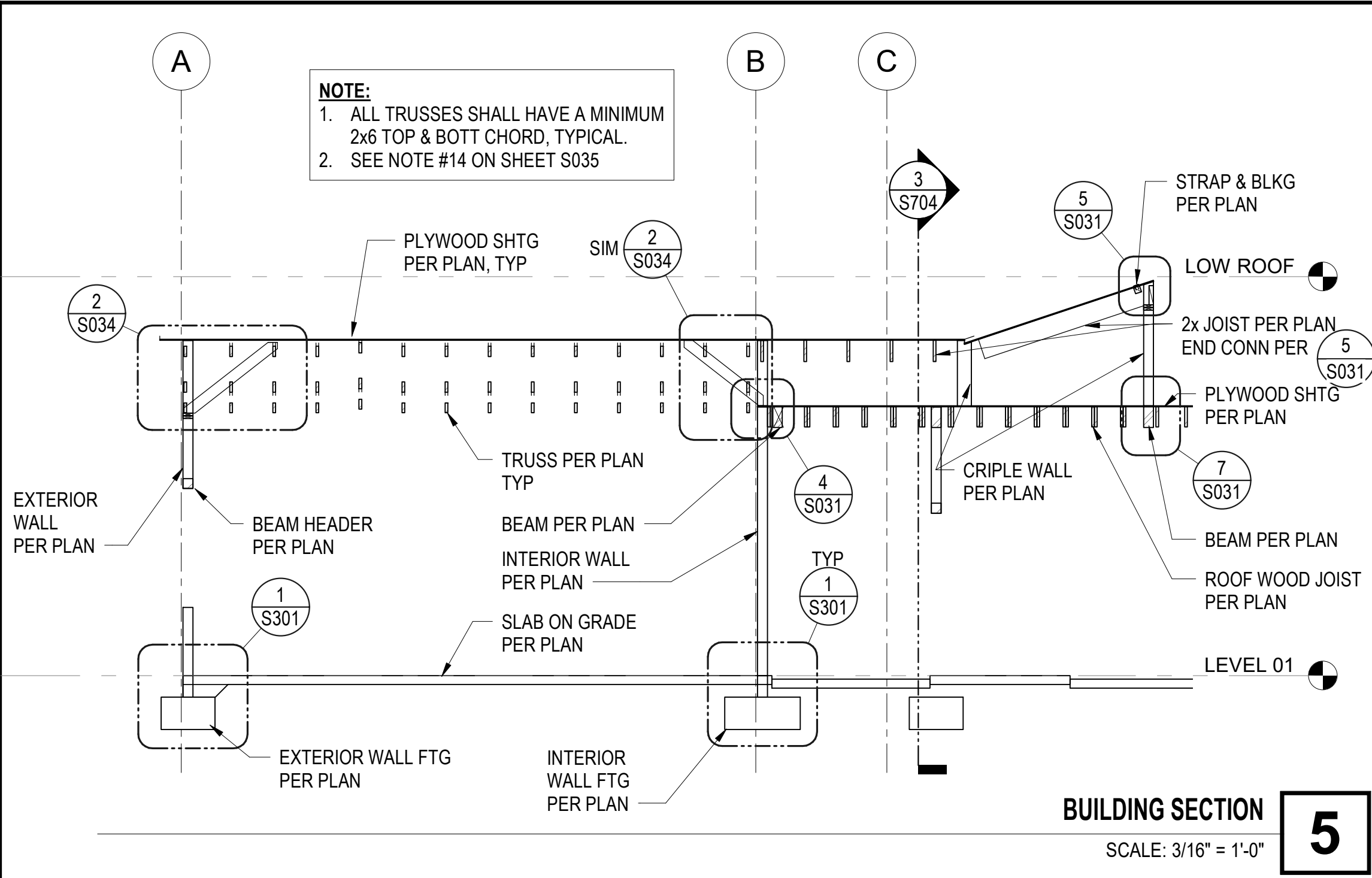
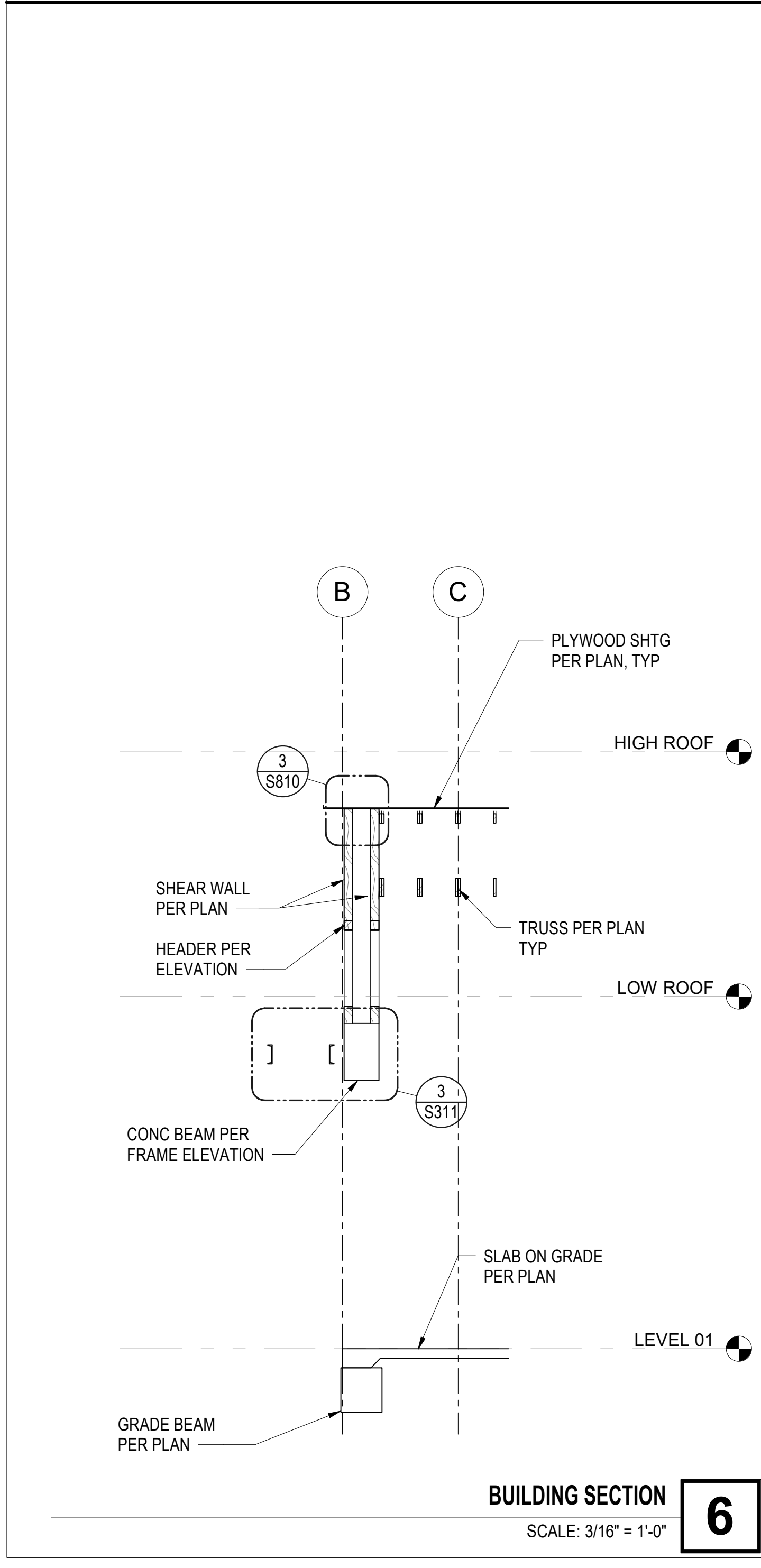
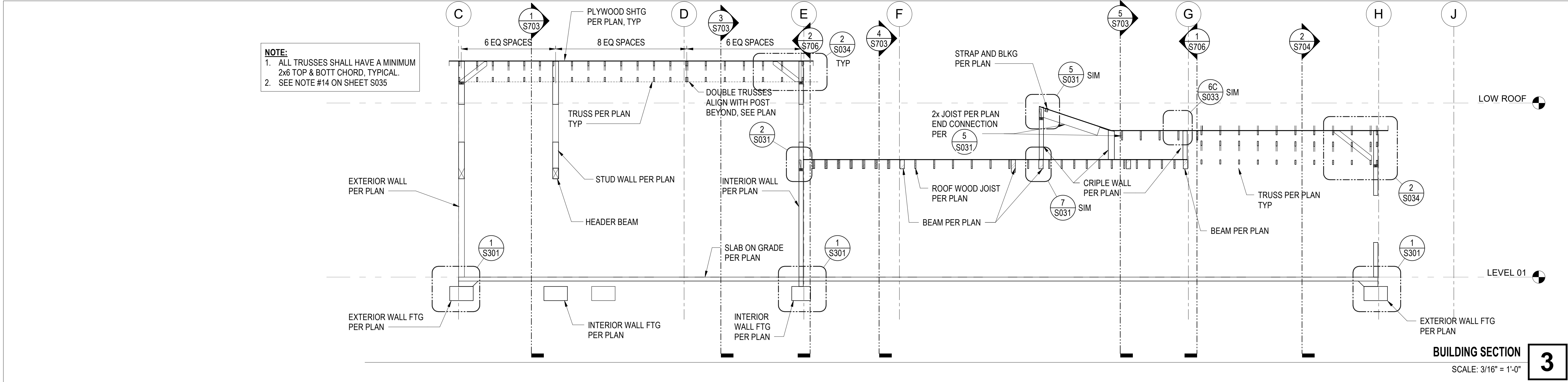
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WILLIAM LOYD JONES ARCHITECT
9415 culver boulevard
culver city, california 90232
TEL 310 392 3995

SO saiful-bouquet structural engineers
725 S. Figueroa St.,
37th floor, Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

BUILDING SECTIONS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S3289
Exp. 12/31/27
STRUCTURAL
STATE OF CALIFORNIA

APPENDIX 5

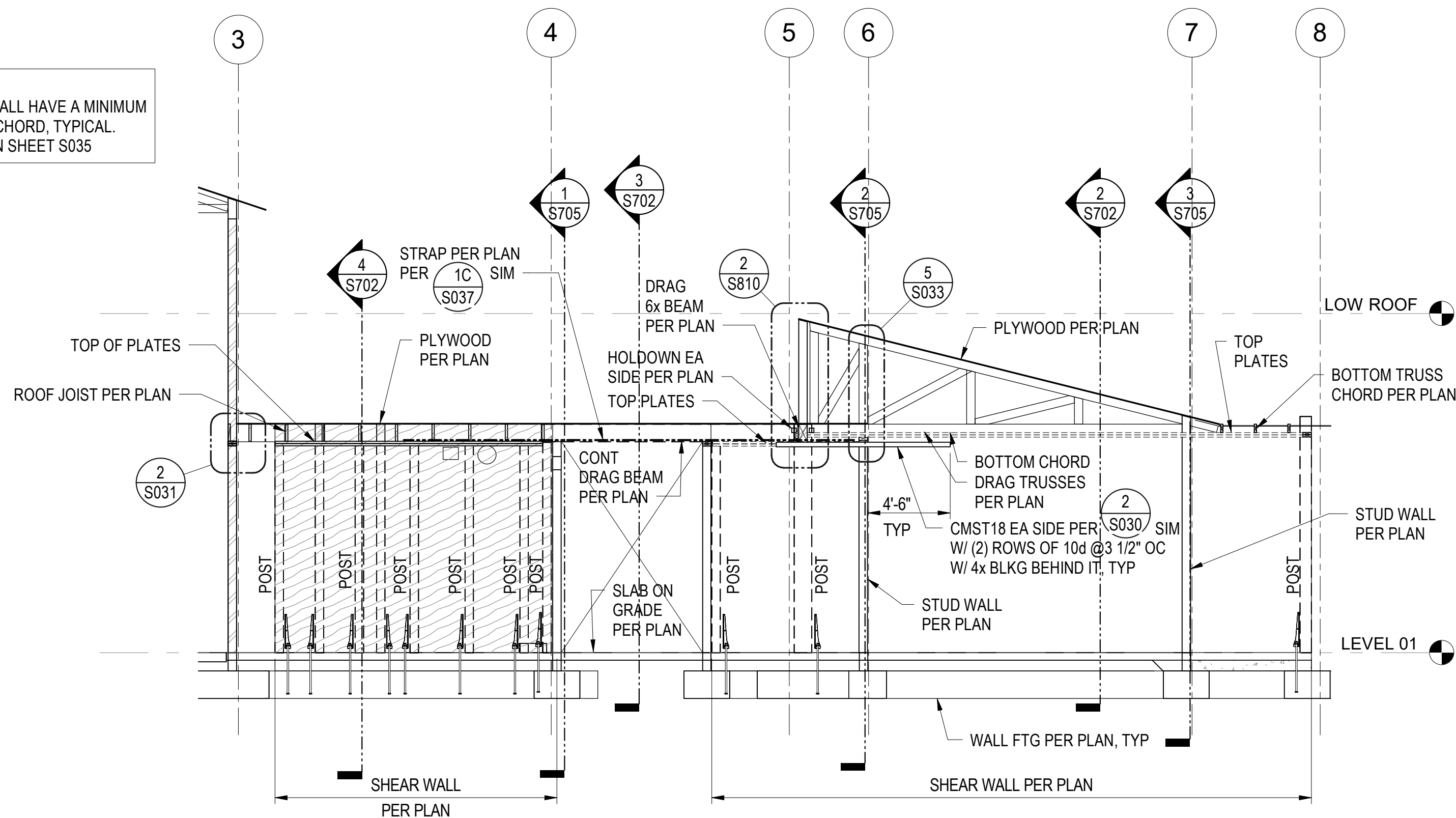
S702

DATE _____ **ISSUE DATE** _____
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CHECKED _____
SCALE _____ **AS NOTED** _____
JOB. NO. _____ **PROJECT NUMBER** _____

PLAN CHECK SUBMITTAL - October 31 2025

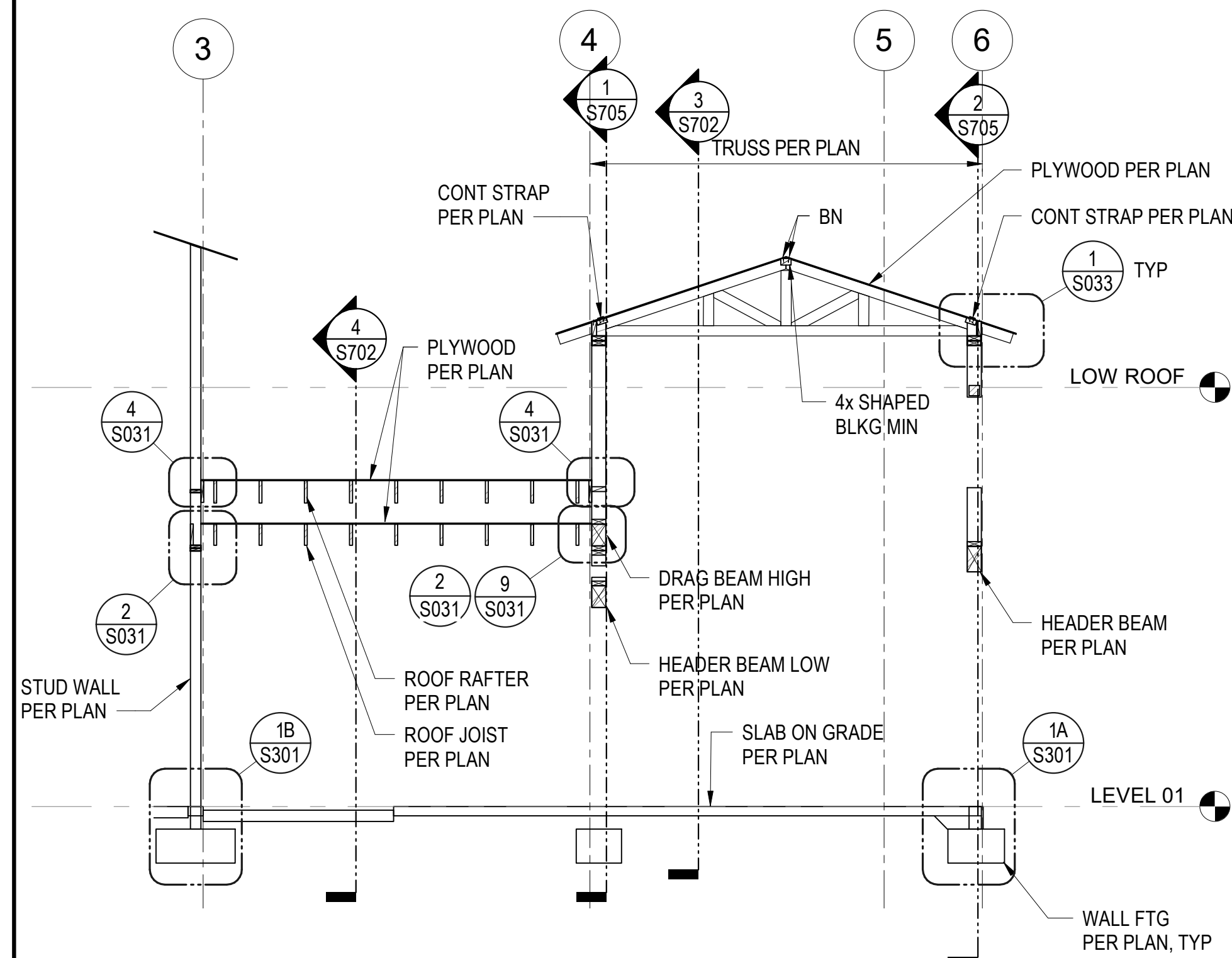
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NOTE:
1. ALL TRUSSES SHALL HAVE A MINIMUM
2x6 TOP & BOTT CHORD, TYPICAL.
2. SEE NOTE #14 ON SHEET S035



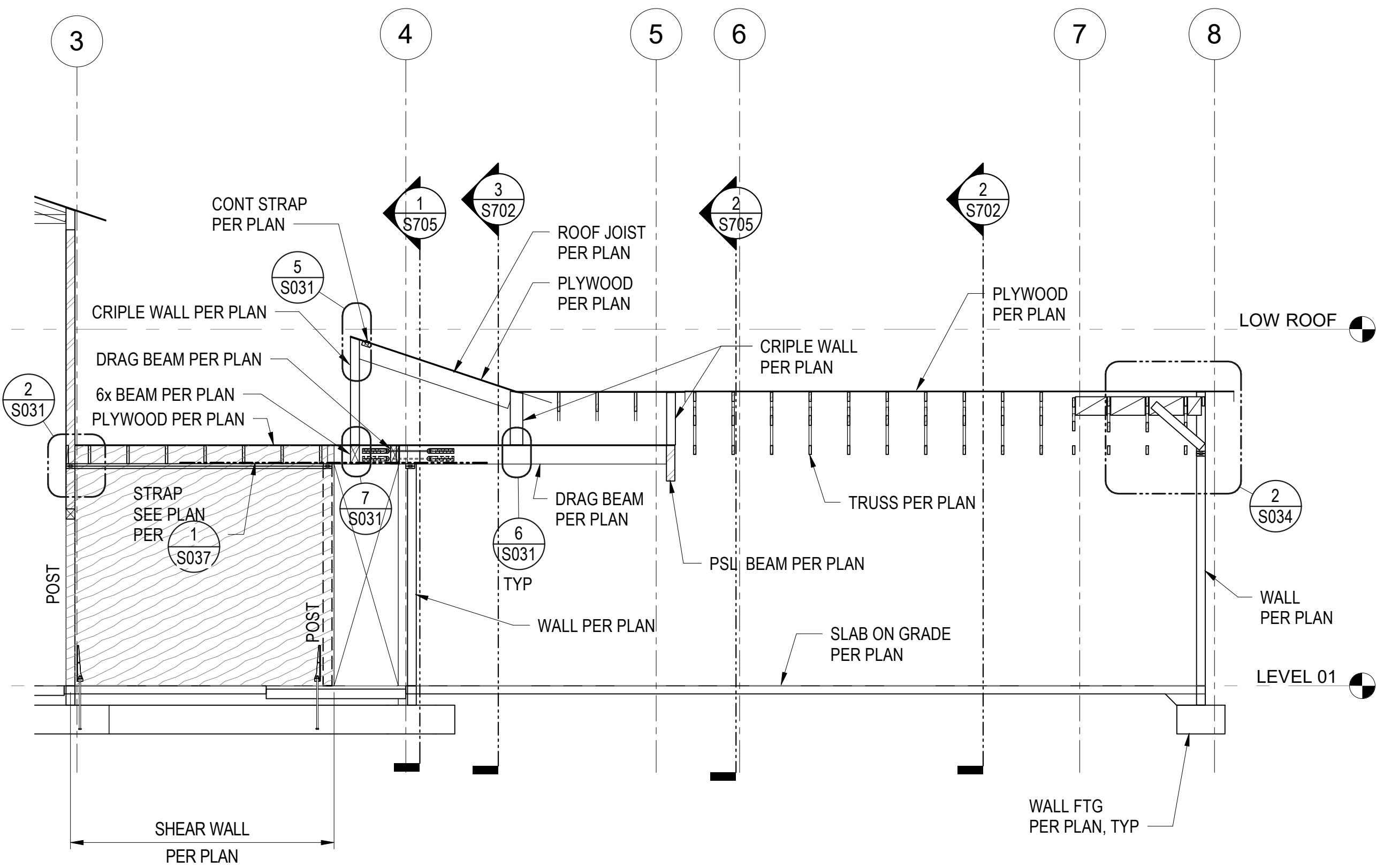
BUILDING SECTION 4
SCALE: 3/16" = 1'-0"

NOTE:
1. ALL TRUSSES SHALL HAVE A MINIMUM
2x6 TOP & BOTT CHORD, TYPICAL.
2. SEE NOTE #14 ON SHEET S035



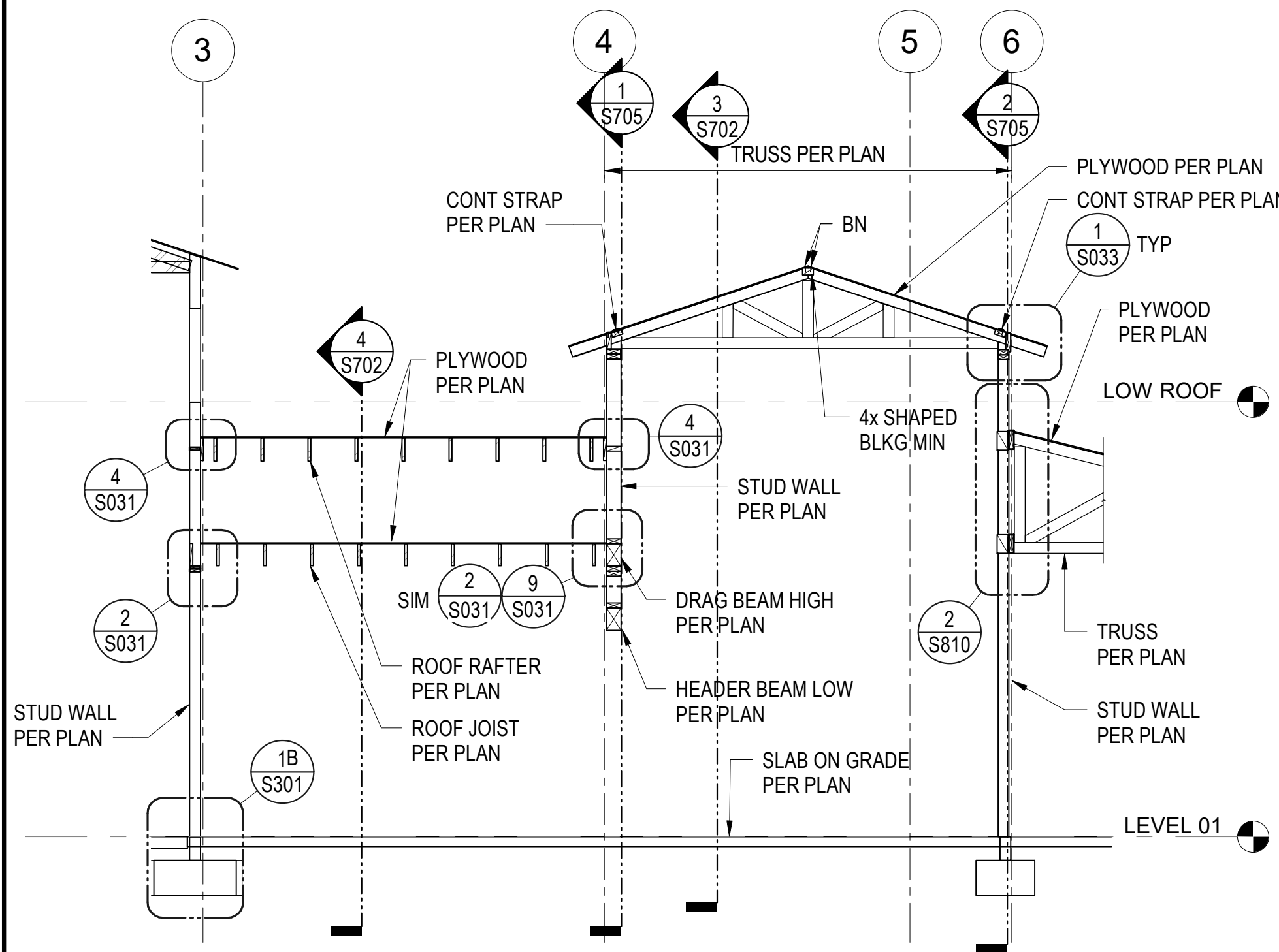
BUILDING SECTION 2
SCALE: 3/16" = 1'-0"

NOTE:
1. ALL TRUSSES SHALL HAVE A MINIMUM
2x6 TOP & BOTT CHORD, TYPICAL.
2. SEE NOTE #14 ON SHEET S035



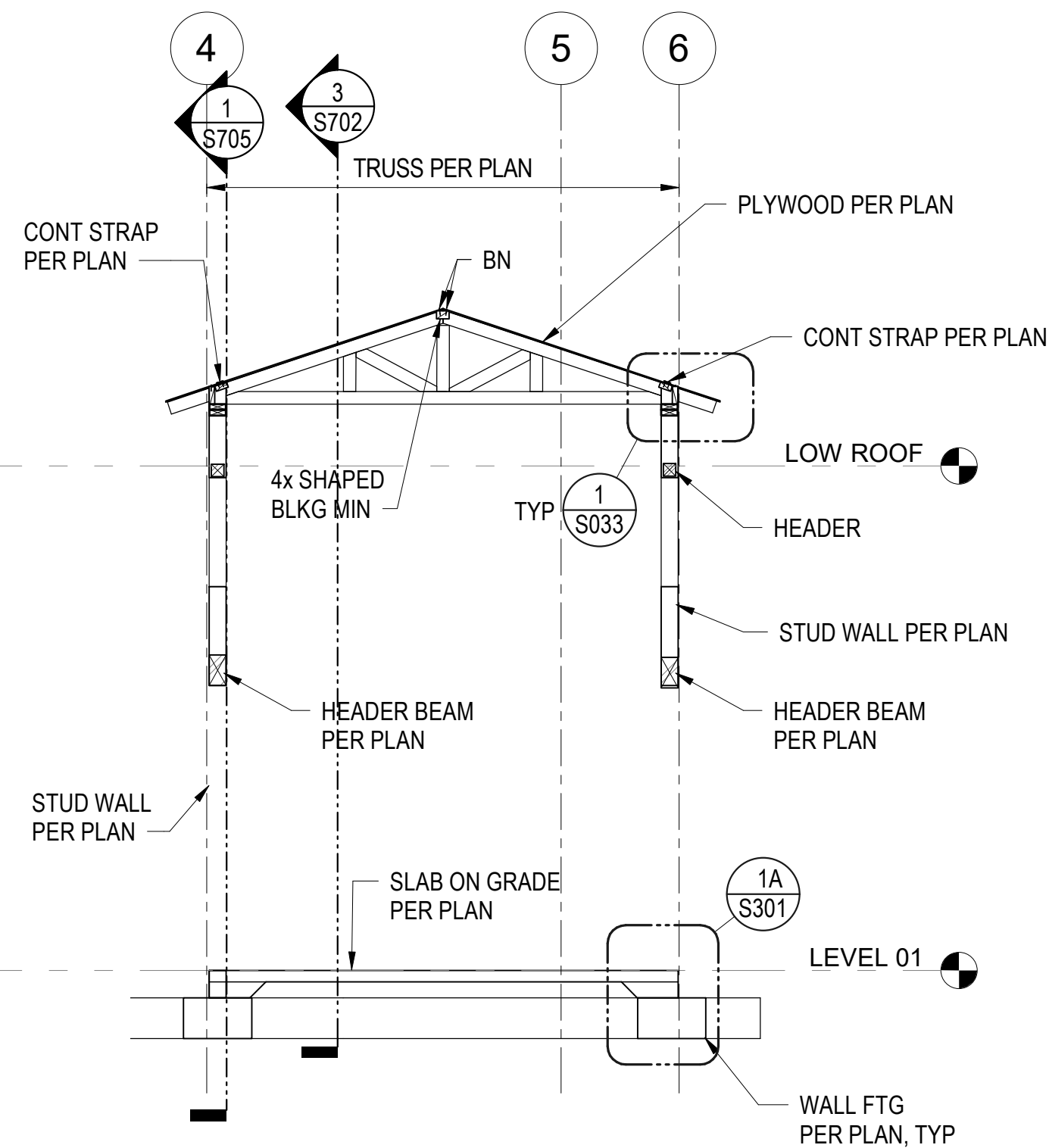
BUILDING SECTION 5
SCALE: 3/16" = 1'-0"

NOTE:
1. ALL TRUSSES SHALL HAVE A MINIMUM
2x6 TOP & BOTT CHORD, TYPICAL.
2. SEE NOTE #14 ON SHEET S035



BUILDING SECTION 3
SCALE: 3/16" = 1'-0"

NOTE:
1. ALL TRUSSES SHALL HAVE A MINIMUM
2x6 TOP & BOTT CHORD, TYPICAL.
2. SEE NOTE #14 ON SHEET S035



BUILDING SECTION 1
SCALE: 3/16" = 1'-0"

**WILLIAM LOYD JONES
ARCHITECT**

9415 culver boulevard
culver city, california
9 0 2 3 2

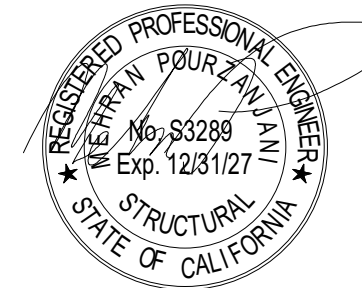
TEL 310 392 3995

SO
saiful-bouquet
structural engineers

725 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
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BUILDING SECTIONS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

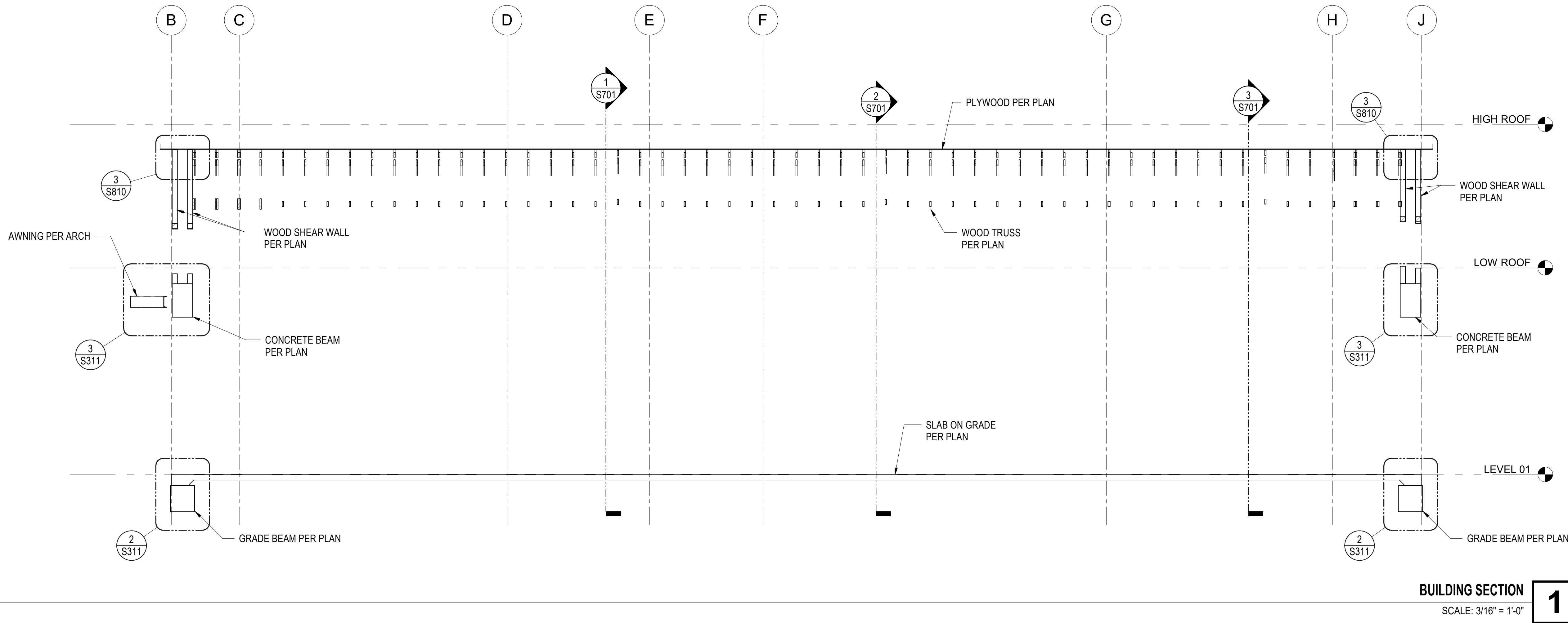


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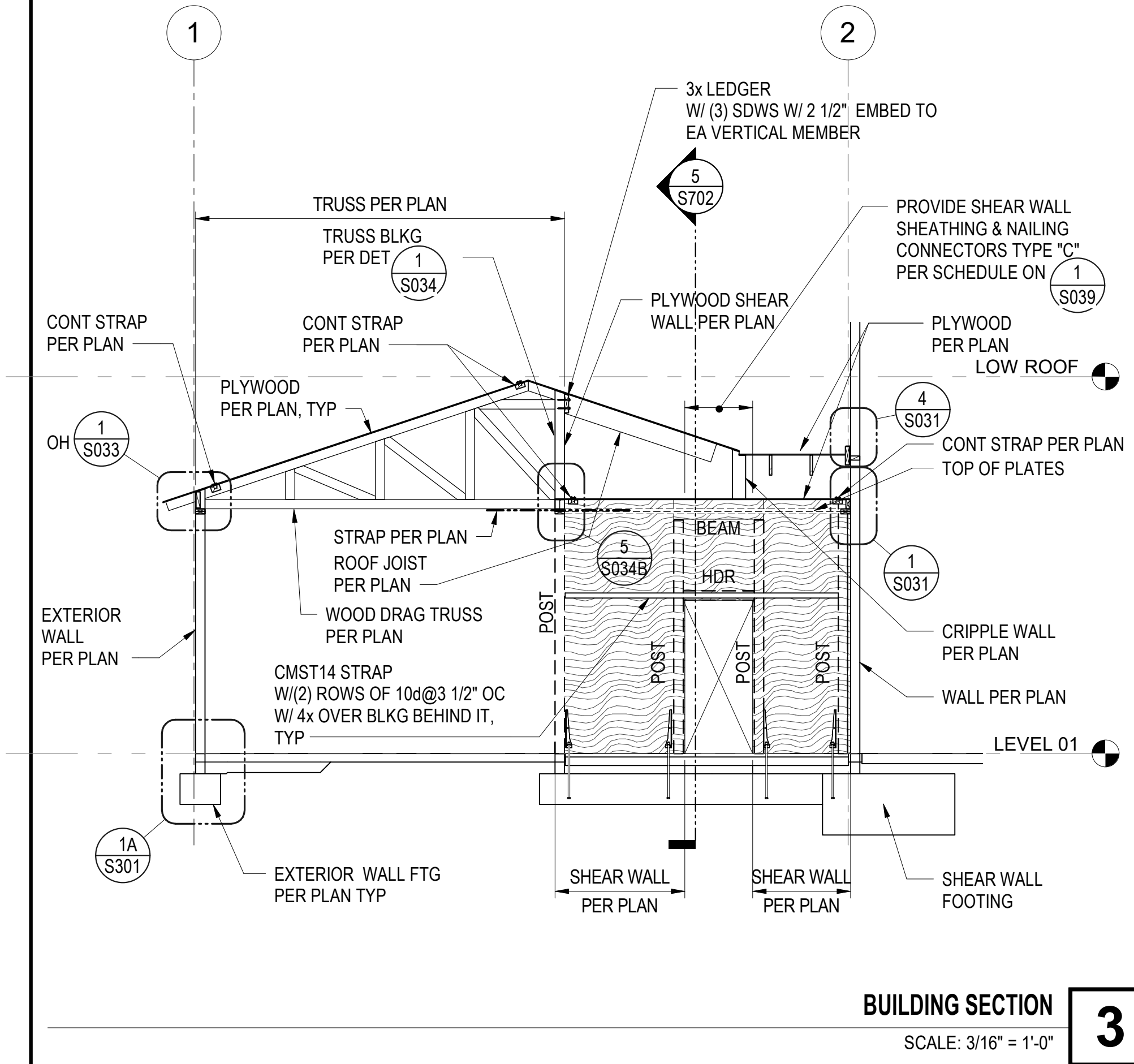
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Scale _____ **AS NOTED**
Job. No. _____ **Project Number** _____

S703

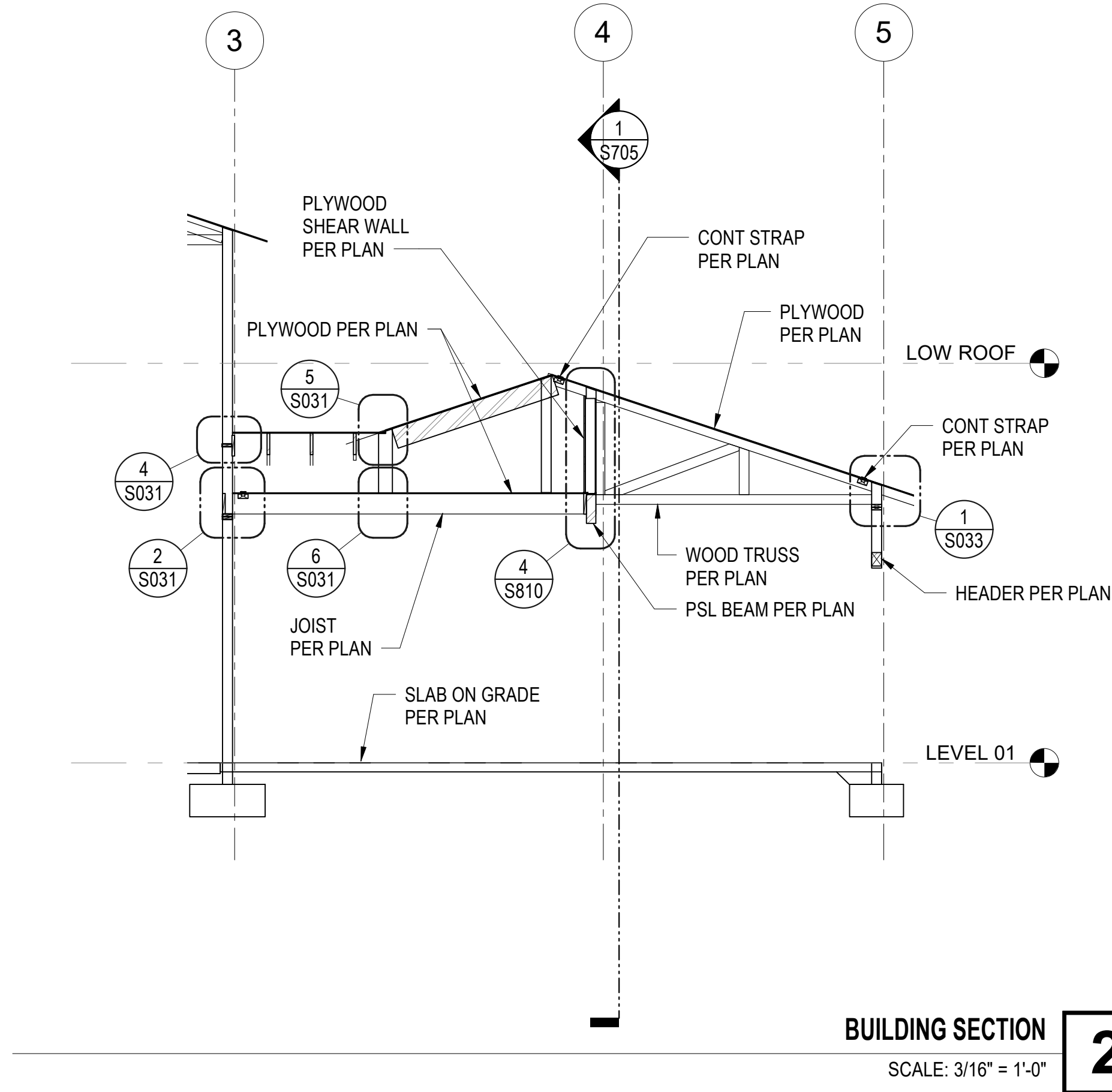
ADDENDUM 5



BUILDING SECTION 1
SCALE: 3/16" = 1'-0"



BUILDING SECTION 3
SCALE: 3/16" = 1'-0"



BUILDING SECTION 2
SCALE: 3/16" = 1'-0"

**WILLIAM LOYD JONES
ARCHITECT**

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

SO
saiful-bouquet
structural engineers

725 S. Figueroa St.,
3rd floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
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BUILDING SECTIONS

FIRE STATION 46

MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

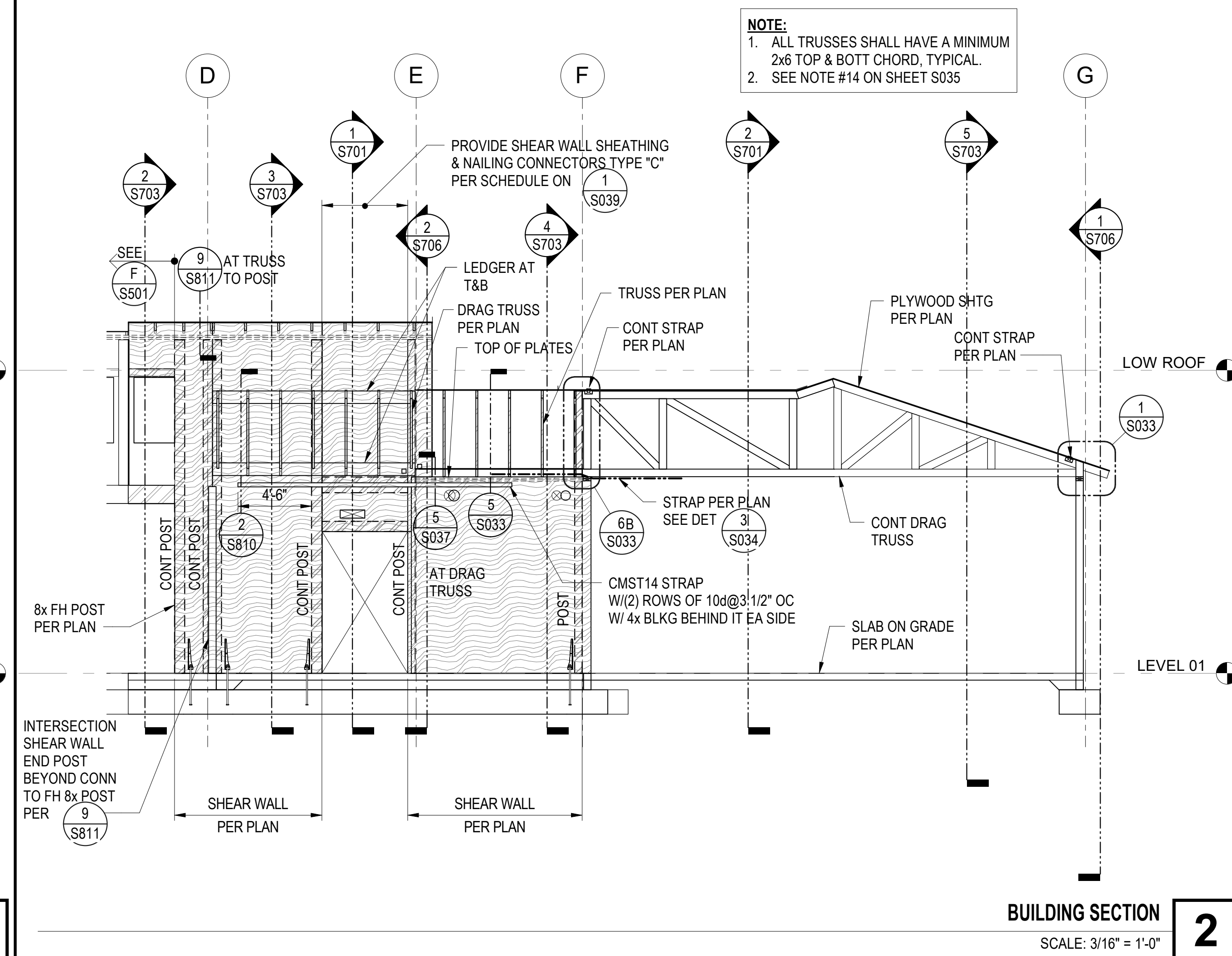
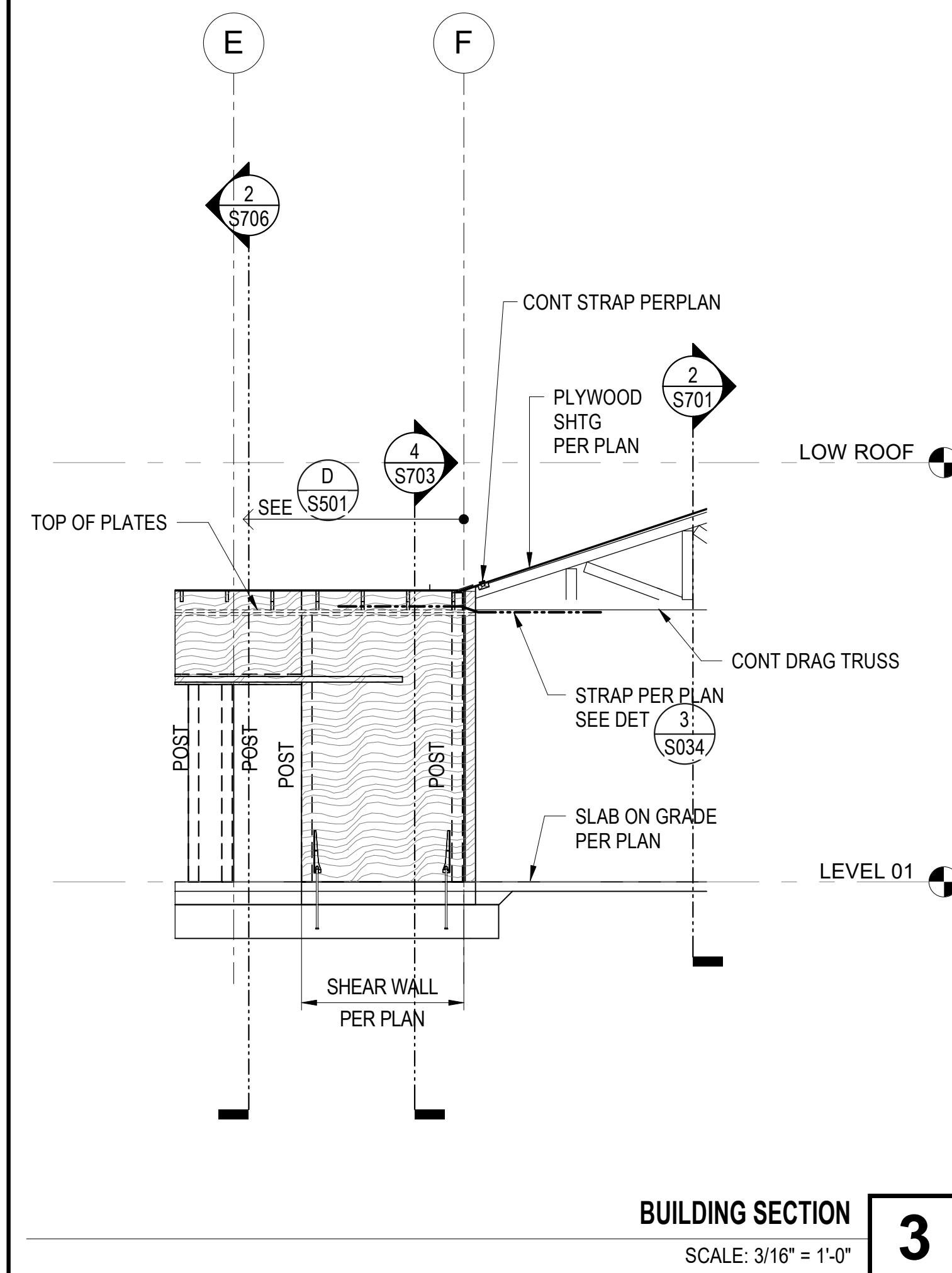
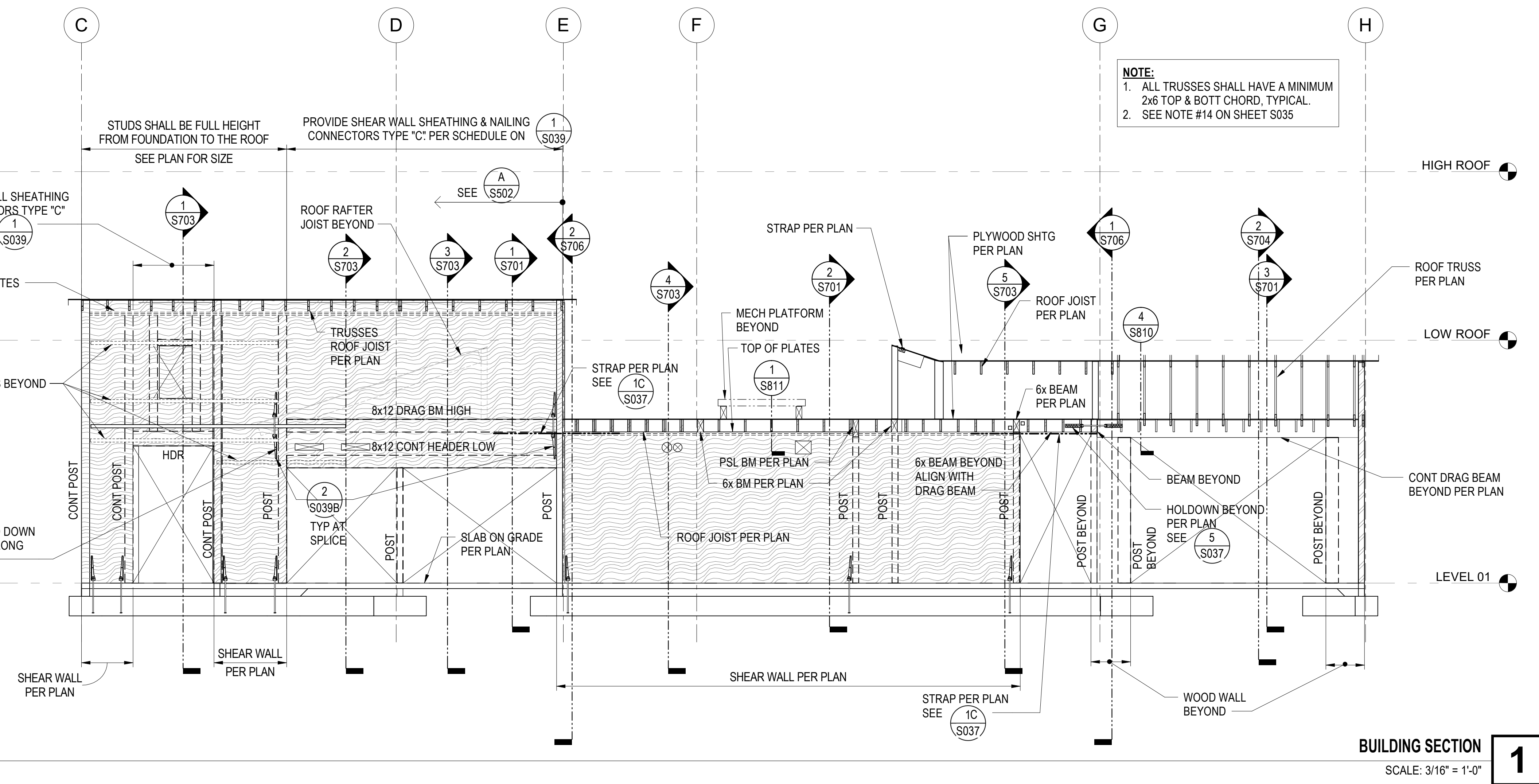


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PROJECT NUMBER: 25534

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APPENDIX 5



WILLIAM LOYD JONES
ARCHITECT



Saiful-bouquet
structural engineers

725 S. Figueroa St.,
37th Floor
Los Angeles, CA 90017
www.safu-bouquet.com
(213) 315-2277
Project #25534

BUILDING SECTIONS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

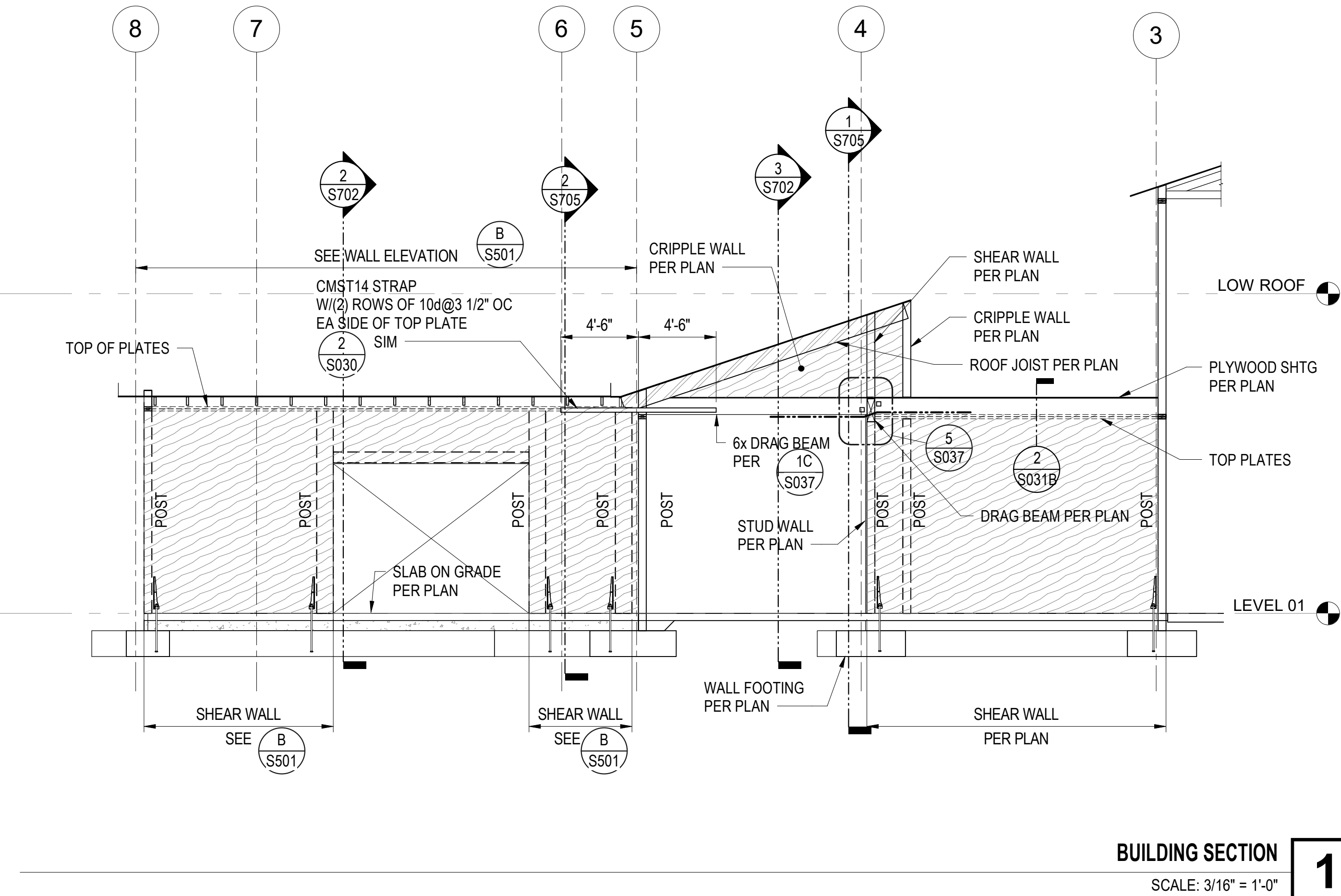
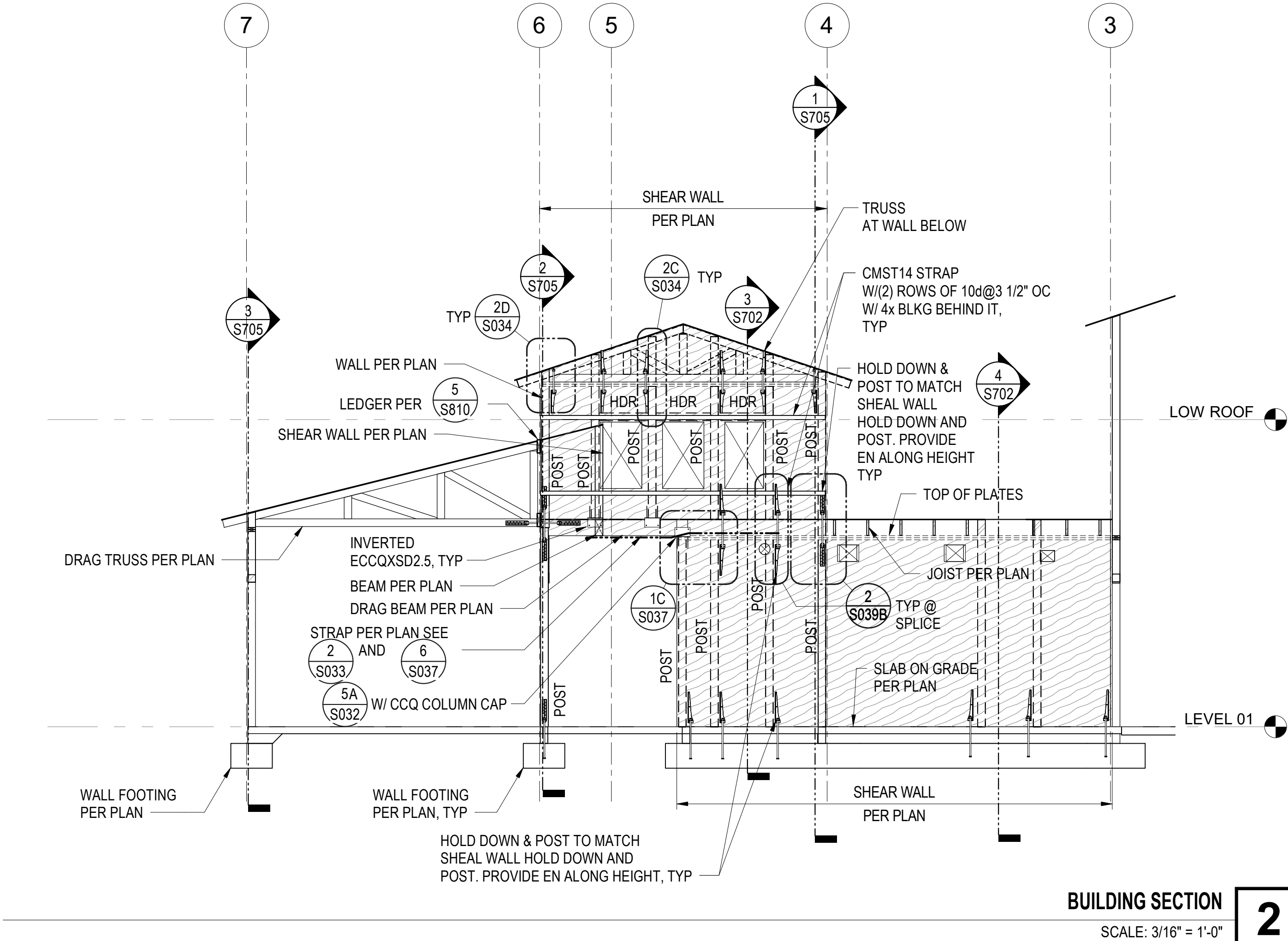


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Job. No.	Project Number

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WORK.



WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

so
saiful-bouquet
structural engineers

725 S. Figueroa St.,
37th Floor
Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
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BUILDING SECTIONS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA



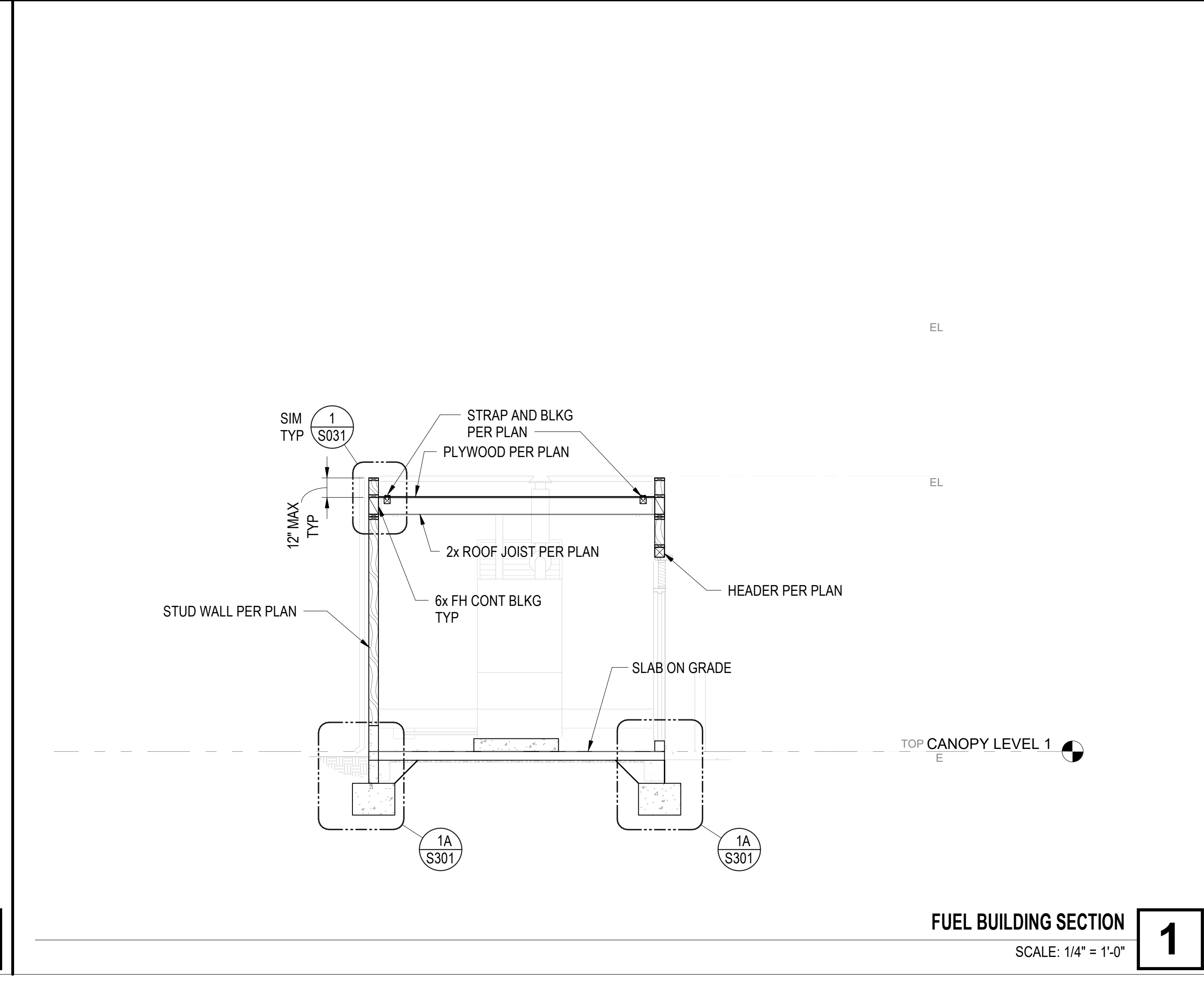
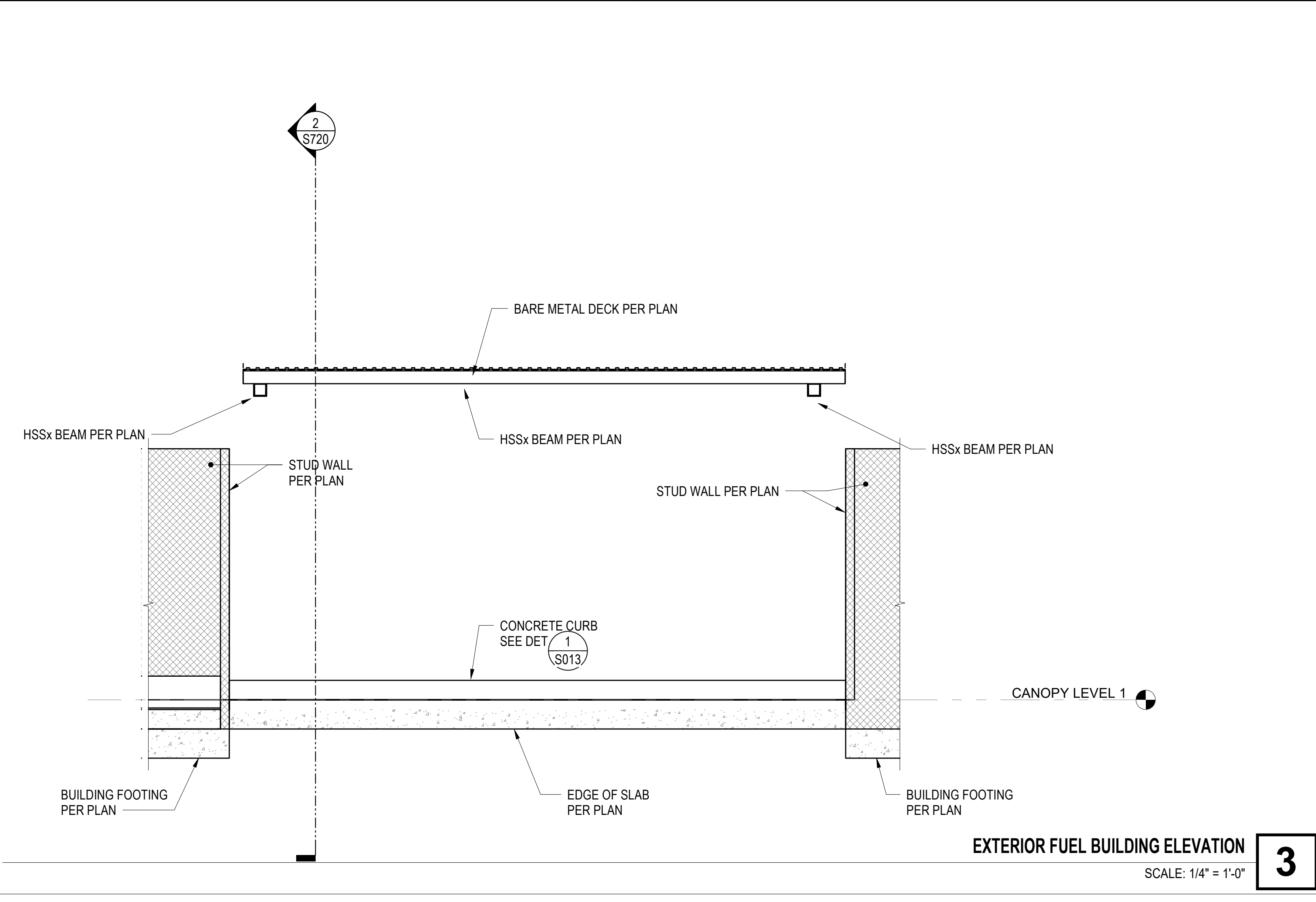
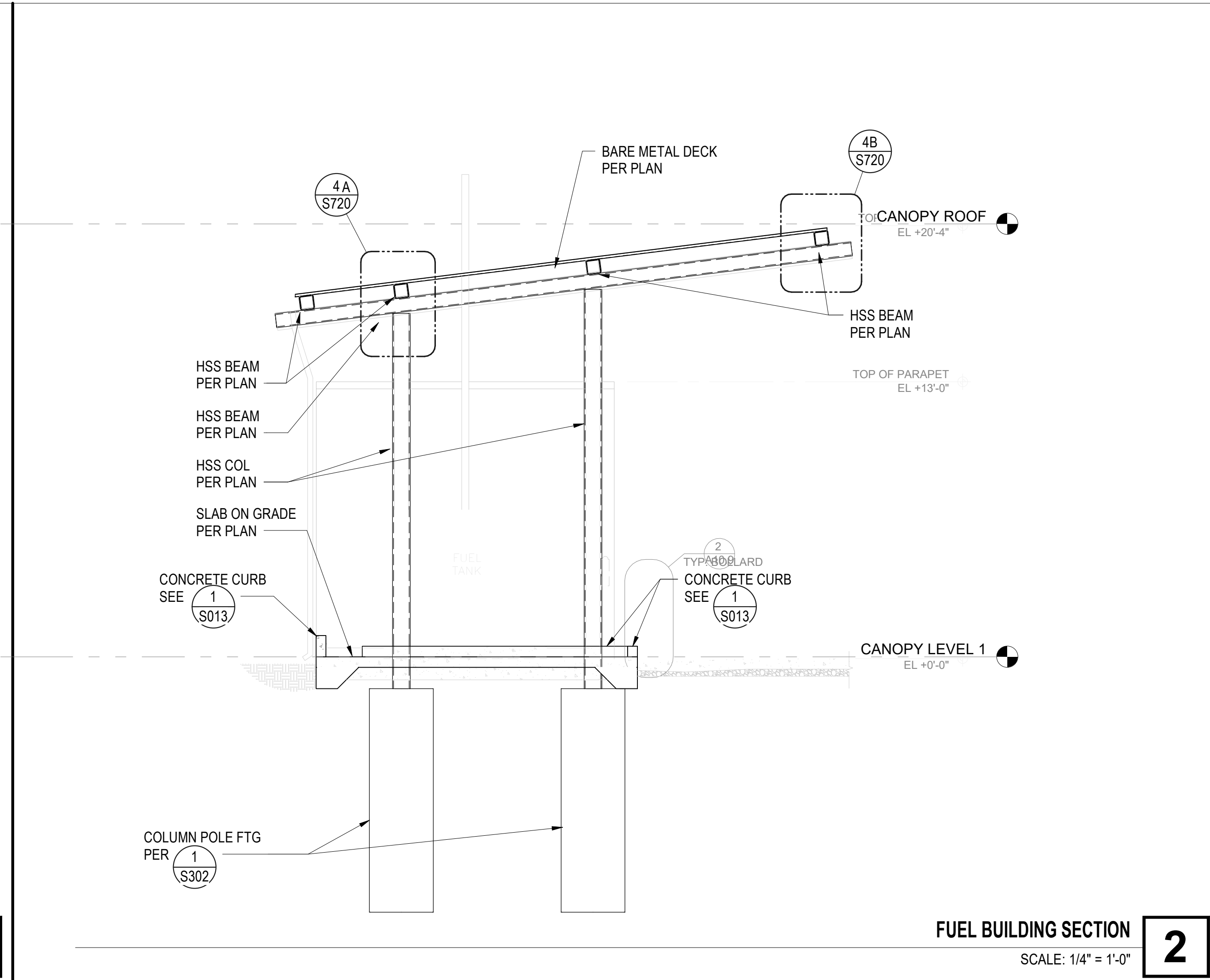
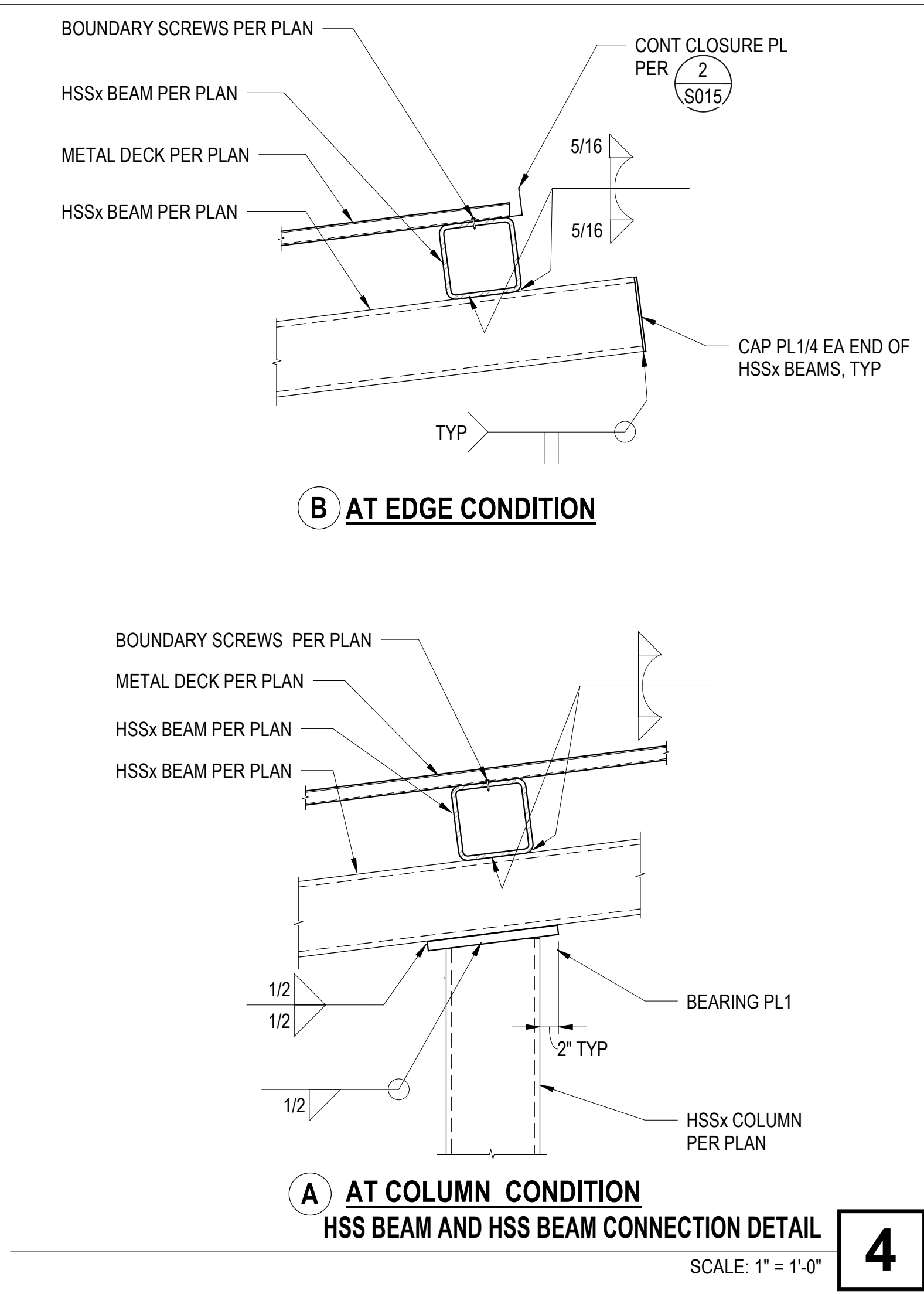
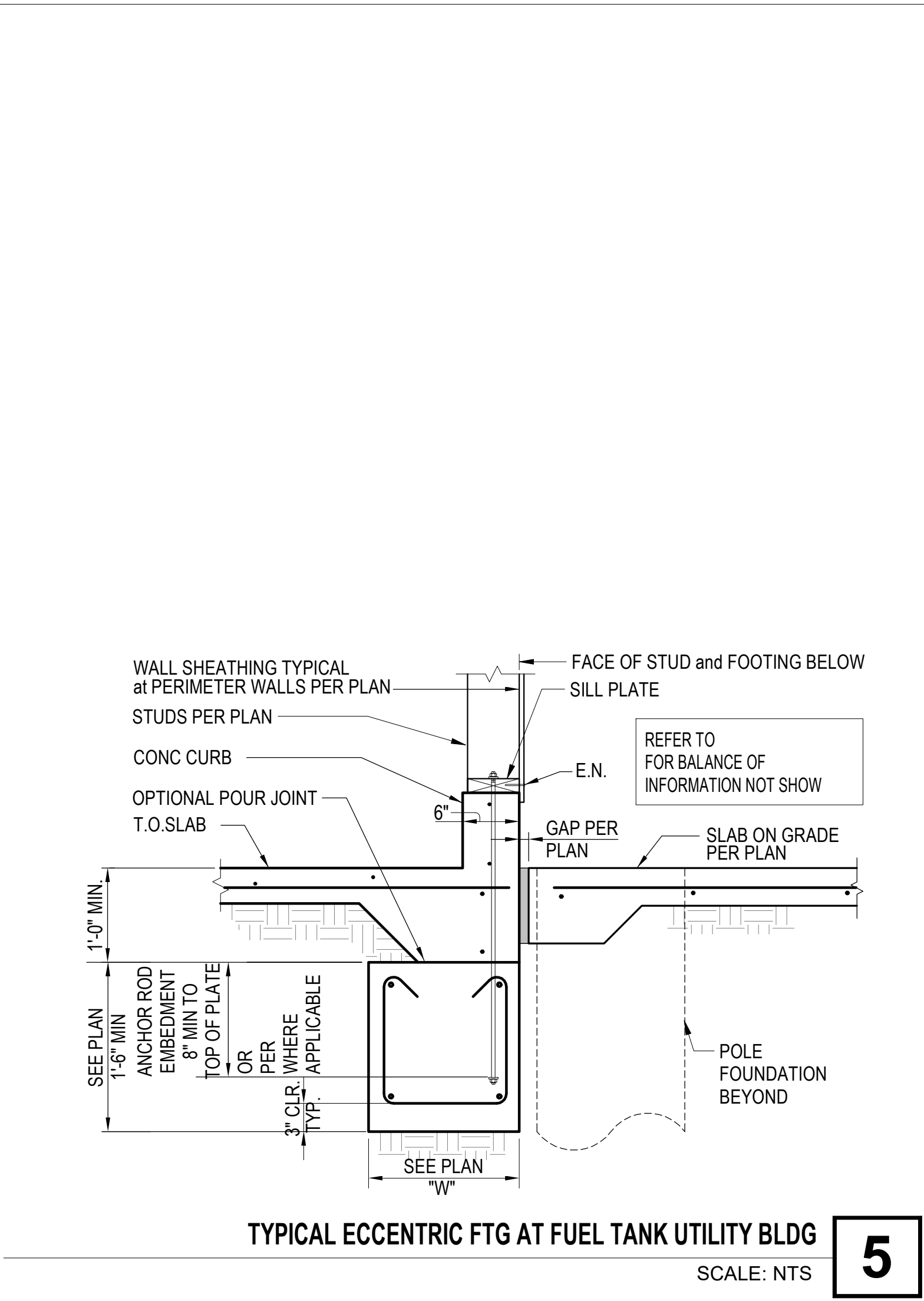
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APPENDIX 5

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WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
9 0 2 3 2

TEL 310 392 3995

so
saiful-bouquet
structural engineers

726 S. Figueroa St.,
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Los Angeles, CA 90017
www.saifulbouquet.com
(213) 315-2277
Project #25534

FUEL DISPENSING
SECTIONS

FIRE STATION 46
MISSION VILLAGE
COUNTY OF LOS ANGELES FIRE DEPARTMENT
VALENCIA, CALIFORNIA

REGISTERED PROFESSIONAL ENGINEER
No. S33288
Exp. 12/31/27
STATE OF CALIFORNIA
STRUCTURAL

5

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Scale
Job. No.

Issue Date
AS NOTED
Project Number

S720

APPENDIX

PLAN CHECK SUBMITTAL - October 31 2025



S801



B BETWEEN TRUSSES CONDITION



SCALE: 1" = 1'-0"



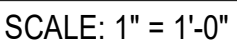
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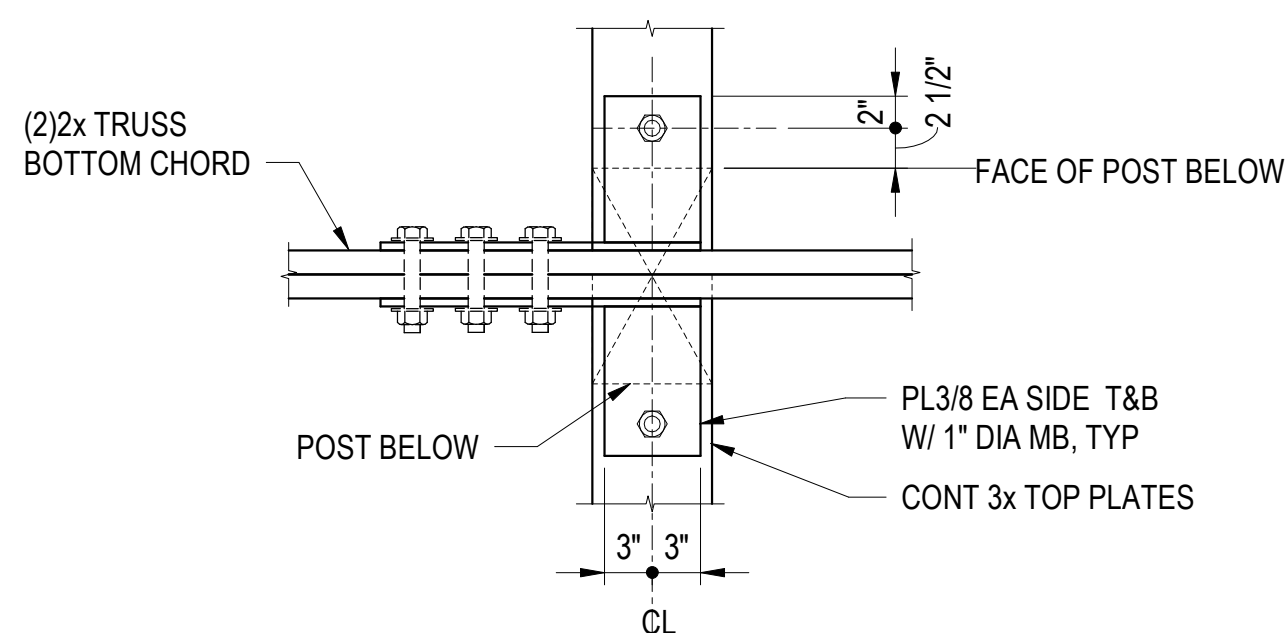
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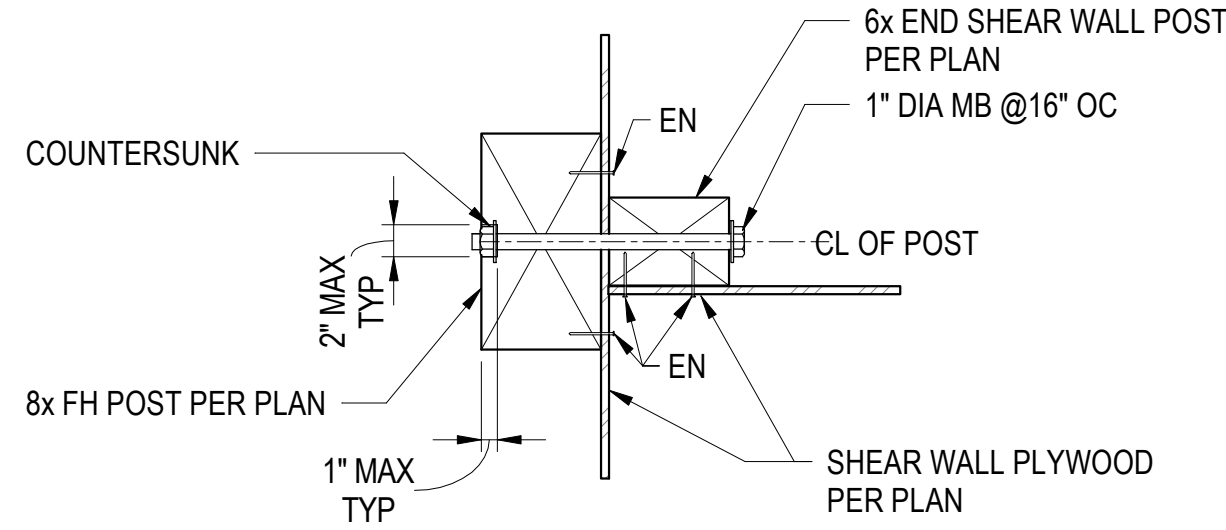
ELEVATION C-C



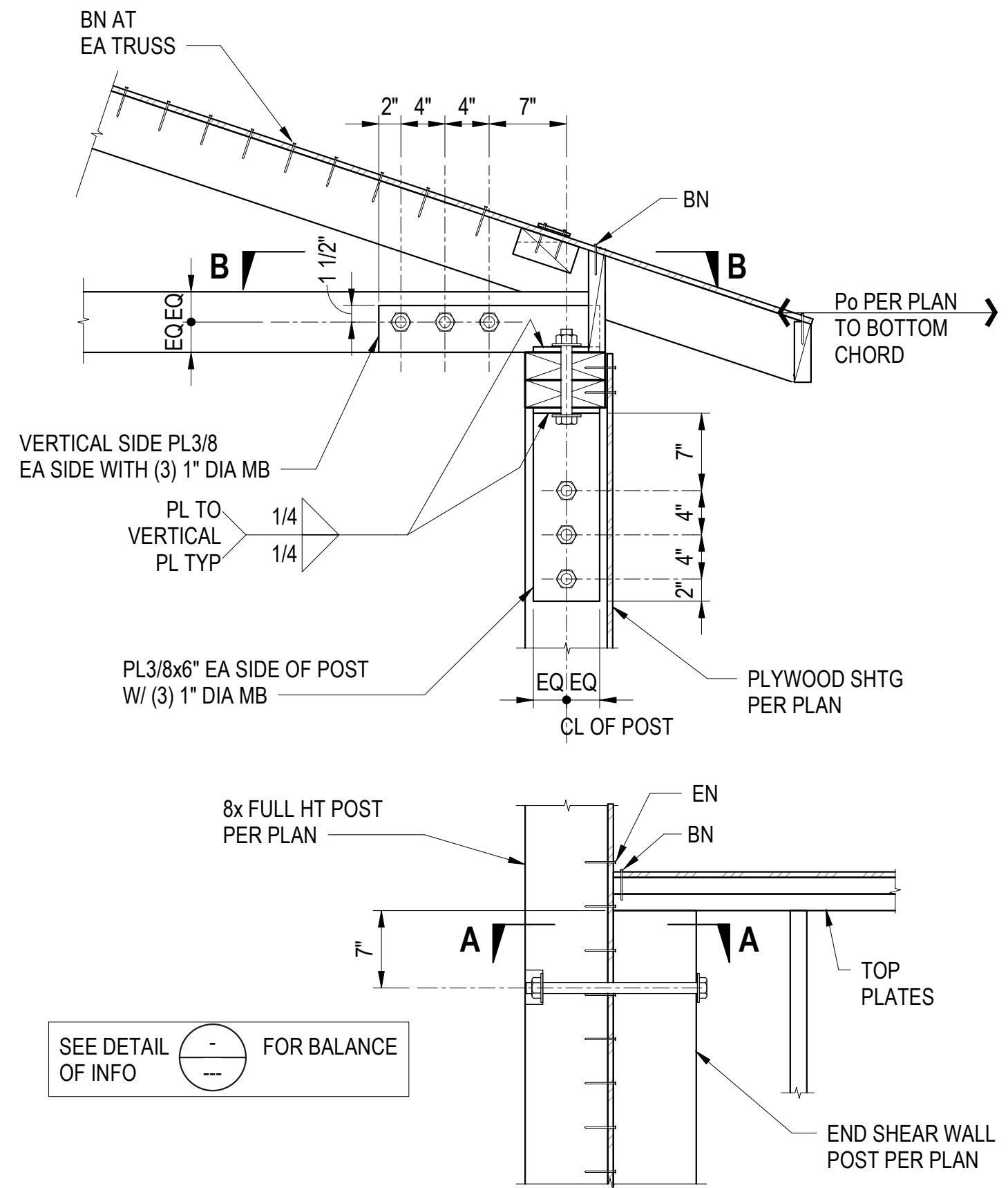
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PLAN VIEW B-B



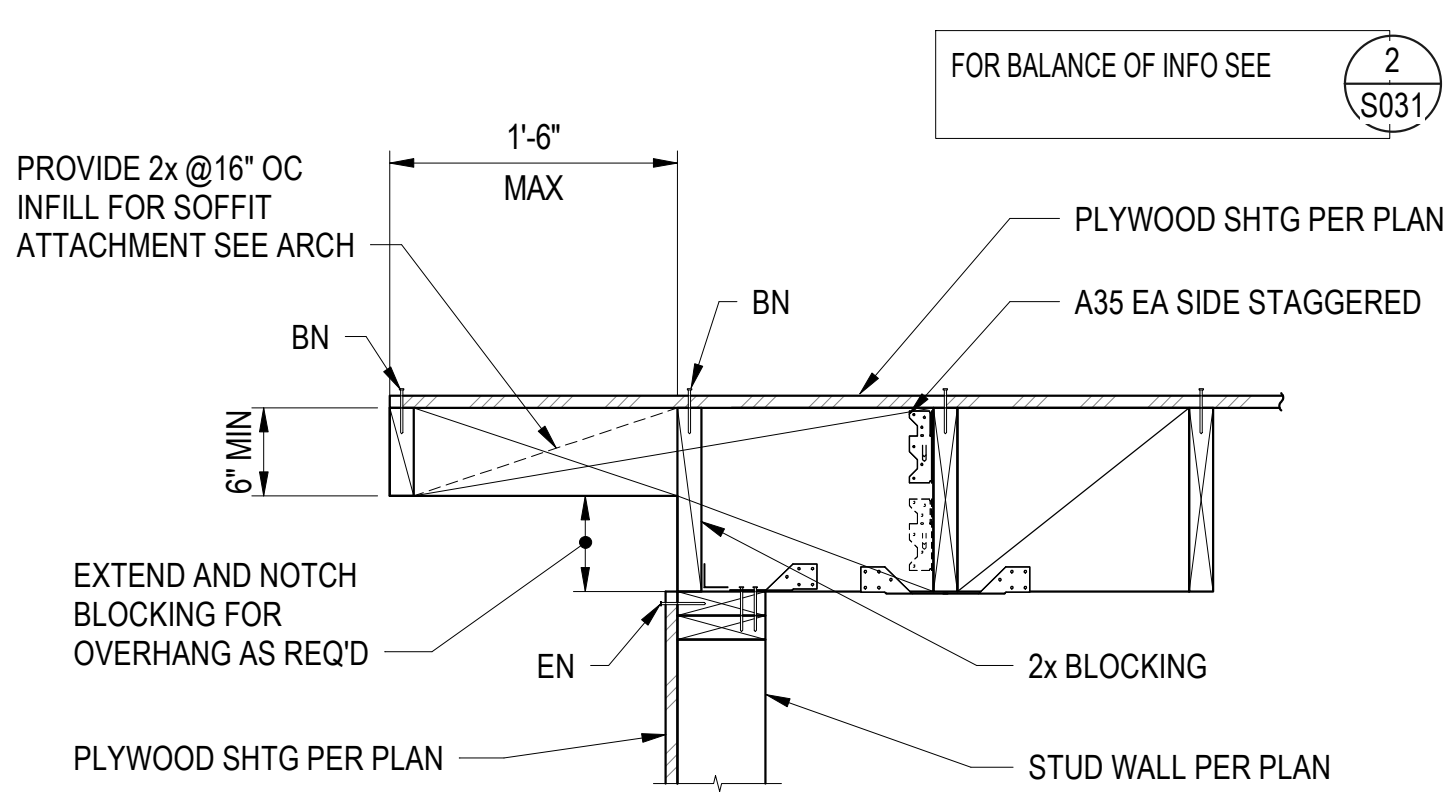
PLAN VIEW A-A



TRUSS TO POST CONNECTION DETAIL

SCALE: 1" = 1'-0"

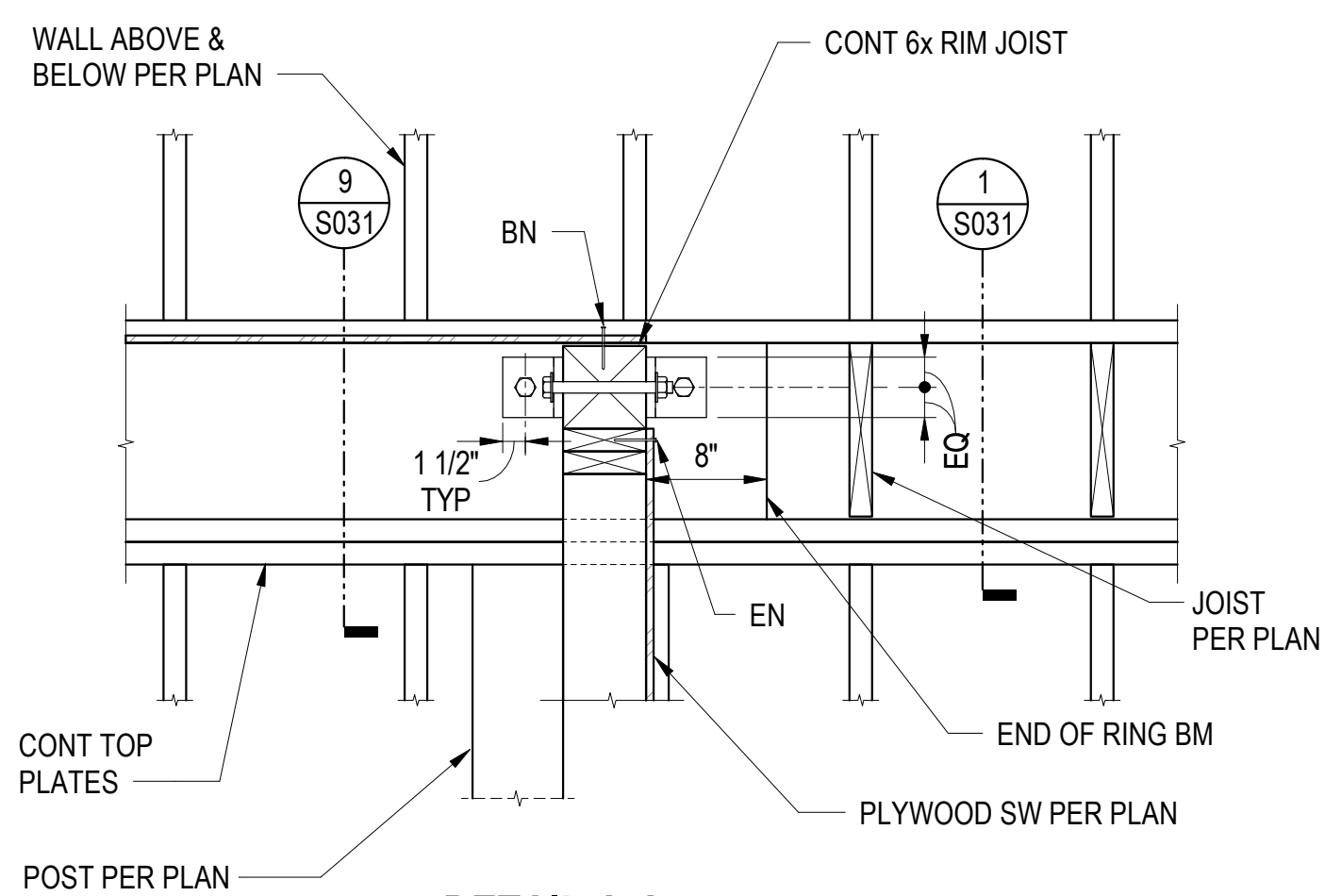
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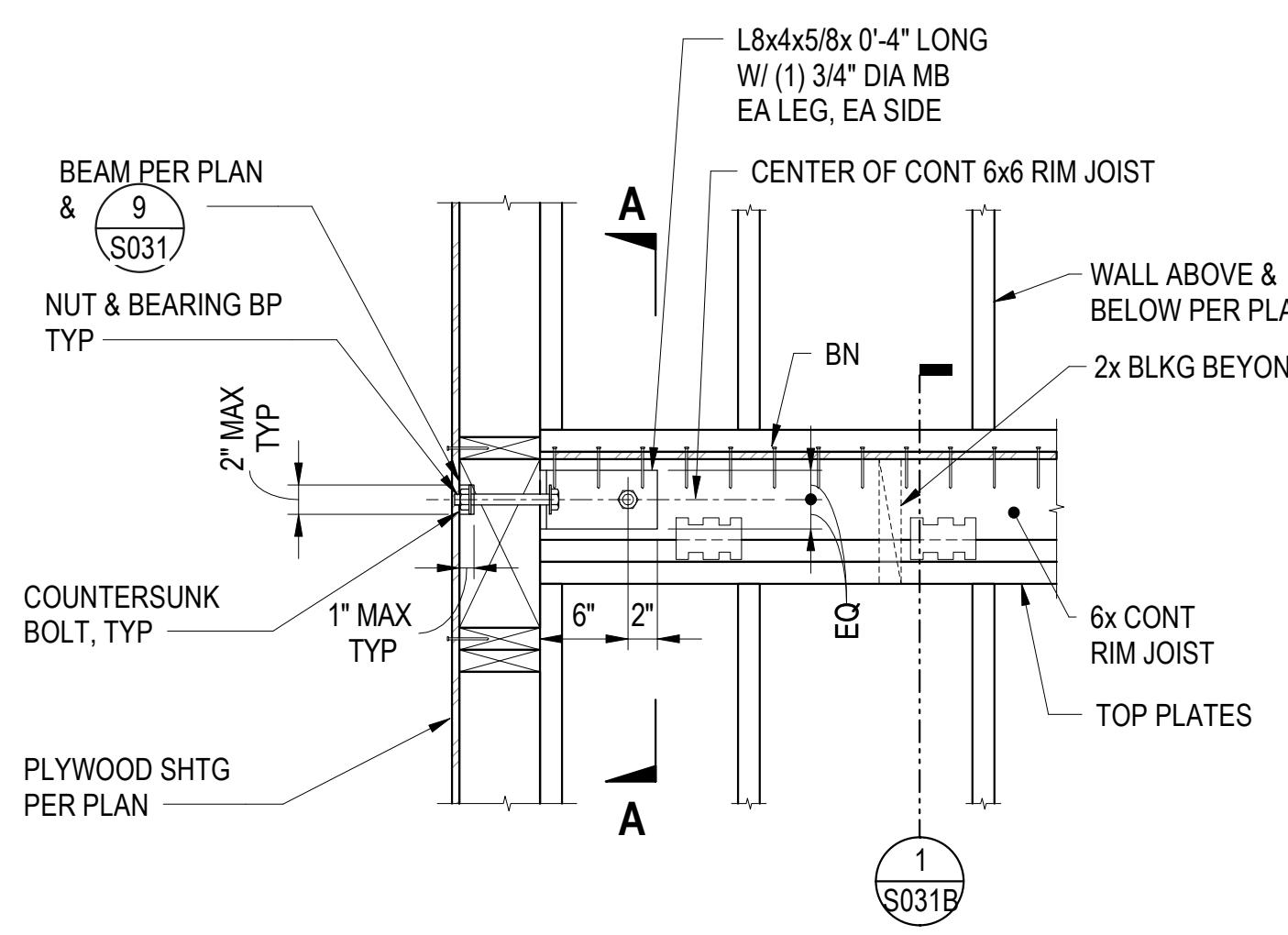
ROOF JOIST PARALLEL TO WALL
AT EAVE CONNECTION DETAIL

SCALE: 1" = 1'-0"

7



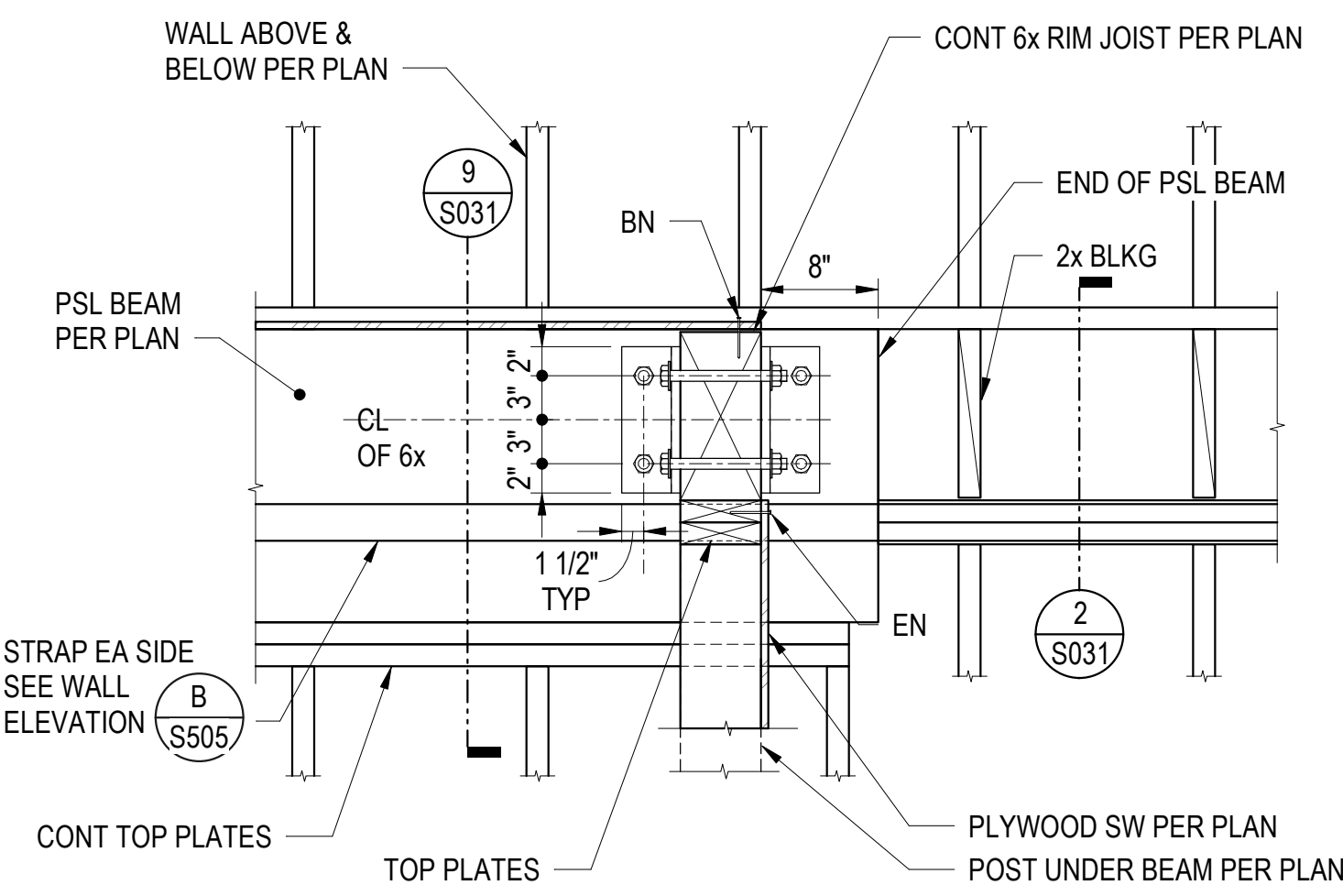
DETAIL A-A



WALL TO BEAM CONNECTION DETAIL

SCALE: 1" = 1'-0"

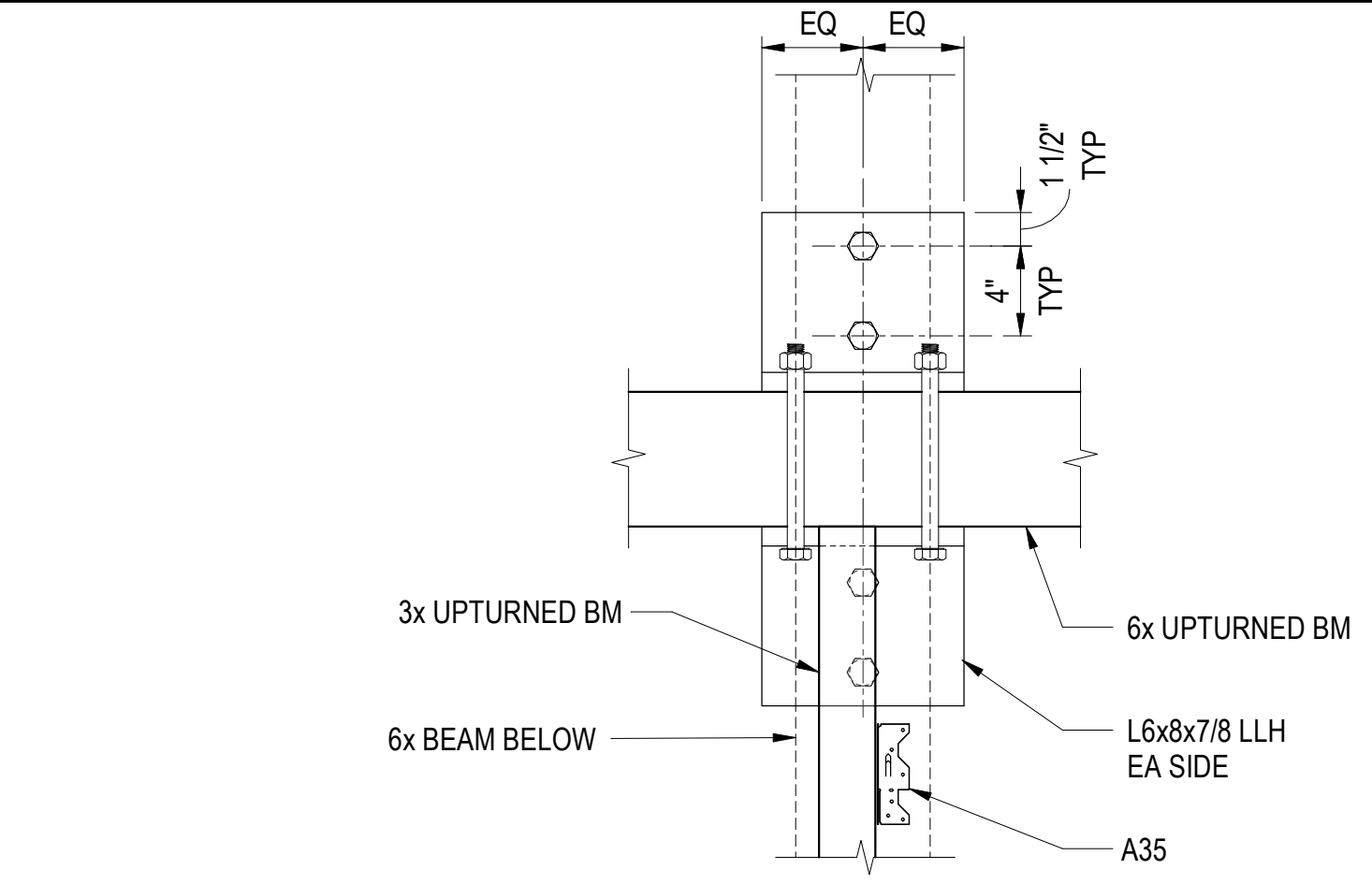
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WALL TO BEAM CONNECTION DETAIL

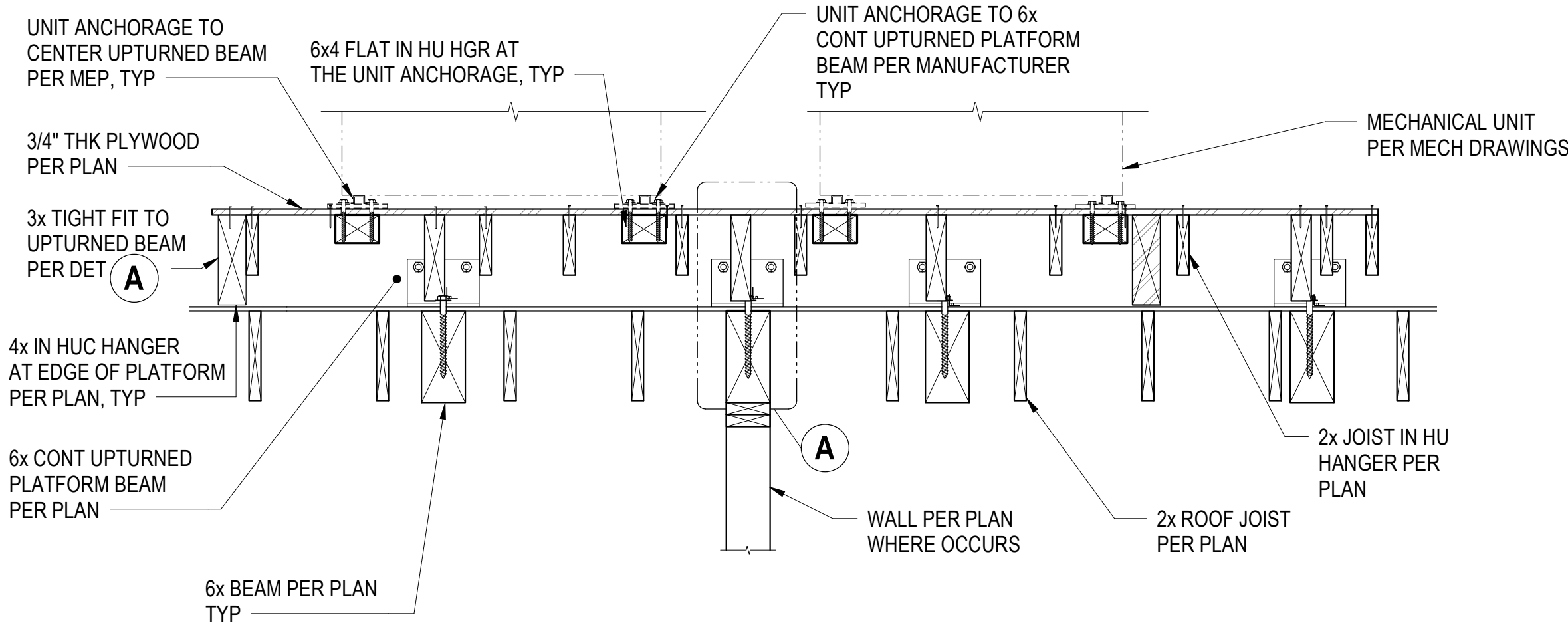
SCALE: 1" = 1'-0"

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DETAIL B-B

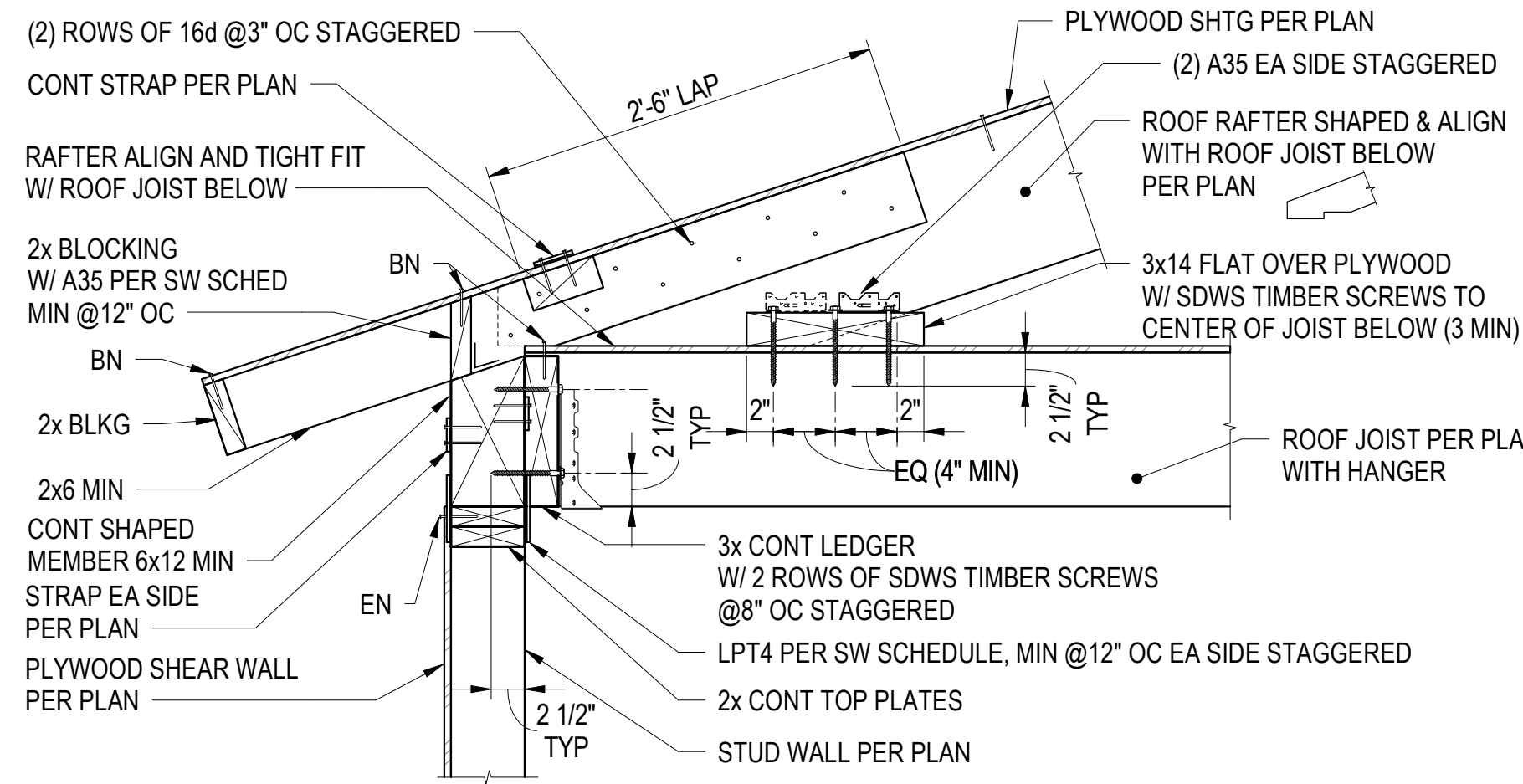
SCALE: 1 1/2" = 1'-0"



SECTION AT MECHANICAL UNIT

SCALE: 3/4" = 1'-0"

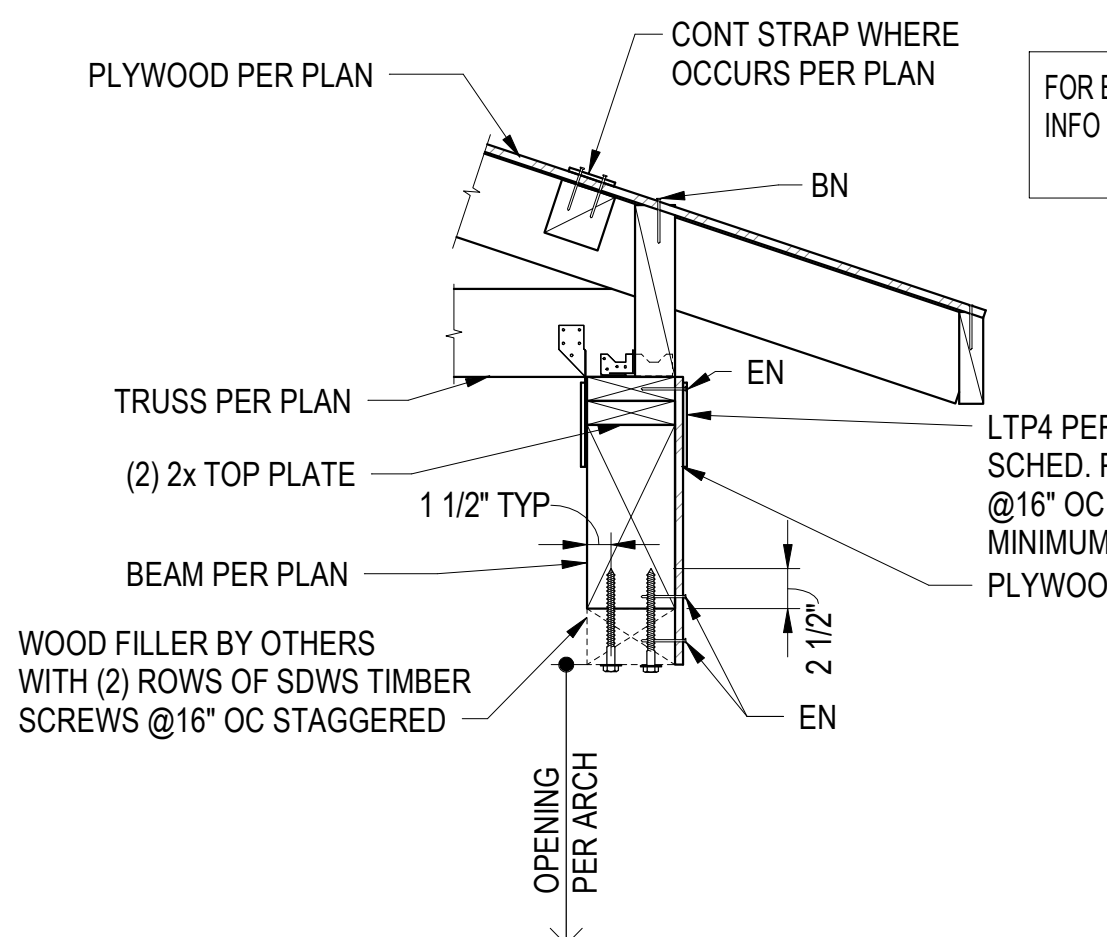
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ROOF RAFTER AT EAVE CONNECTION DETAIL

SCALE: 1" = 1'-0"

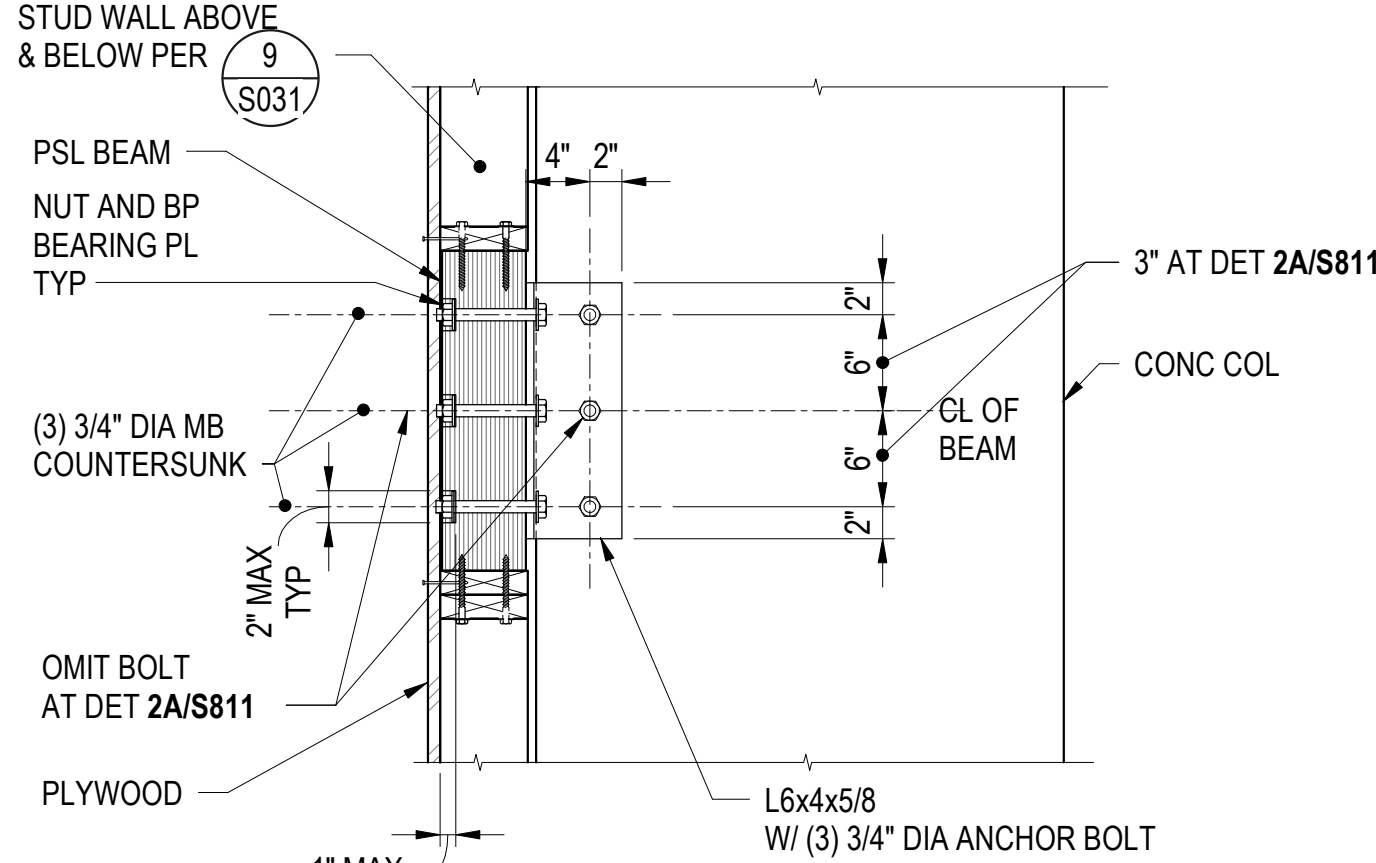
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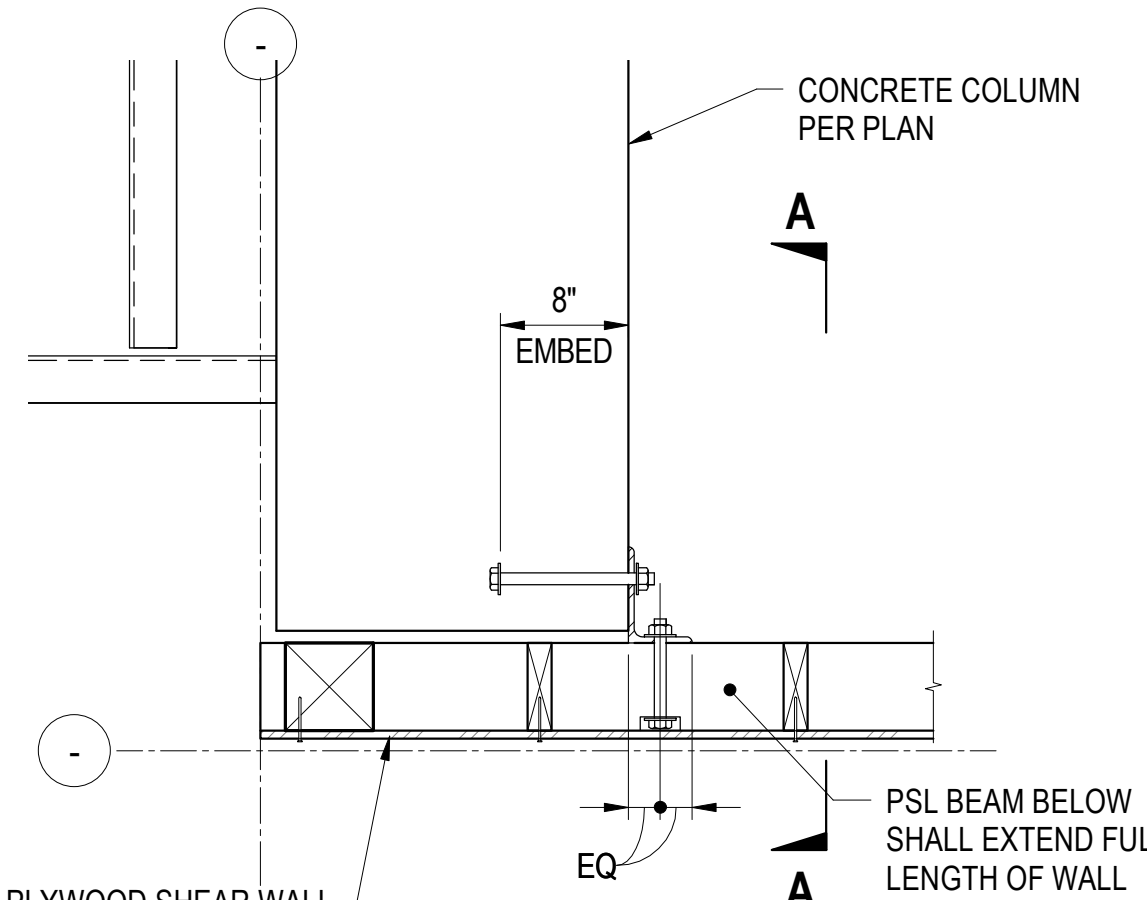
TRUSS TO BEAM CONNECTION DETAIL

SCALE: 1" = 1'-0"

5



DETAIL A-A



PLAN VIEW

WALL BEAM TO CONCRETE COLUMN CONNECTION

SCALE: 1" = 1'-0"

2A

2

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

SO
saiful-boutique
structural engineers

726 S. Figueroa St.,
37th floor
Los Angeles, CA 90017
www.saifulboutique.com
(213) 315-2277
Project #25534

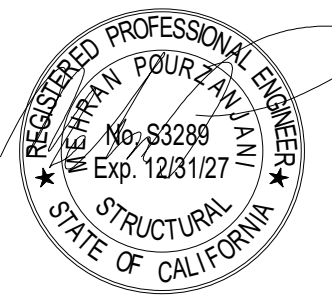
SECTIONS AND DETAILS

FIRE STATION 46

MISSION VILLAGE

COUNTY OF LOS ANGELES FIRE DEPARTMENT

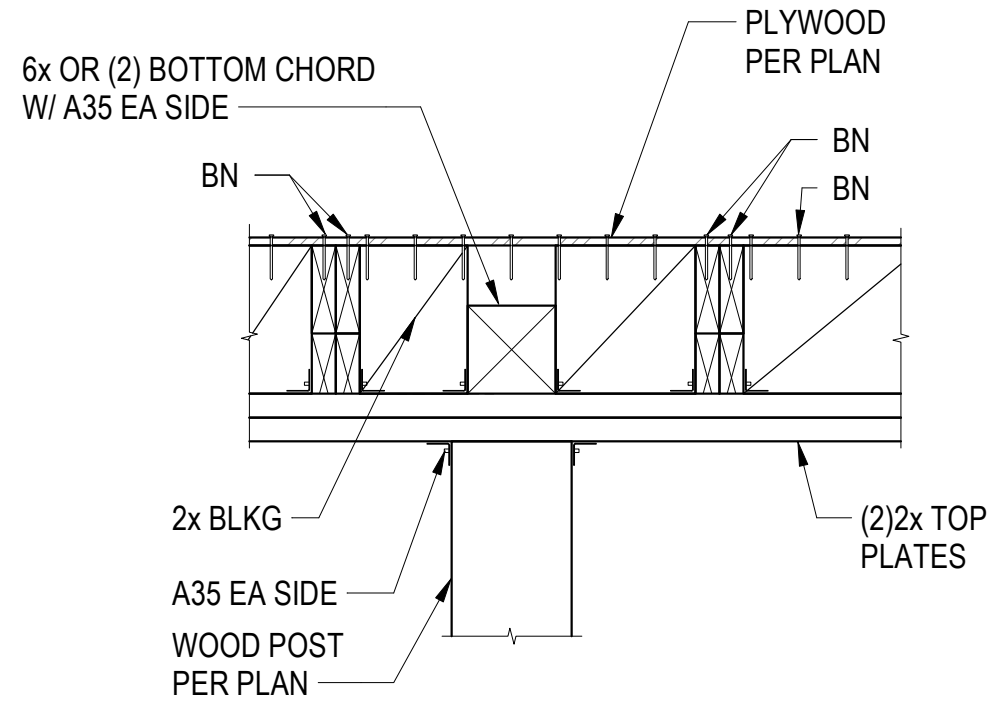
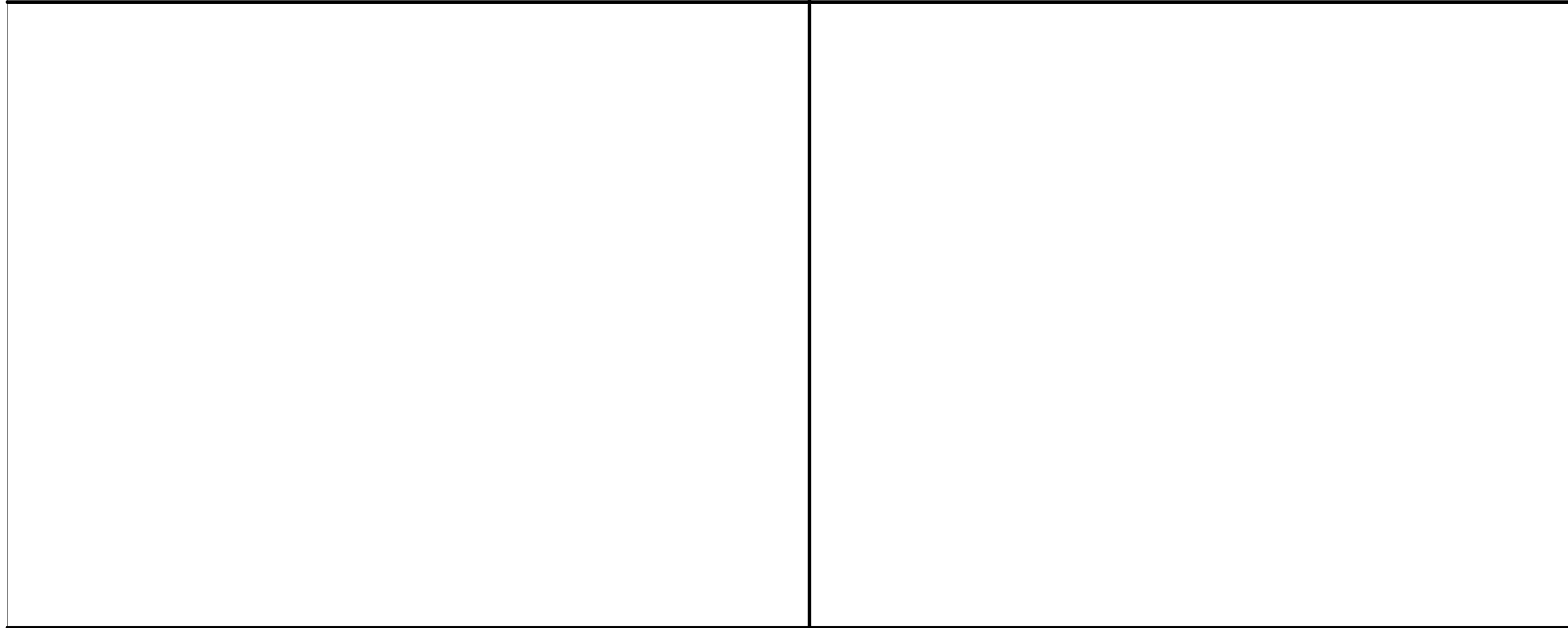
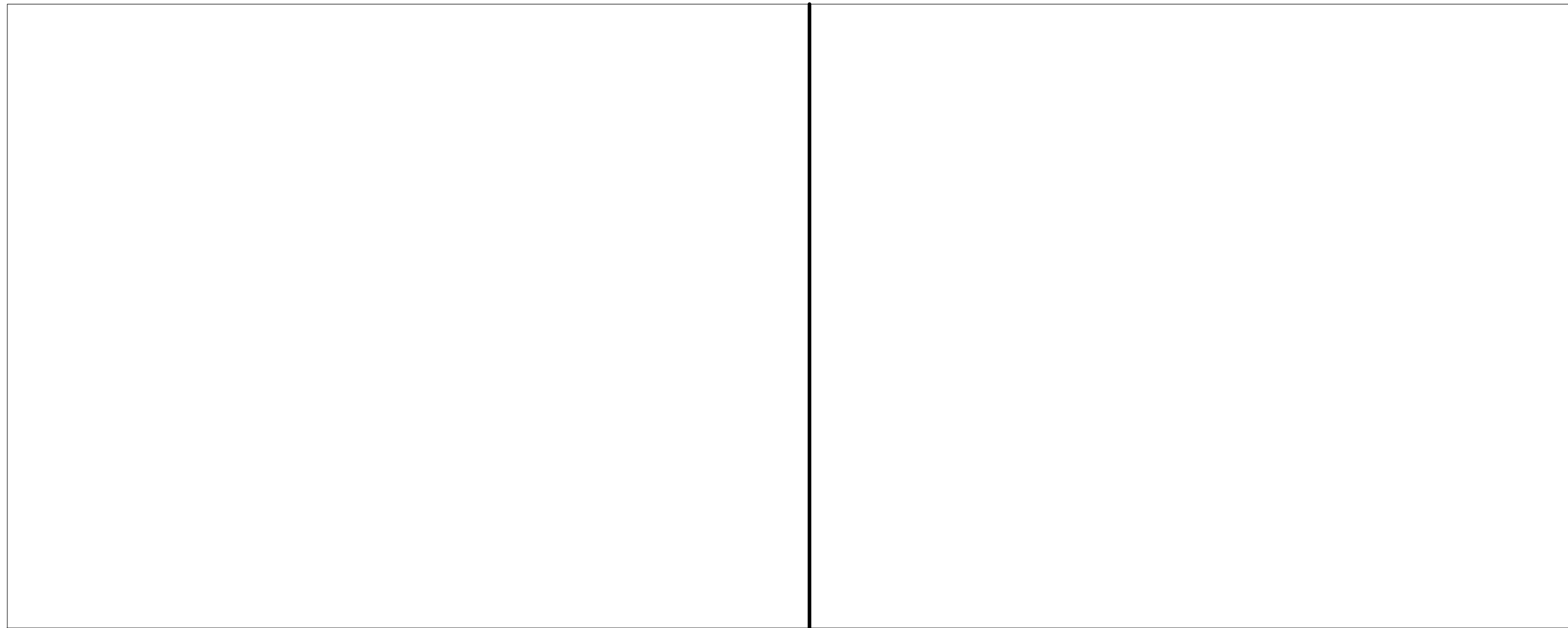
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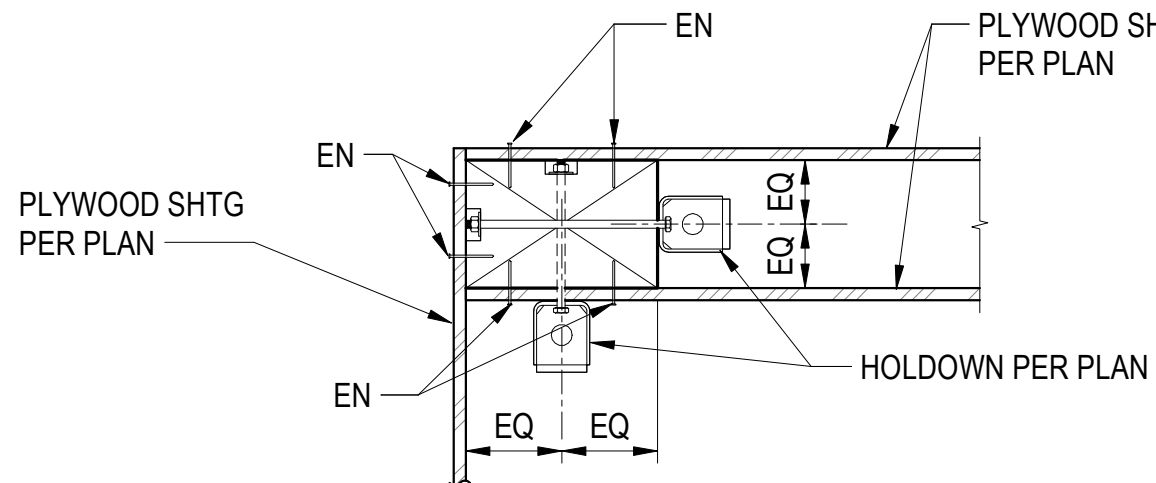
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APPENDIX M 5

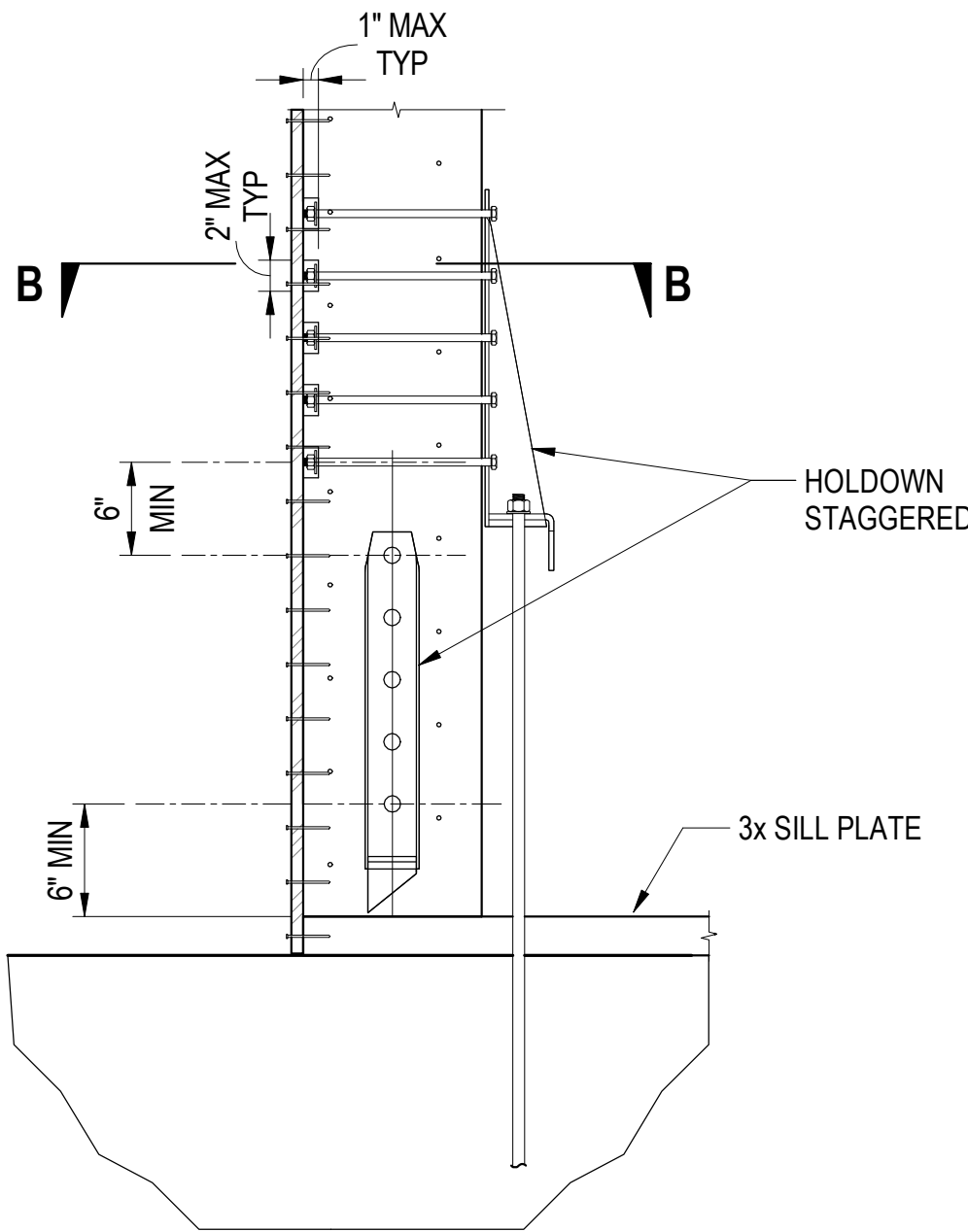


ELEVATION C-C

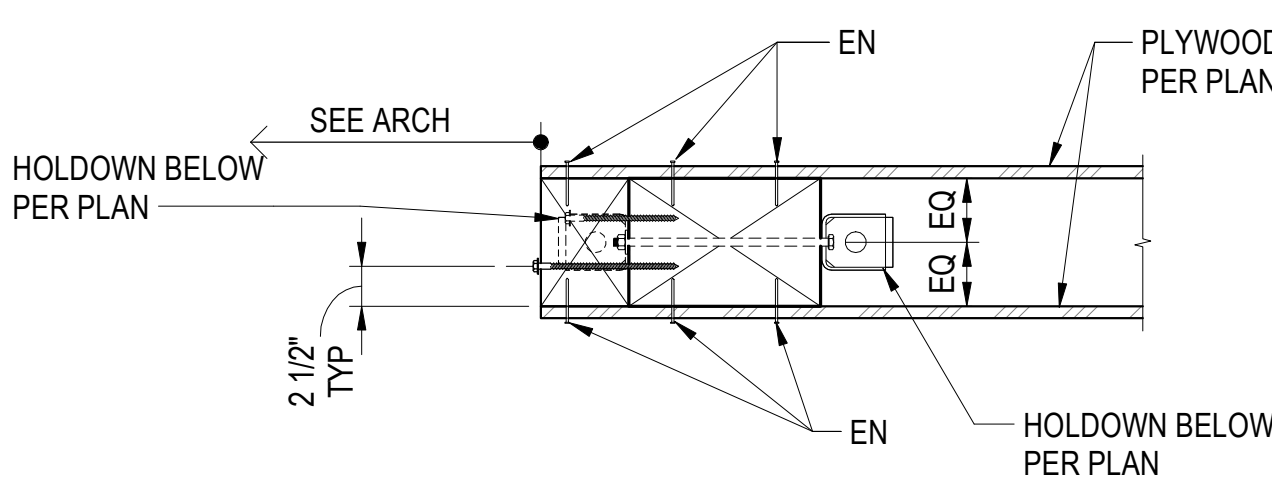
NOTE: SEE DETAIL 1 S033 FOR BALANCE OF INFO



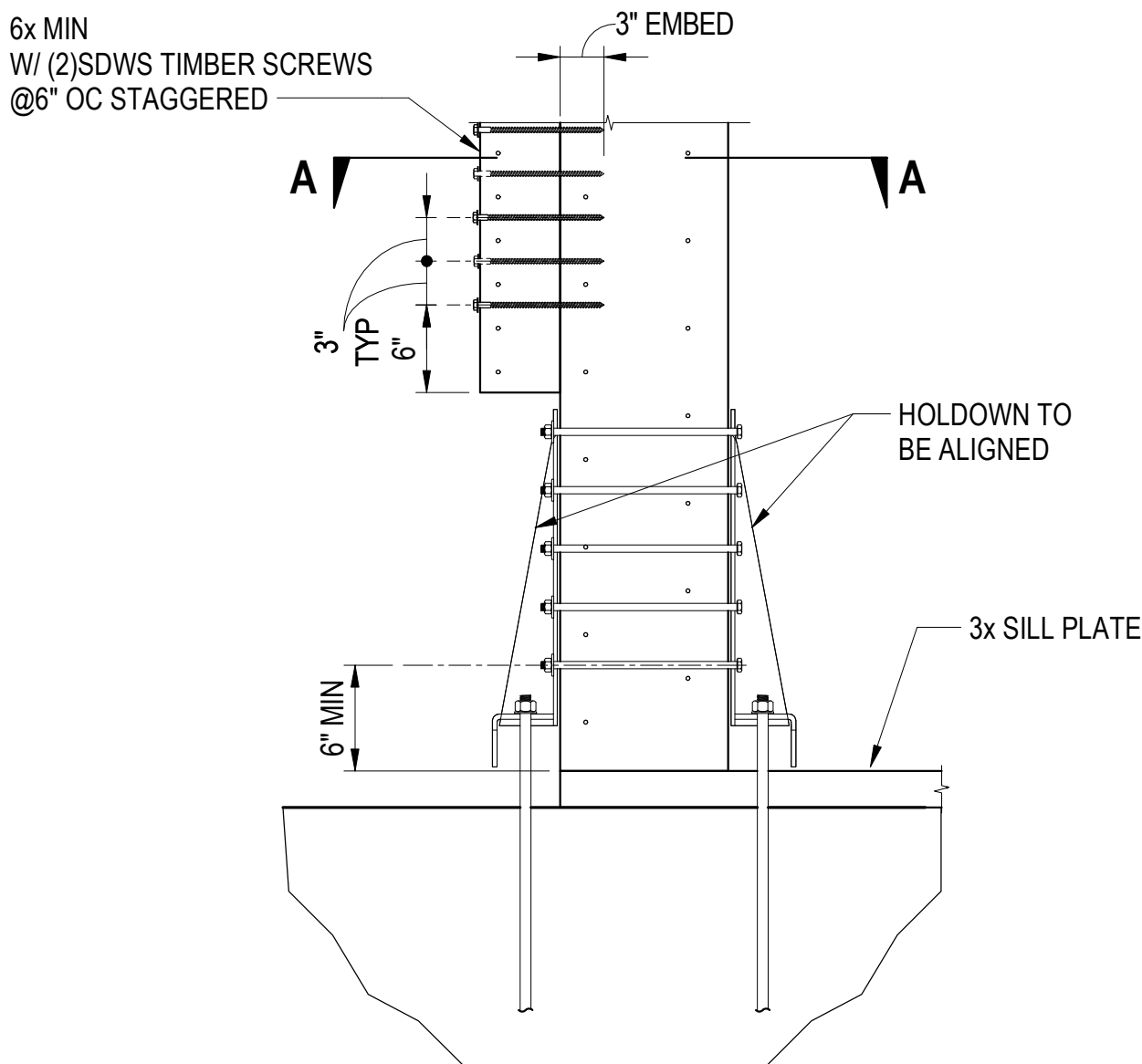
PLAN VIEW B-B



B DOUBLE HOLDOWN AT CORNER WALL



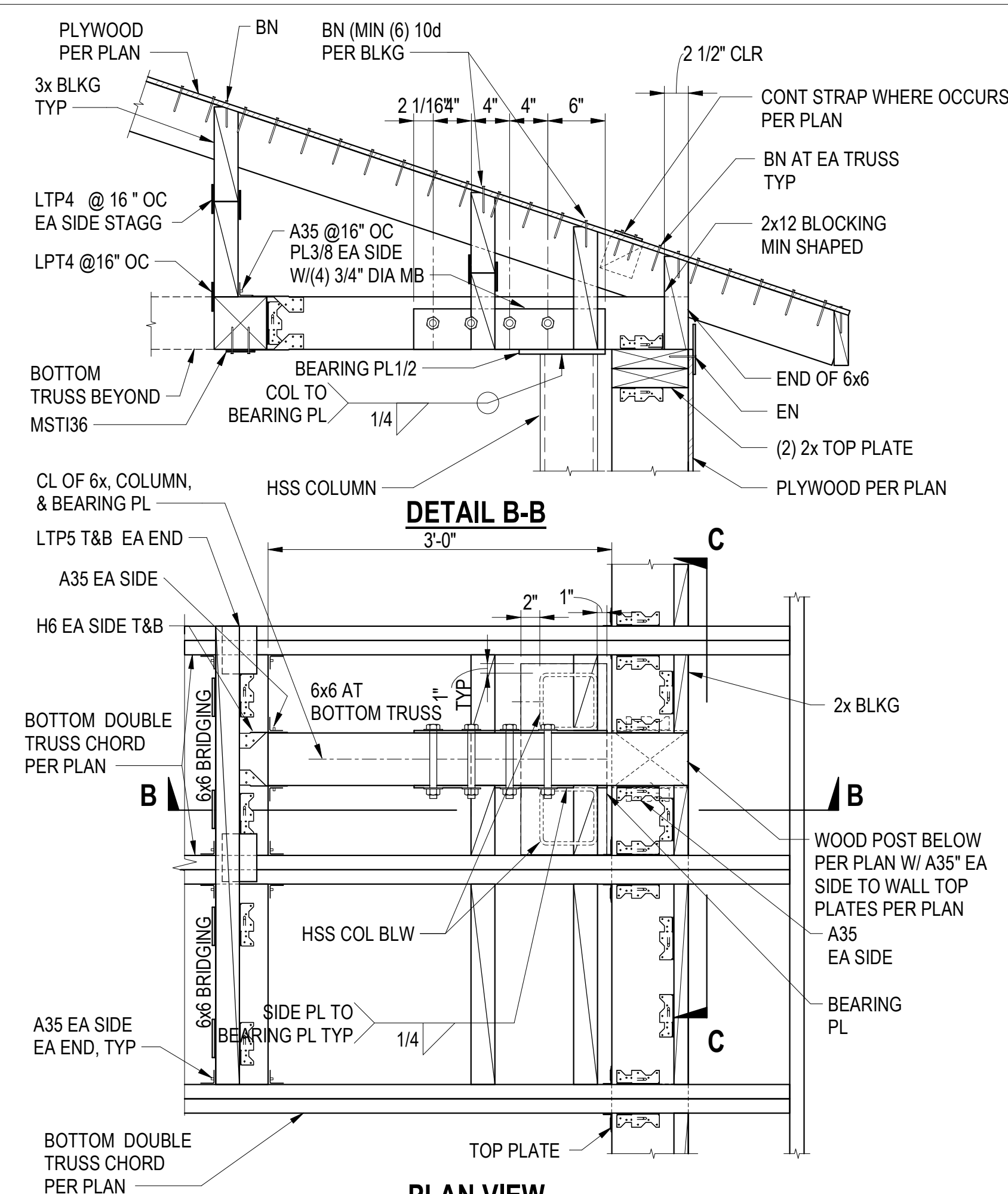
PLAN VIEW A-A



A DOUBLE HOLDOWN AT DOOR
SHEAR WALL END POST WITH
TWO HOLDOWNS CONNECTION DETAIL

SCALE: 1" = 1'-0"

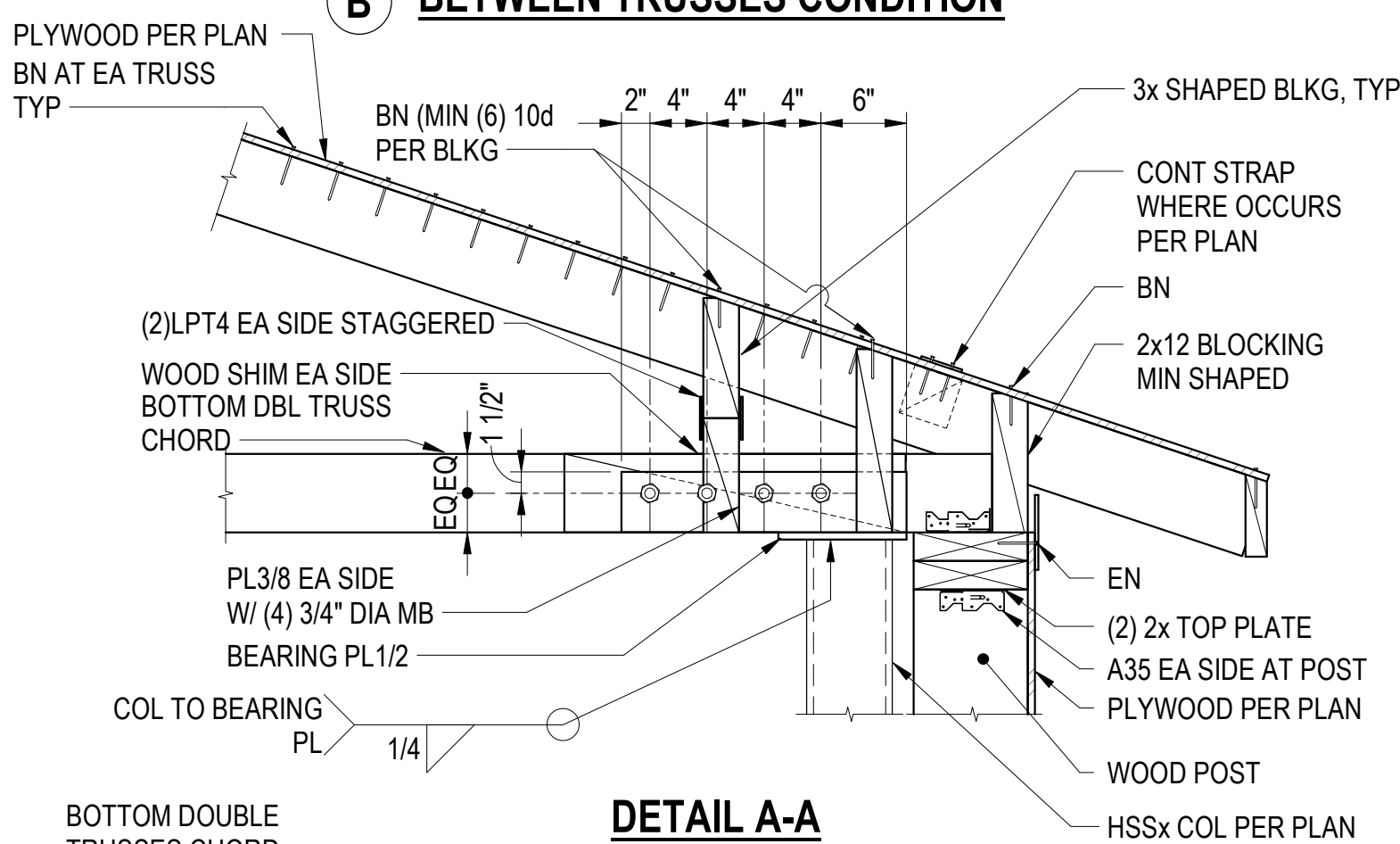
2



DETAIL B-B

PLAN VIEW

B BETWEEN TRUSSES CONDITION



DETAIL A-A

PLAN VIEW

A AT TRUSS CONDITION

DOUBLE HSS COLUMN AT ROLLUP DOOR TO ROOF FRAMING CONNECTION DETAIL

SCALE: 1" = 1'-0"

1

WILLIAM LOYD JONES
ARCHITECT

9415 culver boulevard
culver city, california
90232

TEL 310 392 3995

SO
saiful-bouquet
structural engineers

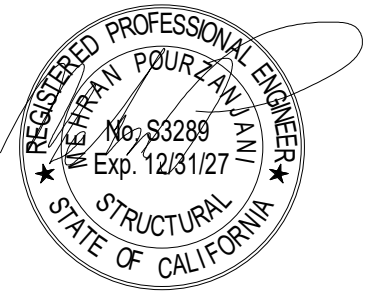
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SECTIONS AND DETAILS

FIRE STATION 46

MISSION VILLAGE

COUNTY OF LOS ANGELES FIRE DEPARTMENT
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